

# ***Minor Property***

***Drainage Report***

***Presented to: City of Marysville***



# Minor Property

## *Construction Drainage Report*

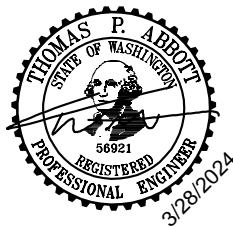
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## **SECTION 1: PROJECT OVERVIEW**

The proposed Minor project is approximately a 3.94-acre site. The project is a single-family residential development on parcel #00590700017600 and addressed at 8512 E Sunnyside School Road, Marysville WA, 98270. The project proposes to construct 29 new single-family lots along with associated private and public utilities proposed to serve project development. The site will gain access from the adjacent Cornelius/Lacey project to the west. No frontage improvements along E Sunnyside School Rd will be required. See the Vicinity Map in Appendix 1 for visual representation of the subject property.

### **Existing Site**

The parcel #00590700017600 is currently occupied by one single family home with associated outbuildings taking access from E Sunnyside School Rd. The central portion of the site containing the existing home consists of impervious surfaces associated with the buildings and paved driveways, while the rest of the site contains lawn and landscape. Existing topography generally descends to the north/northeast along moderate slopes, towards conveyance ditches along E Sunnyside School Rd. The project parcel is currently zoned WR-R-6-18 in the City of Marysville.

The proposed development currently straddles two stormwater threshold discharge basins (TDAs) since site runoff from developed surfaces splits to either the Northwest or southeast upon reaching the E Sunnyside School Rd conveyance system, prior to reaching the ¼ mile boundary downstream of the project site.

A Geotechnical Report has been prepared by Terra Associates on January 25, 2024 on the site. The report indicates “relatively high soil fines content, evidence of shallow perched groundwater, and the underlying presence of dense to very dense, cemented, till and till-like soils.” As a result, Terra does not recommend using infiltration for stormwater control onsite.

### **Proposed Development**

The proposed single-family project will construct 29 new single-family lots and will gain access from the adjacent Cornelius/Lacey project to the west upon its completion. Frontage improvements along E Sunnyside School Rd will not be required.

### **Proposed Drainage System**

This project is designed to comply with the 2019 Department of Ecology Stormwater Manual for Western Washington (2019 DOE SWMMWW). Stormwater will be mitigated via two detention vaults located along the northwestern and eastern portions of the site. Prior to vault intake, Perfilter cartridge filtration units will be used to treat stormwater runoff to meet basic water quality treatment requirements. The proposed detention vault systems will each discharge to their respective existing frontage conveyance ditches in E Sunnyside School Rd, thus maintaining the site's two natural discharge flow paths.

Onsite development will create 2.42 AC of new impervious surfaces which will be collected by either of the two detention vaults for mitigation and stormwater quality treatment. These areas are considered to be within the two Onsite Basins for stormwater modeling, which are referred to as Onsite Basin 1 and Onsite Basin 2 respectively. Upstream run-on from the property to the south will be collected and contribute to the Onsite Basin 2. Runoff from surfaces unable to be collected due to vertical constraints, approximately 0.16 AC, will be considered to bypass detention and be within the Bypass Basin(s).

### **Erosion/Sedimentation Control**

Erosion control measures that will be utilized during construction will include a combination of silt fence, storm drain inlet protection, interceptor swales, and sediment ponds. See Section 2.0 for discussion of how SWPPP Elements are addressed.

### **Minimum Requirements**

Per the 2019 DOE Manual, Minimum Requirements 1-9 apply to the proposed development.

#### Minimum Requirement #1: Preparation of Stormwater Site Plans

This report along with the preliminary plans satisfies the minimum requirement.

#### Minimum Requirement #2: Construction Stormwater Pollution Prevention

See Section 2 of this Report for the SWPPP BMP Elements, and the SWPPP (submitted as a separate document) for a complete discussion of erosion control BMP's and their use specific to the site.

Minimum Requirement #3: Source of Pollution

Permanent source control BMPs are not applicable for the subject site since the associated activities for the new residence do not fall within the types of facilities listed within Volume IV of the DOE Manual (Residential developments are not required to implement source control BMP's). BMPs for erosion and sedimentation control will be specified in the Construction Plans and the CSWPP.

Minimum Requirement #4: Preservation of Natural Drainage Systems and Outfalls

Flow from the site will preserve its natural drainage patterns to the north towards E Sunnyside School Rd frontage. Runoff then flows either towards King Creek to the west of the site, or to Stevens Creek to the southeast of the site.

Minimum Requirement #5: On-Site Stormwater Management

The project proposes BMP T5.13 soils to be underlain within all pervious areas that are disturbed by development. Generally, all other infiltration-related BMPs are infeasible due to "relatively high soil fines content, evidence of shallow perched groundwater, and the underlying presence of dense to very dense, cemented, till and till-like soils" per Geotechnical analysis. Generally, other dispersion related BMPs are considered infeasible due to the absence of adequate flow path lengths. Please see Section 4.5 for additional discussion of Onsite Stormwater Management and Low Impact Development BMP feasibility.

Minimum Requirement #6: Runoff Treatment

Proposed new pollution generating impervious surfaces (PGIS) will exceed the 5,000 SF threshold and thus basic water quality treatment will be provided via water quality treatment structures that will treat stormwater runoff discharging from the stormwater detention vaults. Two Perfilter cartridge filtration units are proposed for this purpose. See Section 4.0 for additional discussion regarding proposed stormwater management and water quality treatment measures.

Minimum Requirement #7: Flow Control

The project will exceed the 10,000 SF new plus replaced impervious threshold within each TDA, thus is required to provide flow control. Two concrete detention vaults will be installed in the northwestern and eastern portions of the project site, accessible via access Tracts. The vaults will discharge via point discharges at historic, mitigated rates towards existing flow paths along E Sunnyside School Rd frontage. Please see Section 4.0 for additional flow control modeling and parameters for detention sizing.

Minimum Requirement #8: Wetlands Protection

There are no critical wetland areas, streams, or associated buffers on/within close proximity to the project site.

Minimum Requirement #9: Operation and Maintenance

See Operations and Maintenance in Section 6 of this report.

## SECTION 2: TEMPORARY EROSION AND SEDIMENT CONTROL DESIGN

### SWPPP Design Elements

A Construction Stormwater Pollution Prevention Plan (SWPPP) will be provided prior to construction. The SWPPP report is modeled under the guidelines of 1.3.4.2 MR2 of the DOE Manual. Construction SWPPP Elements #1 through #13 are addressed below.

#### Element #1 – Mark Clearing Limits

All clearing limits will be delineated with high visibility plastic fence and/or silt fence. See sheets ER-01 of the construction plans (to be submitted at a later date) for locations and details.

#### Element #2 – Establish Construction Access

Stabilized construction accesses will be installed as shown on the preliminary plans. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for locations and details.

#### Element #3 – Control Flow Rates

Detention of construction period runoff will be provided by means of sediment ponds on the site. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for location and details for flow and sediment control BMP's.

#### Element #4 – Install Sediment Controls

Silt fence, catch basin protection, and the temporary sediment pond will be utilized to contain sediments within the project's clearing limits. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for locations and details.

#### Element #5 – Stabilize Soils

Exposed soils will be stabilized as specified in the Grading and Erosion Control Notes with temporary and permanent seeding, mulching, and plastic covering. See sheet ER-02 of the construction plans (to be submitted at a later date) for notes.

#### Element #6 – Protect Slopes

Slopes are minor on the subject site. Slopes shall be protected as specified under Element #5.

#### Element #7 – Protect Drain Inlets

Storm drain inlet protection will be utilized to contain sediments within the project's clearing limits. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for locations and details.

#### Element #8 – Stabilize Channels and Outlets

Temporary channels, shall be stabilized with check dams. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for locations and details.

#### Element #9 – Control Pollutants

Pollutants shall be controlled as specified in Volume IV of the 2019 DOE Manual—Source Control BMPs to address potential sources of pollution which may exacerbate possible soil/groundwater contamination identified onsite.

#### Element #10 – Control De-Watering

There will be no de-watering as a part of this project. See sheet ER-02 of the construction plans (to be submitted at a later date) for notes.

#### Element #11 – Maintain BMPs

Maintenance of the BMPs is specified within the Construction Sequence and Grading and Erosion Control Notes. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for the Construction Sequence and notes.

#### Element #12: Manage the Project

The Grading and Erosion Control Notes specify seasonal work limitations. Maintenance of the BMPs is specified within the Construction Sequence and Grading and Erosion Control Notes. See sheets ER-01 and ER-02 of the construction plans (to be submitted at a later date) for the Construction Sequence and notes.

Element #13: Protect on-site stormwater management BMPs

On-site stormwater management BMPs used for runoff from roofs and other hard surfaces are not feasible due to soil conditions and proposed project density.

## SECTION 3: DOWNSTREAM ANALYSIS

### Task 1. Study Area Definition and Maps

Snohomish County Bare Earth LiDAR, survey, and 2021 aerial photography were the best topographical references available for the area containing the site. The limits of the downstream analysis extend roughly 0.25 miles beyond the subject property's natural discharge location.

### Task 2. Resource Review

All of the resources below have been reviewed for existing and potential issues near the project site:

#### Adopted Basin Plans

No Adopted Basin Plans were located that include the project site.

#### Drainage Basin

This site is split between two TDAs. One is in the King Creek subbasin, within the Ebey Slough watershed. The other is in the Stevens Creek Subbasin, which is tributary to Lake Stevens followed by the Snohomish River.

#### Floodplain / Floodway (FEMA) maps

Per FEMA Floodplain map #53061C0736F the subject property is not within a floodplain.

#### Critical Areas Map

There are no critical wetland areas, streams, or associated buffers on/within close proximity to the project site.

#### Drainage Complaints

No relevant issues were identified near the proposed site.

#### Road Drainage Problems

No issues were identified near the proposed site.

#### Soil Survey

Site soils are classified as Tokul gravelly medial loam (0 to 8 percent slopes) which is classified as Hydrologic Soil Group B. Per geotechnical analysis, site soils do not exhibit acceptable infiltration rates, which is consistent with NRCS soil mapping data.

#### Wetland Inventory Maps

There are no critical wetland areas, streams, or associated buffers on/within close proximity to the project site.

#### Migrating River Studies

Migrating River Studies are not considered applicable to the proposed development.

#### Section 303d List of Polluted Waters

Washington State Department of Ecology's Water Quality Assessment for Washington contains listings for King creek and Stevens Creek which the project is tributary to. Please refer to Appendix 3 for copies of applicable 303(d) listings.

#### Water Quality Problems

King Creek has a category 5 listing in the DOE Water Quality Assessment Review tools for temperature, while Stevens Creek does not have any listings. The development however will improve water quality tributary to the creek via basic treatment device(s).

#### Stormwater Compliance Plans

Not applicable to the proposed project.



### **Task 3. Field Inspection/Downstream Analysis**

On November 27<sup>th</sup>, 2023, a Downstream Analysis was performed at the site. The weather consisted of 46°F and clear skies. The following observations were verified during the visit.

The subject property areas consist primarily of flat to moderately sloped lawn areas surrounding the existing home, associated outbuildings, and driveways that are located in the central/southern portion of the site. There are some areas of upstream sheet flow from the property to the south due to existing topography.

Two flow paths have been identified leaving the east and west sides of the parcel that do not converge within one quarter mile of the site. As a result, the site is located within two threshold discharge areas (TDAs). Both flow paths are formed where runoff travels north towards E Sunnyside School Rd on either the west or east side of the TDA basin split (Image 1).

TDA #1 / Flowpath 1 – Overland flows traveling northwest are collected by an existing roadside ditch along E Sunnyside School Rd (Images 2 & 3). The ditch carries runoff to an existing pond/wetland area (Image 4) located on the neighboring property to the west. Overflow from the pond/wetland travels back to E Sunnyside School Rd (Image 5), crossing under the road to the north side (Image 6). Flow becomes more concentrated as it continues along the north side of E Sunnyside School Rd towards the intersection with 83<sup>rd</sup> Ave NE. Flow converges at a pipe crossing west under 83<sup>rd</sup> Ave NE (Image 9) The pipe outlets on the west side of 83<sup>rd</sup> Ave NE (Image 10), where King Creek continues beyond the quarter mile boundary.

TDA #2 / Flowpath 2 – Overland flows traveling northeast are collected by an existing roadside ditch along E Sunnyside School Rd (Images 11 & 12). The roadside ditch continues east along E Sunnyside School Rd to the eastern property corner (Image 13). E Sunnyside School Road begins to turn southeast beyond the property and the ditch continues under several driveway-culvert crossings (Image 14) until it reaches Densmore Rd. The flow crosses under Densmore Rd (Image 15) and continues southeast beyond the quarter mile buffer (Image 16), eventually joining Stevens Creek.

### **Task 4. Drainage System Description and Problem Descriptions**

Based on the information available and all the resources available including visual inspection of the downstream flow path to the ¼-mile boundary, there is no evidence of existing or anticipated downstream drainage problems. All flows are adequately carried through natural channels to the quarter mile buffer of analysis.

### **Task 5. Mitigation of Existing or Potential Drainage Problems**

No evidence of existing or potential problems with downstream drainage conveyance infrastructure was found. Mitigation is not required.

## SECTION 4: DETENTION AND WATER QUALITY TREATMENT DESIGN

### 4.1 Predeveloped Site Hydrology

The pre-developed and developed conditions were modeled in WWHM for the purpose of matching historic, forested condition flow rates and durations. Based on the site location, the WWHM used the Everett Gage with a Precipitation Scale factor of 1.2. For visual representation of the listed basins, see Figure 4.0, "Predeveloped Hydrology Map".

#### Onsite Basin 1:

The predeveloped condition applied to the Onsite Basin 1 results in a forested land cover condition. This basin is tributary to TDA 1 and the values modeled in WWHM are as follows:

Table 1: Predeveloped Conditions: Onsite Basin 1

Onsite Basin 1	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, mod	0.90
Forest, steep	0.18
<b>Total</b>	<b>1.08</b>

#### Onsite Basin 2:

The predeveloped condition applied to the Onsite Basin 2 results in a forested land cover condition. This basin is tributary to TDA 2 and the values modeled in WWHM are as follows:

Table 2: Predeveloped Conditions: Onsite Basin 2

Onsite Basin 2	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, mod	2.00
Forest, steep	0.77
<b>Total</b>	<b>2.77</b>

#### Upstream Basin:

The predeveloped condition applied to the Upstream Basin results in existing land cover condition. In the developed condition, this basin will be collected alongside Onsite Basin 2. The values as modeled in WWHM are as follows:

Table 3: Predeveloped Conditions: Upstream Basin

Upstream Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, mod	0.80
Lawn, mod	0.20
Rooftops	0.12
Driveways, flat	0.23
<b>Total</b>	<b>1.35</b>

#### 4.2 Developed Site Hydrology

In the developed condition, the proposed residential project will construct 29 single-family lots. Emergency and standard access drives along with associated private and public utilities are proposed to serve project development.

In compliance with the City of Marysville Design & Development Standards and the 2019 DOE Manual, all runoff from onsite developed/disturbed surfaces will be collected, treated, and discharged directly to existing/historic flow paths or will bypass detention and be mitigated within the proposed flow control system. Lots were modeled to have 65% impervious coverage.

##### Onsite Basin 1:

The developed Onsite Basin is 1.00 acres and includes the majority of the developed site within its TDA 1 boundaries. In the developed condition, the Onsite Basin has been modeled using WWHM with the following areas and ground cover designations:

Table 4: Developed Conditions: Onsite Basin 1

Onsite Basin 1	
<u>Ground Cover</u>	<u>Area (acre)</u>
Pasture, flat	0.40
Roads, flat	0.08
Rooftops	0.33
Driveways, flat	0.16
Sidewalks, flat	0.03
<b>Total</b>	<b>1.00</b>

##### Bypass Basin 1:

Bypass Basin 1 is 0.08 acres and is comprised of sloped landscape area adjacent to the detention vault/within Tract 999 that cannot be topographically collected into the detention system. Bypass Basin 1 was modeled using WWHM with the following areas and ground cover designations:

Table 5: Developed Conditions: Bypass Basin 1

Bypass Basin 1	
<u>Ground Cover</u>	<u>Area (acre)</u>
Pasture, mod	0.04
Pasture, steep	0.04
<b>Total</b>	<b>0.08</b>

##### Onsite Basin 2:

The developed Onsite Basin is 2.67 acres and includes the majority of the developed site within its TDA 2 boundaries. In the developed condition, the Onsite Basin has been modeled using WWHM with the following areas and ground cover designations:

Table 6: Developed Conditions: Onsite Basin 2

Onsite Basin 2	
<u>Ground Cover</u>	<u>Area (acre)</u>
Pasture, flat	0.84
Roads, flat	0.29
Rooftops	1.00
Driveways, flat	0.45
Sidewalks, flat	0.09
<b>Total</b>	<b>2.67</b>

Bypass Basin 2:

Bypass Basin 2 is 0.10 acres and is comprised of sloped landscape area adjacent to the detention vault/within Tract 998 and Lot 29 that cannot be topographically collected into the detention system. Bypass Basin 2 was modeled using WWHM with the following areas and ground cover designations:

Table 7: Developed Conditions: Bypass Basin 2

Bypass Basin 2	
<u>Ground Cover</u>	<u>Area (acre)</u>
Pasture, mod	0.06
Pasture, steep	0.04
<b>Total</b>	<b>0.10</b>

Upstream Basin:

The developed condition applied to the Upstream Basin results in existing land cover condition. In the developed condition, this basin will be collected alongside Onsite Basin 2. The values as modeled in WWHM are as follows:

Table 8: Developed Conditions: Upstream Basin

Upstream Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, mod	0.80
Lawn, mod	0.20
Rooftops	0.12
Driveways, flat	0.23
<b>Total</b>	<b>1.35</b>

### 4.3 Detention Facility Design

The proposed detention vault facilities used for mitigating developed condition flows were designed in compliance with the 2019 DOE requirements to model hydrologic conditions and detention in a continuous runoff model (WWHM2012) where the following evaluation parameters are employed:

*“Flow duration is computed by counting the number of flow values that exceed a specified flow level. The specified flow levels used by WWHM in the flow duration analysis are listed below.*

1. 50% of the 2-year predevelopment peak flow.
2. 100% of the 2-year predevelopment peak flow.
3. 100% of the 50-year predevelopment peak flow.

*There are three criteria by which flow duration values are compared:*

1. *If the postdevelopment flow duration values exceed any of the predevelopment flow levels between 50% and 100% of the 2-year predevelopment peak flow values (100 Percent Threshold) then the flow duration requirement has not been met.*
2. *If the postdevelopment flow duration values exceed any of the predevelopment flow levels between 100% of the 2-year and 100% of the 50-year predevelopment peak flow values more than 10 percent of the time (110 Percent Threshold) then the flow duration requirement has not been met.*
3. *If more than 50 percent of the flow duration levels exceed the 100 percent threshold then the flow duration requirement has not been met.”*

Detention Vault Facilities

The proposed cast in place concrete detention vault facilities detains, and releases collected storm water runoff from the two Onsite Basins. The facilities are located within Tract 999 (Vault 1) and Tract 998 (Vault 2). Flows from the Onsite Basin are collected and conveyed to the detention vault via a proposed network of catch basins and storm water conveyance pipes.

Detailed WWHM output is provided in Appendix 4. A summary of the detailed statistics and inputs used for modeling the system in WWHM2012 can be found below.

Table 9.1: Detention Vault 1 Design Summary

Detention Vault 1	
Live Storage Bottom Area (modeled)	2,200 SF
Live Storage Bottom Area (provided)	2,200 SF
Number of Cells	1
Cell Dimensions	22' x 100'
Begin Live Storage Elevation	343.00'
Riser Height	6.00'
Volume (modeled)	13,200 CF
Volume (provided)	13,200 CF
Top of Riser Elevation	349.00'
Top Outside of Vault Elevation	350.50'

See table below for the flow rates and water surface elevations by storm event for detention vault 1.

Table 10.1: Flow Rates and Water Surface Elevations by Storm Event

Storm Event	Predeveloped Rate (cfs)	Mitigated Rates (cfs)
2-Year	0.0429	0.0251
10-Year	0.0895	0.0456
50-Year	0.1459	0.0719
100-Year	0.1747	0.0860

The table below is design summary for Detention Vault 2.

Table 9.2: Detention Vault 2 Design Summary

Detention Vault 2	
Live Storage Bottom Area (modeled)	4,968 SF
Live Storage Bottom Area (provided)	4,968 SF
Number of Cells	2
Cell Dimensions	(23' x 140') & (23' x 76')
Begin Live Storage Elevation	337.00'
Riser Height	6.50'
Volume (modeled)	32,292 CF
Volume (provided)	32,292 CF
Top of Riser Elevation	403.50'
Top Outside of Vault Elevation	405.00'

See table below for the flow rates and water surface elevations by storm event for the detention vault 2.

Table 10.2: Flow Rates and Water Surface Elevations by Storm Event

Storm Event	Predeveloped Rate (cfs)	Mitigated Rates (cfs)
2-Year	0.3216	0.1609
10-Year	0.6210	0.2542
50-Year	0.9733	0.3601
100-Year	1.1519	0.4127

#### 4.4 Water Quality Treatment

##### Perkfilter 1 - West

Water Quality Treatment for the Onsite Basin 1 is accomplished through a Perkfilter structure located upstream of the detention vault 1. A summary of design criteria is provided below:

Table 11: Perkfilter 1 Design Summary

48" Ø Perkfilter Manhole	
Tributary Area	1.00 AC
Tributary Impervious Area	0.60 AC
Water Quality Flow Rate	0.062 cfs
2-yr unmitigated peak	0.33 cfs
Number of Cartridges	2
Cartridge Height	12"+18"
Internal Drop	2.3'
Peak Flow Rate	1.23 cfs
Peak Flow Storm Event	100-year

##### Perkfilter 2 - East

Water Quality Treatment for the Onsite Basin 2 is accomplished through a Perkfilter structure located upstream of the detention vault 2. A summary of design criteria is provided below:

Table 12: Perkfilter 2 Design Summary

Perkfilter Vault – 6'x8'	
Tributary Area	4.02 AC
Tributary Impervious Area	2.18 AC
Water Quality Flow Rate	0.233 cfs
2-yr unmitigated peak	1.24 cfs
Number of Cartridges	9
Cartridge Height	12"+18"
Internal Drop	2.3'
Peak Flow Rate	4.48 cfs
Peak Flow Storm Event	100-year

#### 4.5 Onsite Stormwater Management

The project does not propose the LID performance standard and minimum requirements 1-9 are required for the project but choose to implement List #2 to evaluate low impact design. The following BMP's below are assessed for implementation:

##### Lawn and Landscaped Areas:

1. *Post-Construction Soil Quality and Depth*
  - BMP T5.13 soils will be applied to all permeable and landscaped areas in developed condition.
    - i. **Conclusion: Infeasible**

##### Roofs:

1. *Downspout Full Infiltration per BMP T5.10A or Downspout Full Dispersion per BMP T5.30*
  - Infiltration is not feasible on site, which has been confirmed by testing found in the geotechnical report and thus BMP T5.10A is infeasible. Due to site specific constraints including building and fire lane location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site per BMP T5.30.
    - i. **Conclusion: Infeasible**
2. *Bioretention*
  - Due to spatial constraints provided by the development footprint and infiltration infeasibility confirmed by testing in the geotechnical report, a bioretention facility cannot be designed to provide the required horizontally projected surface area.
    - i. **Conclusion: Infeasible**
3. *Downspout Dispersion per BMP T5.10B.*
  - Due to site specific constraints including building and fire lane location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site.
    - i. **Conclusion: Infeasible**
4. *Perforated Stub-Out Connections per BMP T5.10C.*
  - No stub-out connections will be implemented in the design as soils are not suitable for infiltration as well as the site's proximity to steep slopes.
    - i. **Conclusion: Infeasible**

##### Other Hard Surfaces:

1. *Full Dispersion per BMP T5.30*
  - Due to site specific constraints including building and fire lane location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site.
    - i. **Conclusion: Infeasible**
2. *BMP T5.15 Permeable Pavement*
  - Infiltration is not feasible on site which has been confirmed by testing found in the geotechnical report.
    - i. **Conclusion: Infeasible**
3. *Bioretention*
  - Due to spatial constraints provide by the development footprint and infiltration infeasibility as confirmed by testing in the geotechnical report, a bioretention facility cannot be designed to provide the required horizontally projected surface area.
    - i. **Conclusion: Infeasible**
4. *Sheet Flow Dispersion or Concentrated Flow Dispersion in accordance with BMP T5.12 or BMP T5.11*
  - Due to site specific constraints including building and fire lane location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site.
    - i. **Conclusion: Infeasible**

## SECTION 5: CONVEYANCE DESIGN

### Conveyance Capacity Analysis

The proposed collection and conveyance systems are comprised of catch basins, storm drainage pipe and two detention vaults. Catch basins have been located such that each section of storm drainage pipe can adequately convey associated tributary area flows. Conveyance analyses have been performed for all mainline pipes and structures tributary to the proposed detention vault to ensure no catch basins overtop and cause roadway or property flooding.

### 5.1 Onsite Collection and Conveyance

The onsite collection and conveyance systems were designed for the 50-year, 24-hour storm event with the Rational Method by utilizing Autodesk's Storm and Sanitary Analysis 2024. This analysis was performed to ensure that during the 50-year, 24-hour storm event, there would be no significant catch basin overtopping. The table below shows a summary of the Conveyance basin areas used in the analysis. See Appendix 5 for the Conveyance Basins Map for a visual representation of measured areas.

	BASIN	IMPERVIOUS	PERVIOUS	TOTAL (AC)
VAULT 1	1	0.33	0.13	0.46
	2	0.05	0.01	0.06
	3	0.23	0.09	0.32
VAULT 2	5	0.41	0.70	1.11
	6	0.04	0.01	0.05
	7	0.63	0.27	0.90
	8	0.36	0.13	0.49
	9	0.50	0.19	0.69

### Vault 1

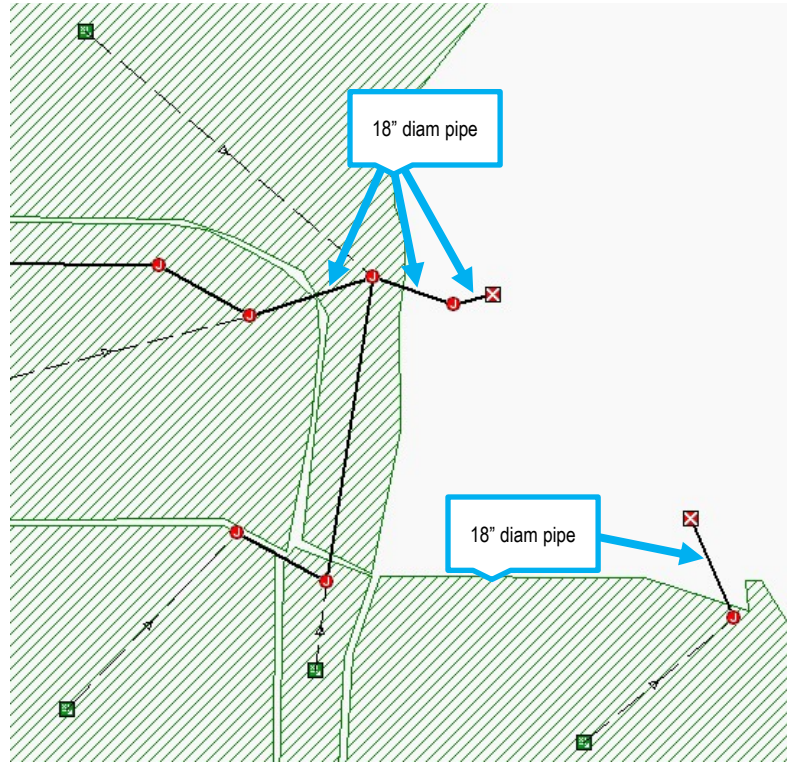
According to the modeling performed, the proposed single run system for Vault 1 did not have any conveyance issues upstream of the detention vault. All conveyance pipes initially sized at 12" diameter will remain as is. See Appendix 5 for the SSA model layout and SSA model results.



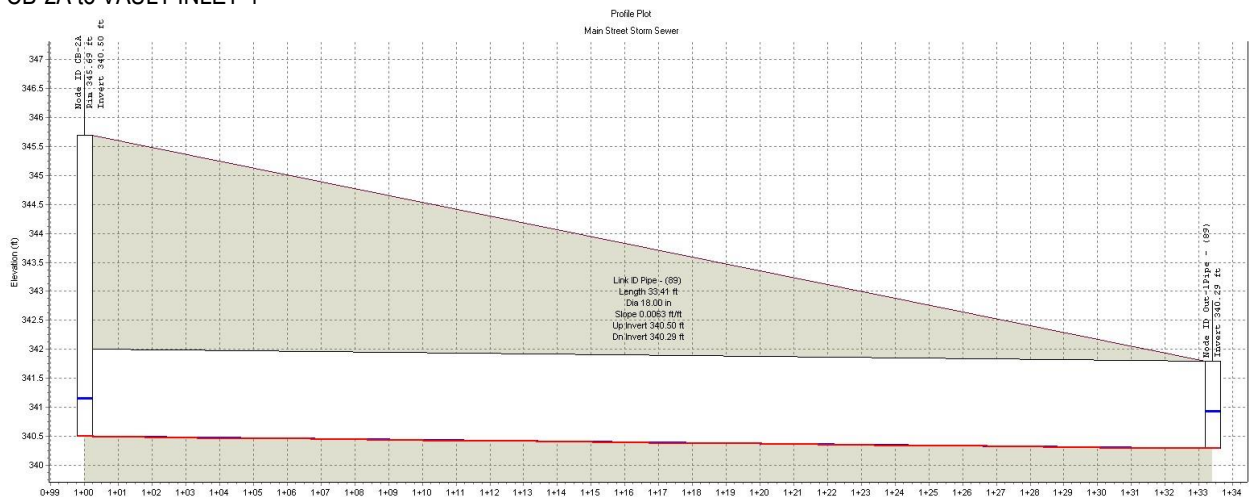


## Vault 2

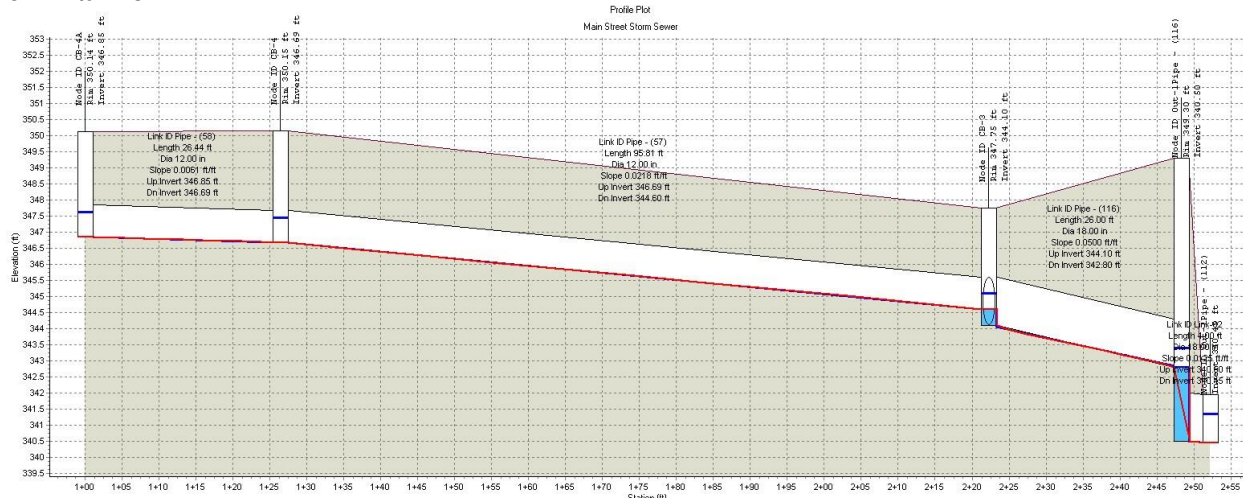
According to the initial modeling performed, the proposed system had several conveyance issues upstream of the detention vault. All conveyance pipes were initially sized at 12" diameter. To fix conveyance issues, four pipe runs were upsized 18" diameter which resulted in a resolution of all overtopping/flooding. See Appendix 5 for the SSA model layout with 18" upsized pipes highlighted in yellow and SSA model results.



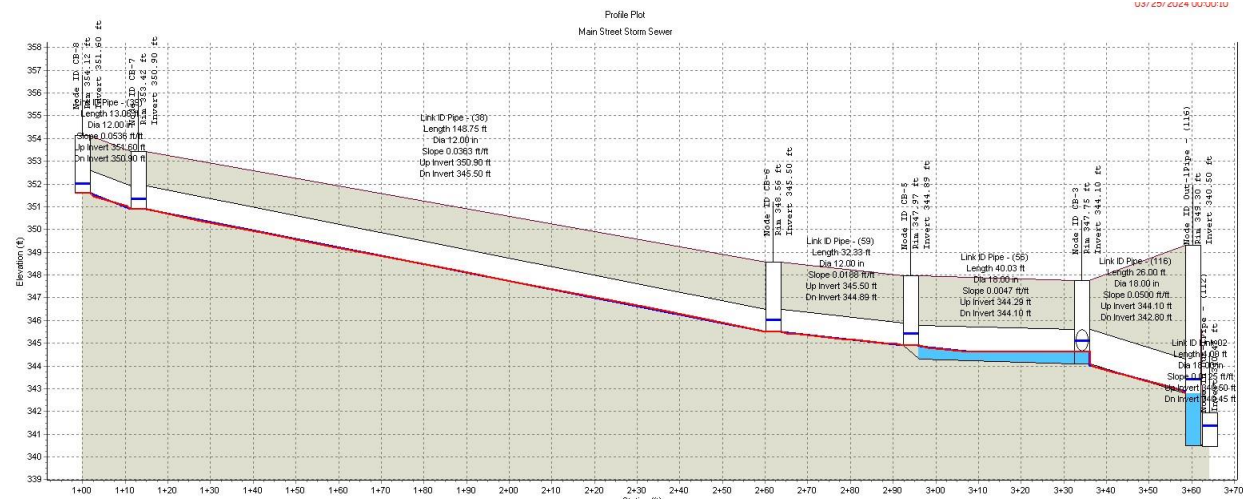
## CB-2A to VAULT INLET-1



## CB-4A to VAULT INLET-2



## CB-8 to VAULT INLET-2



## 5.2 Downstream Discharge Line

The conveyance capacity analyses for each of the pipes downstream the two detention facilities were performed as a Manning's Equation evaluation of the discharge pipe with the smallest diameter, shallowest slope and largest flow capacity requirement. The pipe was analyzed against the 100-year unmitigated flow of two the proposed detention vault outfalls. Analyses confirm adequate designs. The following is a summary of the Manning's analysis performed on the pipes:

### Detention Vault 1

Pipe Diameter: 12" (1.0')  
 Min Pipe Slope: 0.50%  
 100-year flow: 0.91 cfs (WWHM unmitigated – 100-yr flow)  
 Max Flow Depth: 0.90' (90%)  
 Resultant Flow Depth: 0.34'  
 Evaluation: System Adequate

The system is adequate because the resultant flow depth is 0.34' for the 100-year maximum pipe flow, compared to a maximum flow depth of 1.0' (90%). The results spreadsheet is located in Appendix 5.

Detention Vault 2

Pipe Diameter: 18" (1.5')

Min Pipe Slope: 0.50%

100-year flow: 3.34 cfs (WWHM unmitigated – 100-yr flow)

Max Flow Depth: 1.35' (90%)

Resultant Flow Depth: 0.57'

Evaluation: System Adequate

The system is adequate because the resultant flow depth is 0.57' for the 100-year maximum pipe flow, compared to a maximum flow depth of 1.35' (90%). The results spreadsheet is located in Appendix 5.

## **SECTION 6: OPERATIONS AND MAINTENANCE MANUAL**

The proposed storm drainage system consists of buried pipes, catch basins, detention vaults, and perfilter water quality treatment structures. These facilities will require periodic maintenance and inspection. Inspection and maintenance procedures are contained on the following pages.

## **SECTION 7: SPECIAL REPORTS AND STUDIES**

The following studies were conducted in preparation of this Report:

- Geotechnical Report, Terra Associates Inc, January 25, 2024

## **Appendix 1: Project Overview**

1. Vicinity Map
2. Existing Conditions Map
3. Proposed Development Map

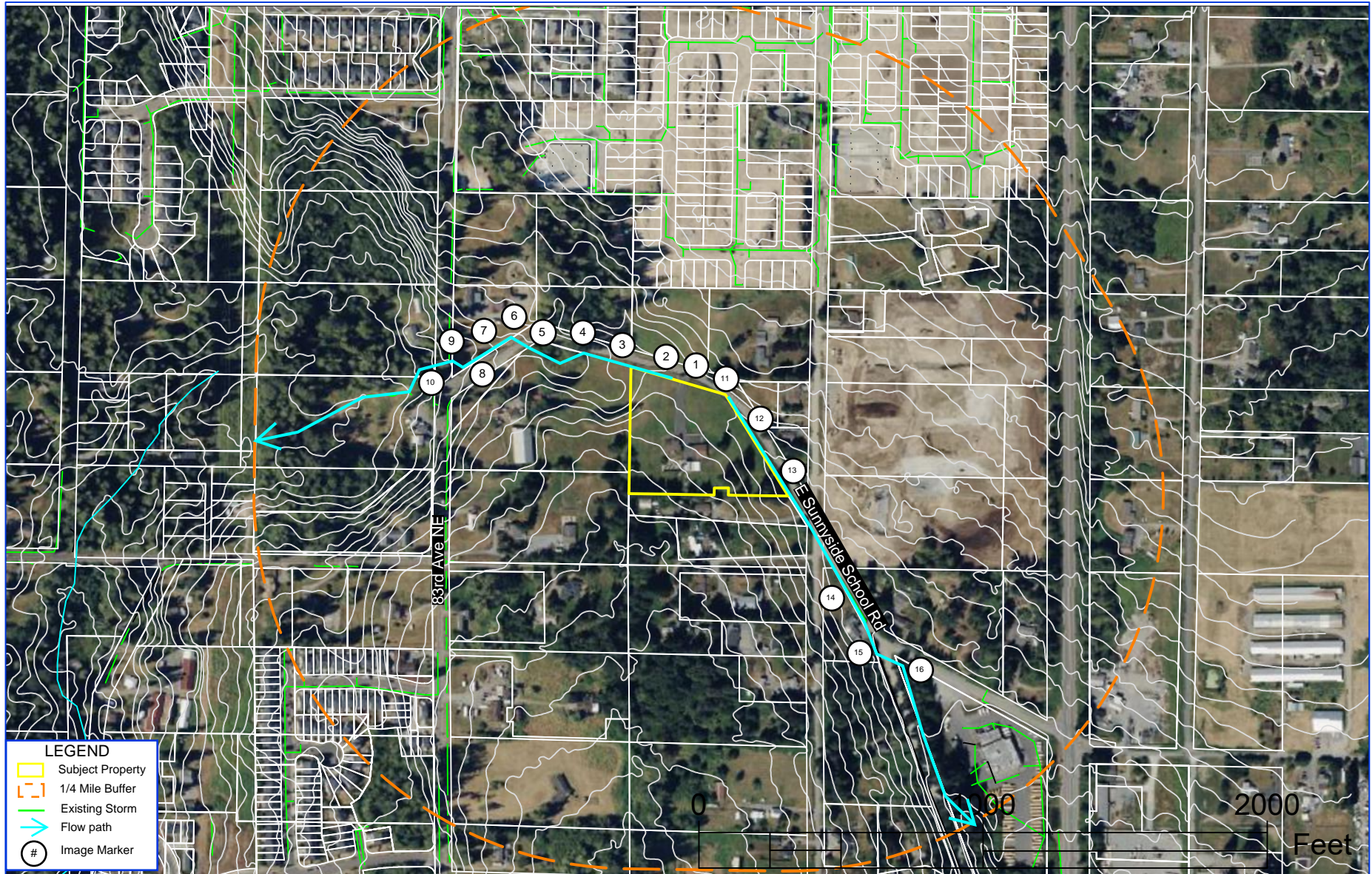
## Appendix 2: Temporary Erosion and Sediment Control Design

### 1. TESC Plans

### **Appendix 3: Downstream Analysis**

1. Downstream Analysis Map
2. Downstream Analysis Site Visit Pictures
3. USDA Soils Map & Description





**LEGEND**

- Subject Property
- 1/4 Mile Buffer
- Existing Storm
- Flow path
- # Image Marker

SOUTH LAKE RIDGE, LLC  
MINOR PROPERTY

DOWNSTREAM ANALYSIS MAP



**Solid Ground Engineering**  
8105 166th Ave NE  
Redmond, WA 98052

JOB NUMBER:	23-0011	DATE:	2-7-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

C:\Users\CooerDanby\Documents\CAD-GIS\Map\Minor\_Marysville\Minor\_Downstream\_Map.dwg

## Downstream Analysis Photographs



**Image 1:** Middle of existing driveway along E Sunnyside School Rd where TDA #1 & #2 split west or east

### TDA #1 – Flowpath 1



**Image 2:** Facing west along E Sunnyside School Rd where Flowpath 1 continues in a roadside ditch



**Image 3:** Northwest property corner where runoff sheet flows west down existing slopes



**Image 4:** Property west of project site along E Sunnyside School Rd where runoff enters existing pond/wetland



**Image 5:** west of existing pond/wetland where overflow runoff continues back to cross under E Sunnyside School Rd



**Image 6:** north side of E Sunnyside School Rd where channelized flow continues west



**Image 7:** north side of E Sunnyside School Rd where channelized flow continues west



**Image 8:** Intersection of E Sunnyside School Rd and 83<sup>rd</sup> Ave NE



**Image 9:** Intersection of E Sunnyside School Rd and 83<sup>rd</sup> Ave NE where flow crosses west under 83<sup>rd</sup> via culvert



**Image 10:** Culvert outlet west of 83<sup>rd</sup> where King Creek continues to quarter mile boundary

**TDA #2 – Flowpath 2**



**Image 11:** Facing east along E Sunnyside School Rd where Flowpath 2 continues in a roadside ditch



**Image 12:** Facing southeast along E Sunnyside School Rd where Flowpath 2 continues in a roadside ditch



**Image 13:** east property corner along E Sunnyside School Rd where Flowpath 2 continues



**Image 14:** along E Sunnyside School Rd where Flowpath 2 continues





**Image 15:** near intersection of E Sunnyside School Rd and Densmore Rd, flow crosses east under Densmore Rd

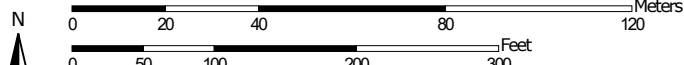


**Image 16:** east side of Densmore Rd, flow continues southeast beyond quarter mile boundary

Soil Map—Snohomish County Area, Washington



Map Scale: 1:1,620 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 10N WGS84



Natural Resources Conservation Service

Web Soil Survey National Cooperative Soil Survey

11/17/2023 Page 1 of 3

## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

### Water Features



Streams and Canals

### Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

### Background



Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

**Warning:** Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Snohomish County Area, Washington

Survey Area Data: Version 25, Aug 29, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 14, 2022—Sep 1, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
72	Tokul gravelly medial loam, 0 to 8 percent slopes	6.0	100.0%
<b>Totals for Area of Interest</b>		<b>6.0</b>	<b>100.0%</b>

## Snohomish County Area, Washington

### 72—Tokul gravelly medial loam, 0 to 8 percent slopes

#### Map Unit Setting

*National map unit symbol:* 2t61k

*Elevation:* 160 to 1,150 feet

*Mean annual precipitation:* 45 to 70 inches

*Mean annual air temperature:* 46 to 52 degrees F

*Frost-free period:* 140 to 200 days

*Farmland classification:* All areas are prime farmland

#### Map Unit Composition

*Tokul and similar soils:* 85 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Tokul

##### Setting

*Landform:* Hillslopes, till plains

*Landform position (two-dimensional):* Toeslope

*Landform position (three-dimensional):* Side slope, tread

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Parent material:* Volcanic ash mixed with loess over glacial till

##### Typical profile

*O<sub>i</sub> - 0 to 1 inches:* slightly decomposed plant material

*O<sub>a</sub> - 1 to 2 inches:* highly decomposed plant material

*A - 2 to 6 inches:* gravelly medial loam

*B<sub>s1</sub> - 6 to 9 inches:* gravelly medial loam

*B<sub>s2</sub> - 9 to 17 inches:* gravelly medial loam

*B<sub>s3</sub> - 17 to 24 inches:* gravelly medial loam

*BC - 24 to 33 inches:* gravelly medial fine sandy loam

*2B<sub>sm</sub> - 33 to 62 inches:* cemented material

##### Properties and qualities

*Slope:* 0 to 8 percent

*Depth to restrictive feature:* 20 to 39 inches to densic material; 20 to 39 inches to cemented horizon

*Drainage class:* Moderately well drained

*Capacity of the most limiting layer to transmit water (K<sub>sat</sub>):* Very low to moderately low (0.00 to 0.06 in/hr)

*Depth to water table:* About 18 to 36 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Moderate (about 8.7 inches)

### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3s  
*Hydrologic Soil Group:* B  
*Ecological site:* F002XA005WA - Puget Lowlands Moist Forest  
*Forage suitability group:* Limited Depth Soils (G002XN302WA),  
Limited Depth Soils (G002XF303WA)  
*Other vegetative classification:* Limited Depth Soils  
(G002XN302WA), Limited Depth Soils (G002XF303WA)  
*Hydric soil rating:* No

### **Minor Components**

#### **Pastik**

*Percent of map unit:* 5 percent  
*Landform:* Terraces  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Hydric soil rating:* No

#### **Barneston**

*Percent of map unit:* 5 percent  
*Landform:* Moraines, eskers, kames  
*Landform position (two-dimensional):* Summit, shoulder  
*Landform position (three-dimensional):* Interfluve, crest  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Hydric soil rating:* No

#### **Norma**

*Percent of map unit:* 3 percent  
*Landform:* Drainageways, depressions  
*Landform position (three-dimensional):* Dip  
*Down-slope shape:* Linear, concave  
*Across-slope shape:* Concave  
*Hydric soil rating:* Yes

#### **Mckenna**

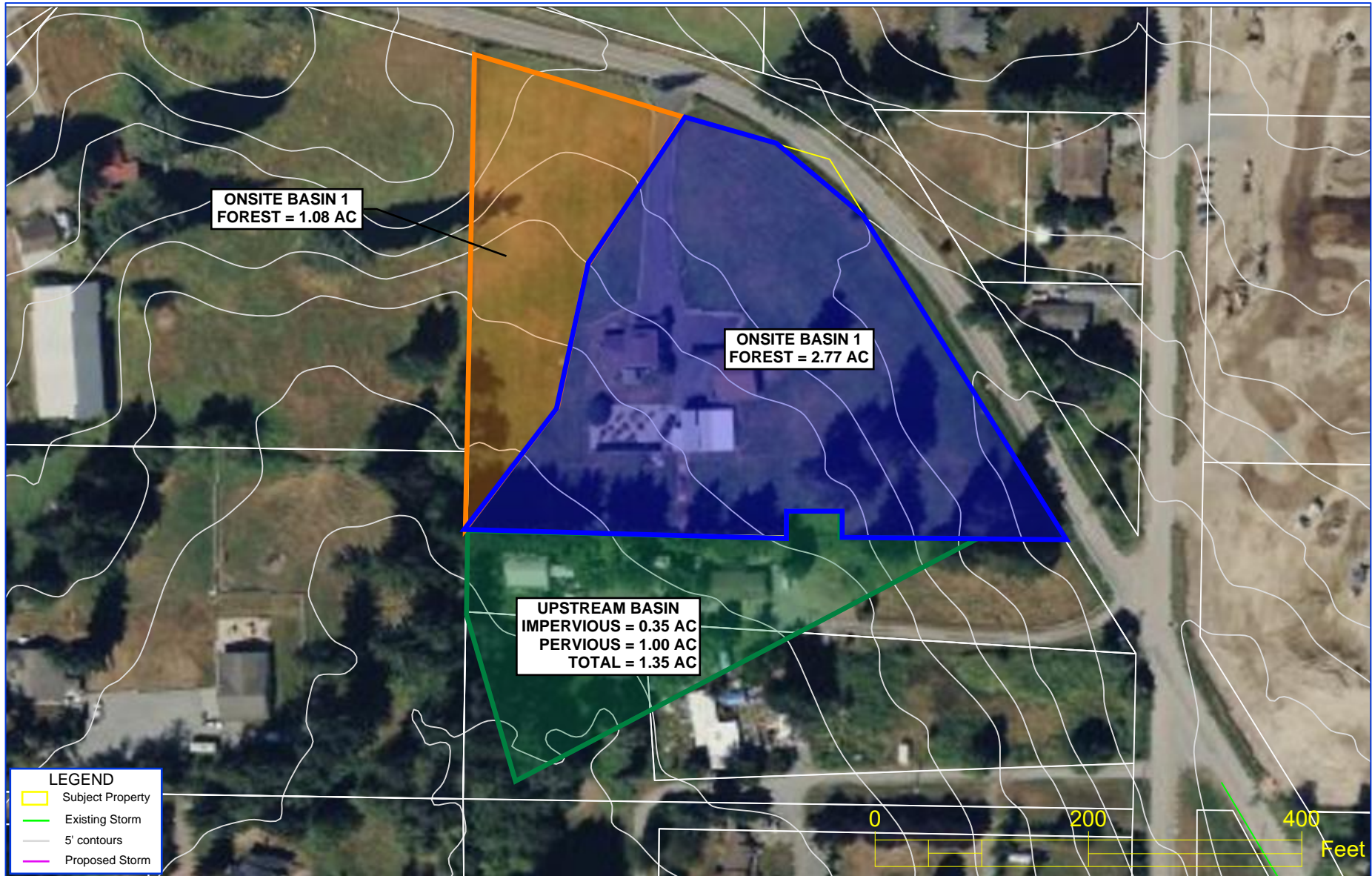
*Percent of map unit:* 2 percent  
*Landform:* Drainageways, depressions  
*Landform position (three-dimensional):* Dip  
*Down-slope shape:* Linear, concave  
*Across-slope shape:* Concave  
*Hydric soil rating:* Yes

## **Data Source Information**

Soil Survey Area: Snohomish County Area, Washington  
Survey Area Data: Version 25, Aug 29, 2023

#### **Appendix 4: Detention and Water Quality Design Analysis**

1. Predeveloped Hydrology Map
2. Developed Hydrology Map
3. WWHM2012 Output – Detention Vault 1
4. WWHM2012 Output – Detention Vault 2
  5. Perfilter 1 Detail
  6. Perfilter 2 Detail
7. Water Quality Design Data Sheet



**Solid Ground Engineering**

8105 166th Ave NE  
Redmond, WA 98052

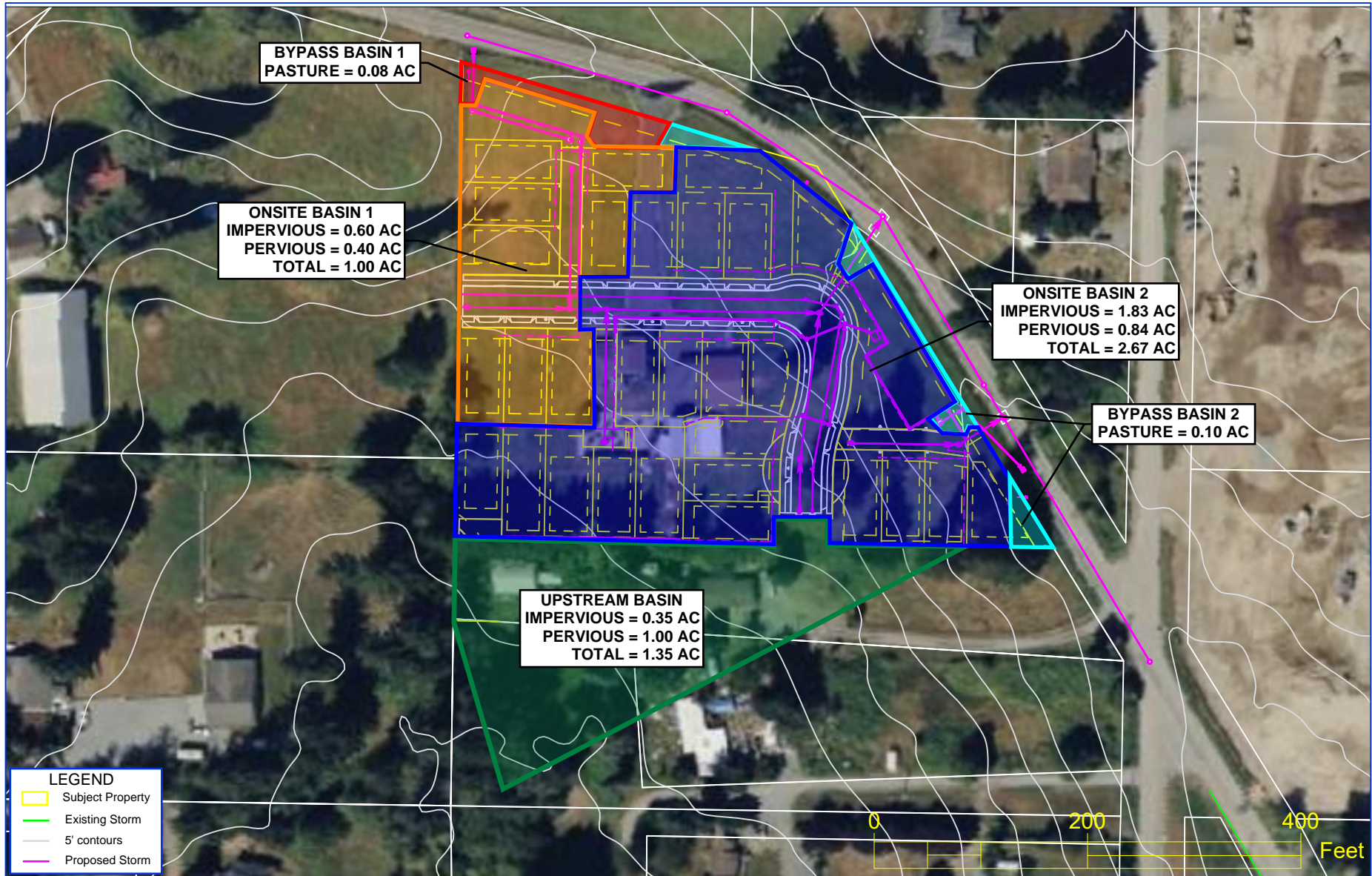
SOUTH LAKE RIDGE, LLC

MINOR PROPERTY

PREDEVELOPED HYDROLOGY MAP

JOB NUMBER:	23-0011	DATE:	2-7-24
JURISDICTION:	Marysville	DRAWN BY:	CJD





**Solid Ground Engineering**

8105 166th Ave NE  
Redmond, WA 98052

SOUTH LAKE RIDGE, LLC

MINOR PROPERTY

DEVELOPED HYDROLOGY MAP

JOB NUMBER:	23-0011	DATE:	2-7-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

**WWHM2012**  
**PROJECT REPORT**

# General Model Information

WWHM2012 Project Name: Minor West Vault

Site Name:

Site Address:

City:

Report Date: 3/12/2024

Gage: Everett

Data Start: 1948/10/01

Data End: 2009/09/30

Timestep: 15 Minute

Precip Scale: 1.200

Version Date: 2023/01/27

Version: 4.2.19

## POC Thresholds

---

Low Flow Threshold for POC1: 50 Percent of the 2 Year

High Flow Threshold for POC1: 50 Year

---

## *Landuse Basin Data*

### *Predeveloped Land Use*

#### Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Forest, Mod	0.9
C, Forest, Steep	0.18
Pervious Total	1.08
Impervious Land Use	acre
Impervious Total	0
Basin Total	1.08

## Mitigated Land Use

### Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Pasture, Flat	0.4
Pervious Total	0.4
Impervious Land Use	acre
ROADS FLAT	0.08
ROOF TOPS FLAT	0.33
DRIVEWAYS FLAT	0.16
SIDEWALKS FLAT	0.03
Impervious Total	0.6
Basin Total	1

## Basin 2

Bypass: Yes

GroundWater: No

Pervious Land Use acre

C, Pasture, Steep 0.04

C, Pasture, Mod 0.04

Pervious Total 0.08

Impervious Land Use acre

Impervious Total 0

Basin Total 0.08

*Routing Elements*  
*Predeveloped Routing*

## Mitigated Routing

### Vault 1

Width: 100 ft.  
 Length: 22 ft.  
 Depth: 7 ft.  
 Discharge Structure  
 Riser Height: 6 ft.  
 Riser Diameter: 12 in.  
 Orifice 1 Diameter: 0.688 in. Elevation:0 ft.  
 Orifice 2 Diameter: 1.156 in. Elevation:3.55 ft.  
 Orifice 3 Diameter: 0.563 in. Elevation:5.25 ft.  
 Element Flows To:  
 Outlet 1                      Outlet 2

Vault Hydraulic Table

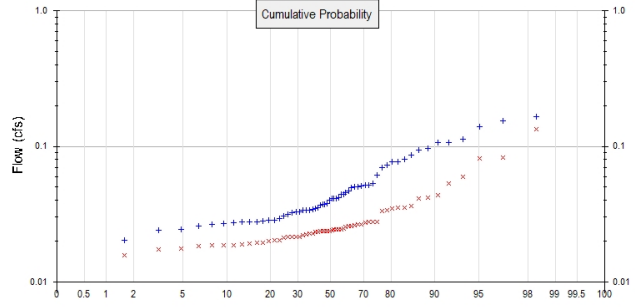
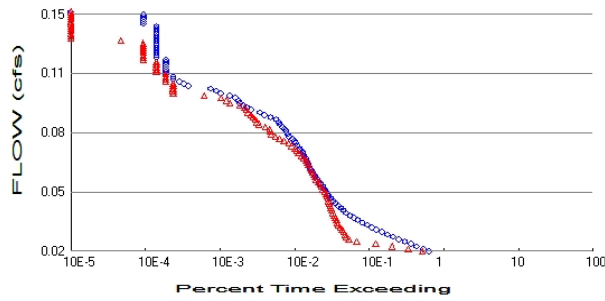
Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
0.0000	0.050	0.000	0.000	0.000
0.0778	0.050	0.003	0.003	0.000
0.1556	0.050	0.007	0.005	0.000
0.2333	0.050	0.011	0.006	0.000
0.3111	0.050	0.015	0.007	0.000
0.3889	0.050	0.019	0.008	0.000
0.4667	0.050	0.023	0.008	0.000
0.5444	0.050	0.027	0.009	0.000
0.6222	0.050	0.031	0.010	0.000
0.7000	0.050	0.035	0.010	0.000
0.7778	0.050	0.039	0.011	0.000
0.8556	0.050	0.043	0.011	0.000
0.9333	0.050	0.047	0.012	0.000
1.0111	0.050	0.051	0.012	0.000
1.0889	0.050	0.055	0.013	0.000
1.1667	0.050	0.058	0.013	0.000
1.2444	0.050	0.062	0.014	0.000
1.3222	0.050	0.066	0.014	0.000
1.4000	0.050	0.070	0.015	0.000
1.4778	0.050	0.074	0.015	0.000
1.5556	0.050	0.078	0.016	0.000
1.6333	0.050	0.082	0.016	0.000
1.7111	0.050	0.086	0.016	0.000
1.7889	0.050	0.090	0.017	0.000
1.8667	0.050	0.094	0.017	0.000
1.9444	0.050	0.098	0.017	0.000
2.0222	0.050	0.102	0.018	0.000
2.1000	0.050	0.106	0.018	0.000
2.1778	0.050	0.110	0.018	0.000
2.2556	0.050	0.113	0.019	0.000
2.3333	0.050	0.117	0.019	0.000
2.4111	0.050	0.121	0.019	0.000
2.4889	0.050	0.125	0.020	0.000
2.5667	0.050	0.129	0.020	0.000
2.6444	0.050	0.133	0.020	0.000
2.7222	0.050	0.137	0.021	0.000
2.8000	0.050	0.141	0.021	0.000
2.8778	0.050	0.145	0.021	0.000



2.9556	0.050	0.149	0.022	0.000
3.0333	0.050	0.153	0.022	0.000
3.1111	0.050	0.157	0.022	0.000
3.1889	0.050	0.161	0.022	0.000
3.2667	0.050	0.165	0.023	0.000
3.3444	0.050	0.168	0.023	0.000
3.4222	0.050	0.172	0.023	0.000
3.5000	0.050	0.176	0.024	0.000
3.5778	0.050	0.180	0.030	0.000
3.6556	0.050	0.184	0.036	0.000
3.7333	0.050	0.188	0.040	0.000
3.8111	0.050	0.192	0.043	0.000
3.8889	0.050	0.196	0.046	0.000
3.9667	0.050	0.200	0.049	0.000
4.0444	0.050	0.204	0.051	0.000
4.1222	0.050	0.208	0.053	0.000
4.2000	0.050	0.212	0.055	0.000
4.2778	0.050	0.216	0.057	0.000
4.3556	0.050	0.220	0.059	0.000
4.4333	0.050	0.223	0.061	0.000
4.5111	0.050	0.227	0.062	0.000
4.5889	0.050	0.231	0.064	0.000
4.6667	0.050	0.235	0.066	0.000
4.7444	0.050	0.239	0.067	0.000
4.8222	0.050	0.243	0.069	0.000
4.9000	0.050	0.247	0.070	0.000
4.9778	0.050	0.251	0.072	0.000
5.0556	0.050	0.255	0.073	0.000
5.1333	0.050	0.259	0.074	0.000
5.2111	0.050	0.263	0.076	0.000
5.2889	0.050	0.267	0.079	0.000
5.3667	0.050	0.271	0.081	0.000
5.4444	0.050	0.275	0.083	0.000
5.5222	0.050	0.278	0.085	0.000
5.6000	0.050	0.282	0.087	0.000
5.6778	0.050	0.286	0.089	0.000
5.7556	0.050	0.290	0.090	0.000
5.8333	0.050	0.294	0.092	0.000
5.9111	0.050	0.298	0.093	0.000
5.9889	0.050	0.302	0.095	0.000
6.0667	0.050	0.306	0.279	0.000
6.1444	0.050	0.310	0.671	0.000
6.2222	0.050	0.314	1.145	0.000
6.3000	0.050	0.318	1.610	0.000
6.3778	0.050	0.322	1.981	0.000
6.4556	0.050	0.326	2.218	0.000
6.5333	0.050	0.330	2.405	0.000
6.6111	0.050	0.333	2.568	0.000
6.6889	0.050	0.337	2.721	0.000
6.7667	0.050	0.341	2.866	0.000
6.8444	0.050	0.345	3.004	0.000
6.9222	0.050	0.349	3.136	0.000
7.0000	0.050	0.353	3.262	0.000
7.0778	0.050	0.357	3.383	0.000
7.1556	0.000	0.000	3.500	0.000

# Analysis Results

## POC 1



+ Predeveloped    x Mitigated

### Predeveloped Landuse Totals for POC #1

Total Pervious Area: 1.08  
 Total Impervious Area: 0

### Mitigated Landuse Totals for POC #1

Total Pervious Area: 0.48  
 Total Impervious Area: 0.6

Flow Frequency Method: Log Pearson Type III 17B

### Flow Frequency Return Periods for Predeveloped. POC #1

Return Period	Flow(cfs)
2 year	0.042903
5 year	0.068847
10 year	0.089544
25 year	0.119954
50 year	0.145867
100 year	0.174716

### Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs)
2 year	0.025093
5 year	0.036383
10 year	0.045643
25 year	0.059644
50 year	0.071934
100 year	0.085989

## Annual Peaks

### Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1949	0.047	0.021
1950	0.049	0.023
1951	0.040	0.019
1952	0.034	0.018
1953	0.028	0.017
1954	0.167	0.033
1955	0.052	0.027
1956	0.045	0.035
1957	0.061	0.028
1958	0.077	0.037

1959	0.041	0.024
1960	0.041	0.025
1961	0.107	0.053
1962	0.042	0.021
1963	0.070	0.024
1964	0.053	0.020
1965	0.033	0.024
1966	0.020	0.019
1967	0.041	0.021
1968	0.050	0.026
1969	0.153	0.041
1970	0.028	0.020
1971	0.052	0.027
1972	0.033	0.023
1973	0.031	0.022
1974	0.086	0.024
1975	0.034	0.019
1976	0.034	0.022
1977	0.025	0.019
1978	0.029	0.019
1979	0.094	0.026
1980	0.044	0.020
1981	0.029	0.020
1982	0.038	0.027
1983	0.077	0.024
1984	0.037	0.042
1985	0.050	0.025
1986	0.113	0.081
1987	0.051	0.059
1988	0.027	0.024
1989	0.034	0.016
1990	0.035	0.026
1991	0.037	0.024
1992	0.028	0.024
1993	0.028	0.018
1994	0.026	0.023
1995	0.037	0.035
1996	0.073	0.028
1997	0.140	0.133
1998	0.024	0.019
1999	0.030	0.024
2000	0.027	0.035
2001	0.010	0.015
2002	0.035	0.024
2003	0.027	0.021
2004	0.046	0.043
2005	0.032	0.024
2006	0.107	0.034
2007	0.081	0.028
2008	0.096	0.083
2009	0.028	0.022

### Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	Mitigated
1	0.1667	0.1330
2	0.1533	0.0828
3	0.1399	0.0812

4	0.1132	0.0594
5	0.1074	0.0530
6	0.1072	0.0433
7	0.0961	0.0420
8	0.0939	0.0415
9	0.0857	0.0365
10	0.0807	0.0354
11	0.0769	0.0351
12	0.0768	0.0346
13	0.0725	0.0338
14	0.0696	0.0334
15	0.0613	0.0278
16	0.0534	0.0277
17	0.0519	0.0277
18	0.0517	0.0273
19	0.0508	0.0265
20	0.0505	0.0265
21	0.0501	0.0262
22	0.0493	0.0257
23	0.0469	0.0257
24	0.0456	0.0253
25	0.0446	0.0248
26	0.0441	0.0244
27	0.0416	0.0244
28	0.0410	0.0243
29	0.0410	0.0243
30	0.0409	0.0239
31	0.0400	0.0238
32	0.0377	0.0238
33	0.0375	0.0237
34	0.0373	0.0235
35	0.0368	0.0235
36	0.0353	0.0235
37	0.0348	0.0232
38	0.0341	0.0227
39	0.0340	0.0226
40	0.0336	0.0224
41	0.0336	0.0222
42	0.0330	0.0216
43	0.0330	0.0215
44	0.0322	0.0214
45	0.0314	0.0214
46	0.0305	0.0211
47	0.0293	0.0202
48	0.0287	0.0202
49	0.0285	0.0200
50	0.0280	0.0195
51	0.0278	0.0193
52	0.0278	0.0191
53	0.0276	0.0188
54	0.0273	0.0187
55	0.0269	0.0186
56	0.0265	0.0185
57	0.0257	0.0184
58	0.0245	0.0176
59	0.0240	0.0174
60	0.0204	0.0158
61	0.0095	0.0152



## Duration Flows

The Facility PASSED

Flow(cfs)	Predev	Mit	Percentage	Pass/Fail
0.0215	13490	10917	80	Pass
0.0227	11214	7048	62	Pass
0.0240	9263	4318	46	Pass
0.0252	7679	2599	33	Pass
0.0265	6352	1451	22	Pass
0.0277	5268	1140	21	Pass
0.0290	4470	1047	23	Pass
0.0302	3769	1001	26	Pass
0.0315	3140	966	30	Pass
0.0328	2674	925	34	Pass
0.0340	2314	871	37	Pass
0.0353	1990	809	40	Pass
0.0365	1713	789	46	Pass
0.0378	1497	774	51	Pass
0.0390	1332	749	56	Pass
0.0403	1171	725	61	Pass
0.0416	1036	698	67	Pass
0.0428	953	672	70	Pass
0.0441	887	655	73	Pass
0.0453	813	640	78	Pass
0.0466	754	623	82	Pass
0.0478	703	609	86	Pass
0.0491	651	594	91	Pass
0.0504	609	578	94	Pass
0.0516	586	560	95	Pass
0.0529	565	535	94	Pass
0.0541	540	502	92	Pass
0.0554	511	484	94	Pass
0.0566	478	467	97	Pass
0.0579	454	445	98	Pass
0.0592	430	418	97	Pass
0.0604	405	397	98	Pass
0.0617	384	383	99	Pass
0.0629	366	369	100	Pass
0.0642	353	356	100	Pass
0.0654	340	342	100	Pass
0.0667	327	328	100	Pass
0.0680	315	315	100	Pass
0.0692	307	301	98	Pass
0.0705	298	285	95	Pass
0.0717	285	263	92	Pass
0.0730	274	239	87	Pass
0.0742	260	226	86	Pass
0.0755	247	213	86	Pass
0.0767	233	195	83	Pass
0.0780	218	178	81	Pass
0.0793	211	154	72	Pass
0.0805	197	134	68	Pass
0.0818	180	118	65	Pass
0.0830	172	103	59	Pass
0.0843	164	101	61	Pass
0.0855	155	99	63	Pass
0.0868	144	86	59	Pass

0.0881	137	71	51	Pass
0.0893	130	66	50	Pass
0.0906	120	63	52	Pass
0.0918	101	59	58	Pass
0.0931	84	55	65	Pass
0.0943	73	51	69	Pass
0.0956	55	48	87	Pass
0.0969	49	45	91	Pass
0.0981	43	39	90	Pass
0.0994	39	29	74	Pass
0.1006	38	26	68	Pass
0.1019	34	22	64	Pass
0.1031	29	13	44	Pass
0.1044	22	5	22	Pass
0.1057	19	5	26	Pass
0.1069	16	5	31	Pass
0.1082	8	5	62	Pass
0.1094	7	5	71	Pass
0.1107	6	4	66	Pass
0.1119	5	4	80	Pass
0.1132	5	4	80	Pass
0.1144	4	4	100	Pass
0.1157	4	3	75	Pass
0.1170	4	3	75	Pass
0.1182	4	3	75	Pass
0.1195	4	3	75	Pass
0.1207	4	3	75	Pass
0.1220	4	2	50	Pass
0.1232	3	2	66	Pass
0.1245	3	2	66	Pass
0.1258	3	2	66	Pass
0.1270	3	2	66	Pass
0.1283	3	2	66	Pass
0.1295	3	2	66	Pass
0.1308	3	2	66	Pass
0.1320	3	1	33	Pass
0.1333	3	0	0	Pass
0.1346	3	0	0	Pass
0.1358	3	0	0	Pass
0.1371	3	0	0	Pass
0.1383	3	0	0	Pass
0.1396	3	0	0	Pass
0.1408	2	0	0	Pass
0.1421	2	0	0	Pass
0.1434	2	0	0	Pass
0.1446	2	0	0	Pass
0.1459	2	0	0	Pass

## Water Quality

Water Quality BMP Flow and Volume for POC #1

On-line facility volume: 0.0833 acre-feet

On-line facility target flow: 0.1103 cfs.

Adjusted for 15 min: 0.1103 cfs.

Off-line facility target flow: 0.0623 cfs.

Adjusted for 15 min: 0.0623 cfs.



# LID Report

LID Technique	Used for Treatment ?	Total Volume Needs Treatment (ac-ft)	Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Vault 1 POC	<input type="checkbox"/>	133.23			<input type="checkbox"/>	0.00			
Total Volume Infiltrated		133.23	0.00	0.00		0.00	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

## *Model Default Modifications*

Total of 0 changes have been made.

### *PERLND Changes*

No PERLND changes have been made.

### *IMPLND Changes*

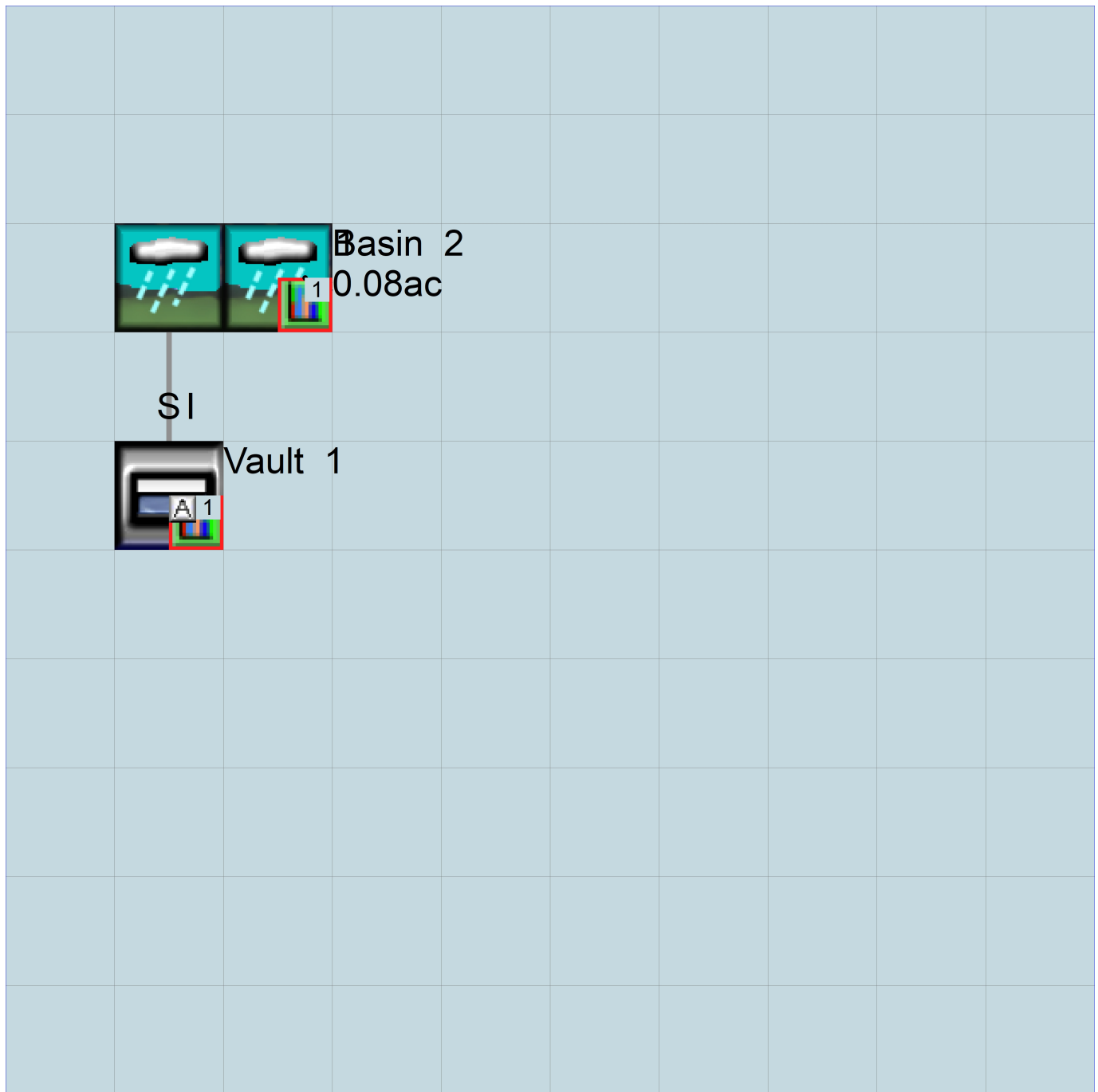
No IMPLND changes have been made.

*Appendix*  
*Predeveloped Schematic*



Basin 1  
1.08ac

Mitigated Schematic



# Predeveloped UCI File

RUN

GLOBAL

WVHM4 model simulation  
START 1948 10 01 END 2009 09 30  
RUN INTERP OUTPUT LEVEL 3 0  
RESUME 0 RUN 1 UNIT SYSTEM 1  
END GLOBAL

FILES

```
<File> <Un#> <-----File Name----->***  
<-ID-> ***  
WDM 26 Minor West Vault.wdm  
MESSU 25 PreMinor West Vault.MES  
27 PreMinor West Vault.L61  
28 PreMinor West Vault.L62  
30 POCMinor West Vault1.dat
```

END FILES

OPN SEQUENCE

INGRP INDELT 00:15  
PERLND 11  
PERLND 12  
COPY 501  
DISPLY 1

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INFO1

```
# - #<-----Title----->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND  
1 Basin 1 MAX 1 2 30 9
```

END DISPLY-INFO1

END DISPLY

COPY

TIMESERIES

```
# - # NPT NMN ***  
1 1 1  
501 1 1
```

END TIMESERIES

END COPY

GENER

OPCODE

```
# # OPCD ***
```

END OPCODE

PARM

```
# # K ***
```

END PARM

END GENER

PERLND

GEN-INFO

```
<PLS ><-----Name----->NBLKS Unit-systems Printer ***  
# - # User t-series Engl Metr ***  
in out ***
```

```
11 C, Forest, Mod 1 1 1 1 27 0  
12 C, Forest, Steep 1 1 1 1 27 0
```

END GEN-INFO

\*\*\* Section PWATER\*\*\*

ACTIVITY

```
<PLS > ***** Active Sections *****  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***  
11 0 0 1 0 0 0 0 0 0 0 0 0  
12 0 0 1 0 0 0 0 0 0 0 0 0
```

END ACTIVITY

PRINT-INFO

```
<PLS > ***** Print-flags ***** PIVL PYR  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
```

```
11      0  0  4  0  0  0  0  0  0  0  0  0  1  9
12      0  0  4  0  0  0  0  0  0  0  0  0  1  9
END PRINT-INFO
```

```
PWAT-PARM1
<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
11      0  0  0  0  0  0  0  0  0  0  0  0
12      0  0  0  0  0  0  0  0  0  0  0  0
END PWAT-PARM1
```

```
PWAT-PARM2
<PLS > PWATER input info: Part 2 ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
11      0  4.5 0.08 400 0.1 0.5 0.996
12      0  4.5 0.08 400 0.15 0.5 0.996
END PWAT-PARM2
```

```
PWAT-PARM3
<PLS > PWATER input info: Part 3 ***
# - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
11      0  0  2  2  0  0  0
12      0  0  2  2  0  0  0
END PWAT-PARM3
```

```
PWAT-PARM4
<PLS > PWATER input info: Part 4 ***
# - # CEPSC UZSN NSUR INTFW IRC LZETP ***
11      0.2 0.5 0.35 6 0.5 0.7
12      0.2 0.3 0.35 6 0.3 0.7
END PWAT-PARM4
```

```
PWAT-STATE1
<PLS > *** Initial conditions at start of simulation
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
# - # *** CEPS SURS UZS IFWS LZS AGWS GWVS
11      0  0  0  0  2.5 1 0
12      0  0  0  0  2.5 1 0
END PWAT-STATE1
```

END PERLND

IMPLND

```
GEN-INFO
<PLS ><-----Name-----> Unit-systems Printer ***
# - # User t-series Engl Metr ***
in out ***
END GEN-INFO
*** Section IWATER***
```

```
ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
END ACTIVITY
```

```
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
END PRINT-INFO
```

```
IWAT-PARM1
<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP VRS VNN RTLI ***
END IWAT-PARM1
```

```
IWAT-PARM2
<PLS > IWATER input info: Part 2 ***
# - # *** LSUR SLSUR NSUR RETSC
END IWAT-PARM2
```

IWAT-PARM3



SPEC-ACTIONS  
 END SPEC-ACTIONS  
 FTABLES  
 END FTABLES

EXT SOURCES

<-Volume->	<Member>	SsysSgap	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***
<Name>	#	<Name>	#	tem strg	<-factor-->	strg	<Name>	# # ***
WDM	2	PREC		ENGL	1.2		PERLND	1 999 EXTNL PREC
WDM	2	PREC		ENGL	1.2		IMPLND	1 999 EXTNL PREC
WDM	1	EVAP		ENGL	0.76		PERLND	1 999 EXTNL PETINP
WDM	1	EVAP		ENGL	0.76		IMPLND	1 999 EXTNL PETINP

END EXT SOURCES

EXT TARGETS

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Volume->	<Member>	Tsys	Tgap	Amd	***	
<Name>	#	<Name>	#	#<-factor-->	strg	<Name>	#	<Name>	tem strg	strg***	
COPY	501	OUTPUT	MEAN	1	1	48.4	WDM	501	FLOW	ENGL	REPL

END EXT TARGETS

MASS-LINK

<Volume>	<-Grp>	<-Member->	<--Mult-->	<Target>	<-Grp>	<-Member->	***
<Name>	#	<Name>	#	#<-factor-->	<Name>	#	#***
MASS-LINK			12				
PERLND	PWATER	SURO		0.083333	COPY	INPUT	MEAN
END MASS-LINK			12				
MASS-LINK			13				
PERLND	PWATER	IFWO		0.083333	COPY	INPUT	MEAN
END MASS-LINK			13				

END MASS-LINK

END RUN



# Mitigated UCI File

RUN

GLOBAL

```
WVHM4 model simulation
START      1948 10 01      END      2009 09 30
RUN INTERP OUTPUT LEVEL   3      0
RESUME     0 RUN         1
UNIT SYSTEM                1
END GLOBAL
```

FILES

```
<File> <Un#> <-----File Name----->***
<-ID->                                     ***
WDM      26    Minor West Vault.wdm
MESSU    25    MitMinor West Vault.MES
          27    MitMinor West Vault.L61
          28    MitMinor West Vault.L62
          30    POCMinor West Vault1.dat
```

END FILES

OPN SEQUENCE

```
INGRP          INDELT 00:15
  PERLND        13
  IMPLND         1
  IMPLND         4
  IMPLND         5
  IMPLND         8
  PERLND        15
  PERLND        14
  RCHRES         1
  COPY           1
  COPY          501
  COPY          601
  DISPLY         1
```

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INF01

```
# - #<-----Title----->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND
1      Vault 1      MAX          1      2      30      9
```

END DISPLY-INF01

END DISPLY

COPY

TIMESERIES

```
# - # NPT NMN ***
1      1      1
501    1      1
601    1      1
```

END TIMESERIES

END COPY

GENER

OPCODE

```
#      # OPCD ***
```

END OPCODE

PARM

```
#      #      K ***
```

END PARM

END GENER

PERLND

GEN-INFO

```
<PLS ><-----Name----->NBLKS Unit-systems Printer ***
# - # User t-series Engl Metr ***
          in out ***
13      C, Pasture, Flat      1      1      1      1      27      0
15      C, Pasture, Steep     1      1      1      1      27      0
14      C, Pasture, Mod       1      1      1      1      27      0
```

END GEN-INFO

\*\*\* Section PWATER\*\*\*

ACTIVITY  
 <PLS > \*\*\*\*\* Active Sections \*\*\*\*\*  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*  
 13 0 0 1 0 0 0 0 0 0 0 0 0  
 15 0 0 1 0 0 0 0 0 0 0 0 0  
 14 0 0 1 0 0 0 0 0 0 0 0 0  
 END ACTIVITY

PRINT-INFO  
 <PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*\*\*  
 13 0 0 4 0 0 0 0 0 0 0 0 0 1 9  
 15 0 0 4 0 0 0 0 0 0 0 0 0 1 9  
 14 0 0 4 0 0 0 0 0 0 0 0 0 1 9  
 END PRINT-INFO

PWAT-PARM1  
 <PLS > PWATER variable monthly parameter value flags \*\*\*  
 # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRG VLE INFC HWT \*\*\*  
 13 0 0 0 0 0 0 0 0 0 0 0  
 15 0 0 0 0 0 0 0 0 0 0 0  
 14 0 0 0 0 0 0 0 0 0 0 0  
 END PWAT-PARM1

PWAT-PARM2  
 <PLS > PWATER input info: Part 2 \*\*\*  
 # - # \*\*\*FOREST LZSN INFILF LSUR SLSUR KVARY AGWRC  
 13 0 4.5 0.06 400 0.05 0.5 0.996  
 15 0 4.5 0.06 400 0.15 0.5 0.996  
 14 0 4.5 0.06 400 0.1 0.5 0.996  
 END PWAT-PARM2

PWAT-PARM3  
 <PLS > PWATER input info: Part 3 \*\*\*  
 # - # \*\*\*PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP  
 13 0 0 2 2 0 0 0  
 15 0 0 2 2 0 0 0  
 14 0 0 2 2 0 0 0  
 END PWAT-PARM3

PWAT-PARM4  
 <PLS > PWATER input info: Part 4 \*\*\*  
 # - # CEPSC UZSN NSUR INTFW IRC LZETP \*\*\*  
 13 0.15 0.4 0.3 6 0.5 0.4  
 15 0.15 0.25 0.3 6 0.3 0.4  
 14 0.15 0.4 0.3 6 0.5 0.4  
 END PWAT-PARM4

PWAT-STATE1  
 <PLS > \*\*\* Initial conditions at start of simulation  
 ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 \*\*\*  
 # - # \*\*\* CEPS SURS UZS IFWS LZS AGWS GWVS  
 13 0 0 0 0 2.5 1 0  
 15 0 0 0 0 2.5 1 0  
 14 0 0 0 0 2.5 1 0  
 END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO  
 <PLS > <-----Name-----> Unit-systems Printer \*\*\*  
 # - # User t-series Engr Metr \*\*\*  
 in out \*\*\*  
 1 ROADS/FLAT 1 1 1 27 0  
 4 ROOF TOPS/FLAT 1 1 1 27 0  
 5 DRIVEWAYS/FLAT 1 1 1 27 0  
 8 SIDEWALKS/FLAT 1 1 1 27 0  
 END GEN-INFO  
 \*\*\* Section IWATER\*\*\*

ACTIVITY  
 <PLS > \*\*\*\*\* Active Sections \*\*\*\*\*  
 # - # ATMP SNOW IWAT SLD IWG IQAL \*\*\*  
 1 0 0 1 0 0 0  
 4 0 0 1 0 0 0  
 5 0 0 1 0 0 0  
 8 0 0 1 0 0 0  
 END ACTIVITY

PRINT-INFO  
 <ILS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR  
 # - # ATMP SNOW IWAT SLD IWG IQAL \*\*\*\*\*  
 1 0 0 4 0 0 4 1 9  
 4 0 0 4 0 0 0 1 9  
 5 0 0 4 0 0 0 1 9  
 8 0 0 4 0 0 0 1 9  
 END PRINT-INFO

IWAT-PARM1  
 <PLS > IWATER variable monthly parameter value flags \*\*\*  
 # - # CSNO RTOP VRS VNN RTLI \*\*\*  
 1 0 0 0 0 0  
 4 0 0 0 0 0  
 5 0 0 0 0 0  
 8 0 0 0 0 0  
 END IWAT-PARM1

IWAT-PARM2  
 <PLS > IWATER input info: Part 2 \*\*\*  
 # - # \*\*\* LSUR SLSUR NSUR RETSC  
 1 400 0.01 0.1 0.1  
 4 400 0.01 0.1 0.1  
 5 400 0.01 0.1 0.1  
 8 400 0.01 0.1 0.1  
 END IWAT-PARM2

IWAT-PARM3  
 <PLS > IWATER input info: Part 3 \*\*\*  
 # - # \*\*\*PETMAX PETMIN  
 1 0 0  
 4 0 0  
 5 0 0  
 8 0 0  
 END IWAT-PARM3

IWAT-STATE1  
 <PLS > \*\*\* Initial conditions at start of simulation  
 # - # \*\*\* RETS SURS  
 1 0 0  
 4 0 0  
 5 0 0  
 8 0 0  
 END IWAT-STATE1

END IMPLND

SCHEMATIC  
 <-Source-> <--Area--> <-Target-> MBLK \*\*\*  
 <Name> # <-factor-> <Name> # Tbl# \*\*\*  
 Basin 1\*\*\*  
 PERLND 13 0.4 RCHRES 1 2  
 PERLND 13 0.4 RCHRES 1 3  
 IMPLND 1 0.08 RCHRES 1 5  
 IMPLND 4 0.33 RCHRES 1 5  
 IMPLND 5 0.16 RCHRES 1 5  
 IMPLND 8 0.03 RCHRES 1 5  
 Basin 2\*\*\*  
 PERLND 15 0.04 COPY 501 12  
 PERLND 15 0.04 COPY 601 12  
 PERLND 15 0.04 COPY 501 13

```

PERLND 15          0.04      COPY    601    13
PERLND 14          0.04      COPY    501    12
PERLND 14          0.04      COPY    601    12
PERLND 14          0.04      COPY    501    13
PERLND 14          0.04      COPY    601    13

```

\*\*\*\*\*Routing\*\*\*\*\*

```

PERLND 13          0.4       COPY    1     12
IMPLND 1           0.08      COPY    1     15
IMPLND 4           0.33      COPY    1     15
IMPLND 5           0.16      COPY    1     15
IMPLND 8           0.03      COPY    1     15
PERLND 13          0.4       COPY    1     13
RCHRES 1           1         COPY    501   16
END SCHEMATIC

```

NETWORK

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1

```

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
END NETWORK

```

RCHRES

```

GEN-INFO
RCHRES          Name          Nexits  Unit Systems  Printer          ***
# - #<-----><----> User T-series Engl Metr LKFG          ***
              in out
1 Vault 1          1 1 1 1 28 0 1          ***
END GEN-INFO
*** Section RCHRES***

```

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFQ PKFG PHFG ***
1 1 1 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR *****
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB PIVL PYR *****
1 4 0 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

HYDR-PARM1

```

RCHRES Flags for each HYDR Section          ***
# - # VC A1 A2 A3 ODFVFG for each *** ODGTFG for each FUNCT for each
      FG FG FG FG possible exit *** possible exit possible exit
      * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
1 0 1 0 0 4 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
END HYDR-PARM1

```

HYDR-PARM2

```

# - # FTABNO          LEN          DELTH          STCOR          KS          DB50          ***
<-----><-----><-----><-----><-----><-----><----->          ***
1 1 0.01          0.0          0.0          0.0          0.5          0.0
END HYDR-PARM2

```

HYDR-INIT

```

RCHRES Initial conditions for each HYDR section          ***
# - # *** VOL          Initial value of COLIND          Initial value of OUTDGT
      *** ac-ft          for each possible exit          for each possible exit
<-----><----->          <-----><-----><-----><-----><----->          *** <-----><-----><-----><-----><----->
1 0          4.0 0.0 0.0 0.0 0.0          0.0 0.0 0.0 0.0 0.0
END HYDR-INIT

```

END RCHRES

SPEC-ACTIONS  
 END SPEC-ACTIONS  
 FTTABLES

FTTABLE 1  
 92 4

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflowl (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.050505	0.000000	0.000000		
0.077778	0.050505	0.003928	0.003577		
0.155556	0.050505	0.007856	0.005059		
0.233333	0.050505	0.011785	0.006196		
0.311111	0.050505	0.015713	0.007154		
0.388889	0.050505	0.019641	0.007999		
0.466667	0.050505	0.023569	0.008762		
0.544444	0.050505	0.027497	0.009464		
0.622222	0.050505	0.031425	0.010118		
0.700000	0.050505	0.035354	0.010731		
0.777778	0.050505	0.039282	0.011312		
0.855556	0.050505	0.043210	0.011864		
0.933333	0.050505	0.047138	0.012391		
1.011111	0.050505	0.051066	0.012897		
1.088889	0.050505	0.054994	0.013384		
1.166667	0.050505	0.058923	0.013854		
1.244444	0.050505	0.062851	0.014308		
1.322222	0.050505	0.066779	0.014749		
1.400000	0.050505	0.070707	0.015176		
1.477778	0.050505	0.074635	0.015592		
1.555556	0.050505	0.078563	0.015997		
1.633333	0.050505	0.082492	0.016392		
1.711111	0.050505	0.086420	0.016778		
1.788889	0.050505	0.090348	0.017155		
1.866667	0.050505	0.094276	0.017524		
1.944444	0.050505	0.098204	0.017886		
2.022222	0.050505	0.102132	0.018240		
2.100000	0.050505	0.106061	0.018587		
2.177778	0.050505	0.109989	0.018928		
2.255556	0.050505	0.113917	0.019263		
2.333333	0.050505	0.117845	0.019593		
2.411111	0.050505	0.121773	0.019916		
2.488889	0.050505	0.125701	0.020235		
2.566667	0.050505	0.129630	0.020549		
2.644444	0.050505	0.133558	0.020858		
2.722222	0.050505	0.137486	0.021162		
2.800000	0.050505	0.141414	0.021463		
2.877778	0.050505	0.145342	0.021759		
2.955556	0.050505	0.149270	0.022051		
3.033333	0.050505	0.153199	0.022339		
3.111111	0.050505	0.157127	0.022624		
3.188889	0.050505	0.161055	0.022905		
3.266667	0.050505	0.164983	0.023182		
3.344444	0.050505	0.168911	0.023457		
3.422222	0.050505	0.172840	0.023728		
3.500000	0.050505	0.176768	0.023996		
3.577778	0.050505	0.180696	0.030308		
3.655556	0.050505	0.184624	0.036310		
3.733333	0.050505	0.188552	0.040317		
3.811111	0.050505	0.192480	0.043578		
3.888889	0.050505	0.196409	0.046414		
3.966667	0.050505	0.200337	0.048964		
4.044444	0.050505	0.204265	0.051306		
4.122222	0.050505	0.208193	0.053486		
4.200000	0.050505	0.212121	0.055536		
4.277778	0.050505	0.216049	0.057479		
4.355556	0.050505	0.219978	0.059331		
4.433333	0.050505	0.223906	0.061104		
4.511111	0.050505	0.227834	0.062810		
4.588889	0.050505	0.231762	0.064455		
4.666667	0.050505	0.235690	0.066046		
4.744444	0.050505	0.239618	0.067588		
4.822222	0.050505	0.243547	0.069087		

4.900000	0.050505	0.247475	0.070545
4.977778	0.050505	0.251403	0.071967
5.055556	0.050505	0.255331	0.073355
5.133333	0.050505	0.259259	0.074711
5.211111	0.050505	0.263187	0.076038
5.288889	0.050505	0.267116	0.079032
5.366667	0.050505	0.271044	0.081545
5.444444	0.050505	0.274972	0.083649
5.522222	0.050505	0.278900	0.085571
5.600000	0.050505	0.282828	0.087377
5.677778	0.050505	0.286756	0.089099
5.755556	0.050505	0.290685	0.090756
5.833333	0.050505	0.294613	0.092358
5.911111	0.050505	0.298541	0.093913
5.988889	0.050505	0.302469	0.095427
6.066667	0.050505	0.306397	0.279140
6.144444	0.050505	0.310325	0.670994
6.222222	0.050505	0.314254	1.145797
6.300000	0.050505	0.318182	1.610827
6.377778	0.050505	0.322110	1.981788
6.455556	0.050505	0.326038	2.218085
6.533333	0.050505	0.329966	2.405339
6.611111	0.050505	0.333895	2.568651
6.688889	0.050505	0.337823	2.721920
6.766667	0.050505	0.341751	2.866807
6.844444	0.050505	0.345679	3.004558
6.922222	0.050505	0.349607	3.136137
7.000000	0.050505	0.353535	3.262310
7.077778	0.050505	0.357464	3.383696

END FTABLE 1

END FTABLES

EXT SOURCES

<-Volume->	<Member>	SsysSgap<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***	
<Name>	#	<Name>	#	tem strg<-factor->	strg	<Name>	# #	***
WDM	2	PREC	ENGL	1.2		PERLND	1 999	EXTNL PREC
WDM	2	PREC	ENGL	1.2		IMPLND	1 999	EXTNL PREC
WDM	1	EVAP	ENGL	0.76		PERLND	1 999	EXTNL PETINP
WDM	1	EVAP	ENGL	0.76		IMPLND	1 999	EXTNL PETINP

END EXT SOURCES

EXT TARGETS

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Volume->	<Member>	Tsys	Tgap	Amd	***	
<Name>	#	<Name>	#	#<-factor->	strg	<Name>	#	<Name>	tem strg	strg***	
RCHRES	1	HYDR	RO	1	1	WDM	1000	FLOW	ENGL	REPL	
RCHRES	1	HYDR	STAGE	1	1	WDM	1001	STAG	ENGL	REPL	
COPY	1	OUTPUT	MEAN	1	1	48.4	WDM	701	FLOW	ENGL	REPL
COPY	501	OUTPUT	MEAN	1	1	48.4	WDM	801	FLOW	ENGL	REPL
COPY	601	OUTPUT	MEAN	1	1	48.4	WDM	901	FLOW	ENGL	REPL

END EXT TARGETS

MASS-LINK

<Volume>	<-Grp>	<-Member->	<--Mult-->	<Target>	<-Grp>	<-Member->	***	
<Name>	#	<Name>	#	#<-factor->	<Name>	#	#	***
MASS-LINK			2					
PERLND	PWATER	SURO		0.083333	RCHRES	INFLOW	IVOL	
END MASS-LINK			2					
MASS-LINK			3					
PERLND	PWATER	IFWO		0.083333	RCHRES	INFLOW	IVOL	
END MASS-LINK			3					
MASS-LINK			5					
IMPLND	IWATER	SURO		0.083333	RCHRES	INFLOW	IVOL	
END MASS-LINK			5					
MASS-LINK			12					
PERLND	PWATER	SURO		0.083333	COPY	INPUT	MEAN	
END MASS-LINK			12					

```

    MASS-LINK          13
  PERLND    PWATER  IFWO      0.083333    COPY      INPUT  MEAN
    END MASS-LINK      13

    MASS-LINK          15
  IMPLND    IWATER  SURO      0.083333    COPY      INPUT  MEAN
    END MASS-LINK      15

    MASS-LINK          16
  RCHRES    ROFLOW              COPY      INPUT  MEAN
    END MASS-LINK      16

```

END MASS-LINK

END RUN

**WWHM2012**  
**PROJECT REPORT**



## General Model Information

WWHM2012 Project Name: Minor East w\_Upstream

Site Name:

Site Address:

City:

Report Date: 3/26/2024

Gage: Everett

Data Start: 1948/10/01

Data End: 2009/09/30

Timestep: 15 Minute

Precip Scale: 1.200

Version Date: 2023/01/27

Version: 4.2.19

## POC Thresholds

---

Low Flow Threshold for POC1: 50 Percent of the 2 Year

High Flow Threshold for POC1: 50 Year

---

# *Landuse Basin Data*

## *Predeveloped Land Use*

### Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Forest, Mod	2
C, Forest, Steep	0.77
Pervious Total	2.77
Impervious Land Use	acre
Impervious Total	0
Basin Total	2.77

## Upstream

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Lawn, Mod	0.2
C, Forest, Mod	0.8
Pervious Total	1
Impervious Land Use	acre
ROOF TOPS FLAT	0.12
DRIVEWAYS FLAT	0.23
Impervious Total	0.35
Basin Total	1.35

## *Mitigated Land Use*

### Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Pasture, Flat	0.84
Pervious Total	0.84
Impervious Land Use	acre
ROADS FLAT	0.29
ROOF TOPS FLAT	1
DRIVEWAYS FLAT	0.45
SIDEWALKS FLAT	0.09
Impervious Total	1.83
Basin Total	2.67

## Bypass

Bypass: Yes

GroundWater: No

Pervious Land Use acre

C, Pasture, Mod 0.06

C, Pasture, Steep 0.04

Pervious Total 0.1

Impervious Land Use acre

Impervious Total 0

Basin Total 0.1

## Upstream

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
C, Lawn, Mod	0.2
C, Forest, Mod	0.8
Pervious Total	1
Impervious Land Use	acre
ROOF TOPS FLAT	0.12
DRIVEWAYS FLAT	0.23
Impervious Total	0.35
Basin Total	1.35

*Routing Elements*  
*Predeveloped Routing*

## Mitigated Routing

### Vault 1

Width: 108 ft.  
 Length: 46 ft.  
 Depth: 7.5 ft.  
 Discharge Structure  
 Riser Height: 6.5 ft.  
 Riser Diameter: 18 in.  
 Notch Type: Rectangular  
 Notch Width: 0.100 ft.  
 Notch Height: 0.800 ft.  
 Orifice 1 Diameter: 1.938 in. Elevation:0 ft.  
 Orifice 2 Diameter: 1.656 in. Elevation:3 ft.  
 Orifice 3 Diameter: 2.000 in. Elevation:6 ft.  
 Element Flows To:  
 Outlet 1                      Outlet 2

Vault Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
0.0000	0.114	0.000	0.000	0.000
0.0833	0.114	0.009	0.029	0.000
0.1667	0.114	0.019	0.041	0.000
0.2500	0.114	0.028	0.050	0.000
0.3333	0.114	0.038	0.058	0.000
0.4167	0.114	0.047	0.065	0.000
0.5000	0.114	0.057	0.072	0.000
0.5833	0.114	0.066	0.077	0.000
0.6667	0.114	0.076	0.083	0.000
0.7500	0.114	0.085	0.088	0.000
0.8333	0.114	0.095	0.093	0.000
0.9167	0.114	0.104	0.097	0.000
1.0000	0.114	0.114	0.101	0.000
1.0833	0.114	0.123	0.106	0.000
1.1667	0.114	0.133	0.110	0.000
1.2500	0.114	0.142	0.113	0.000
1.3333	0.114	0.152	0.117	0.000
1.4167	0.114	0.161	0.121	0.000
1.5000	0.114	0.171	0.124	0.000
1.5833	0.114	0.180	0.128	0.000
1.6667	0.114	0.190	0.131	0.000
1.7500	0.114	0.199	0.134	0.000
1.8333	0.114	0.209	0.137	0.000
1.9167	0.114	0.218	0.141	0.000
2.0000	0.114	0.228	0.144	0.000
2.0833	0.114	0.237	0.147	0.000
2.1667	0.114	0.247	0.149	0.000
2.2500	0.114	0.256	0.152	0.000
2.3333	0.114	0.266	0.155	0.000
2.4167	0.114	0.275	0.158	0.000
2.5000	0.114	0.285	0.161	0.000
2.5833	0.114	0.294	0.163	0.000
2.6667	0.114	0.304	0.166	0.000
2.7500	0.114	0.313	0.168	0.000
2.8333	0.114	0.323	0.171	0.000

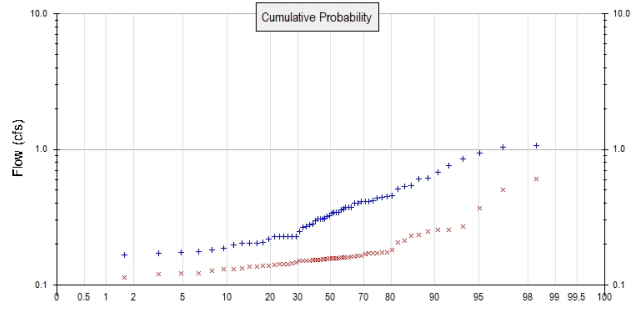
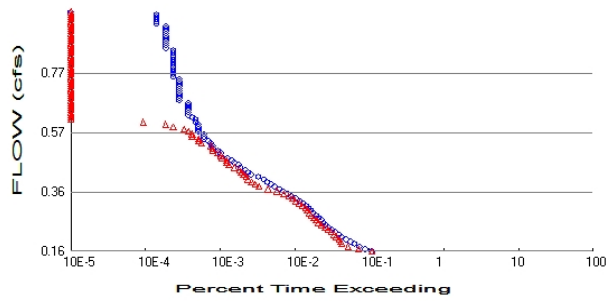


2.9167	0.114	0.332	0.174	0.000
3.0000	0.114	0.342	0.176	0.000
3.0833	0.114	0.351	0.200	0.000
3.1667	0.114	0.361	0.211	0.000
3.2500	0.114	0.370	0.220	0.000
3.3333	0.114	0.380	0.229	0.000
3.4167	0.114	0.389	0.236	0.000
3.5000	0.114	0.399	0.243	0.000
3.5833	0.114	0.408	0.249	0.000
3.6667	0.114	0.418	0.255	0.000
3.7500	0.114	0.427	0.261	0.000
3.8333	0.114	0.437	0.267	0.000
3.9167	0.114	0.446	0.272	0.000
4.0000	0.114	0.456	0.278	0.000
4.0833	0.114	0.465	0.283	0.000
4.1667	0.114	0.475	0.288	0.000
4.2500	0.114	0.484	0.293	0.000
4.3333	0.114	0.494	0.298	0.000
4.4167	0.114	0.503	0.302	0.000
4.5000	0.114	0.513	0.307	0.000
4.5833	0.114	0.522	0.311	0.000
4.6667	0.114	0.532	0.316	0.000
4.7500	0.114	0.541	0.320	0.000
4.8333	0.114	0.551	0.324	0.000
4.9167	0.114	0.560	0.328	0.000
5.0000	0.114	0.570	0.333	0.000
5.0833	0.114	0.579	0.337	0.000
5.1667	0.114	0.589	0.341	0.000
5.2500	0.114	0.598	0.345	0.000
5.3333	0.114	0.608	0.349	0.000
5.4167	0.114	0.617	0.352	0.000
5.5000	0.114	0.627	0.356	0.000
5.5833	0.114	0.636	0.360	0.000
5.6667	0.114	0.646	0.364	0.000
5.7500	0.114	0.655	0.371	0.000
5.8333	0.114	0.665	0.387	0.000
5.9167	0.114	0.674	0.407	0.000
6.0000	0.114	0.684	0.429	0.000
6.0833	0.114	0.693	0.486	0.000
6.1667	0.114	0.703	0.526	0.000
6.2500	0.114	0.712	0.564	0.000
6.3333	0.114	0.722	0.601	0.000
6.4167	0.114	0.731	0.638	0.000
6.5000	0.114	0.741	0.675	0.000
6.5833	0.114	0.750	1.067	0.000
6.6667	0.114	0.760	1.768	0.000
6.7500	0.114	0.769	2.641	0.000
6.8333	0.114	0.779	3.593	0.000
6.9167	0.114	0.788	4.531	0.000
7.0000	0.114	0.798	5.366	0.000
7.0833	0.114	0.807	6.028	0.000
7.1667	0.114	0.817	6.496	0.000
7.2500	0.114	0.826	6.820	0.000
7.3333	0.114	0.836	7.225	0.000
7.4167	0.114	0.845	7.548	0.000
7.5000	0.114	0.855	7.856	0.000
7.5833	0.114	0.864	8.152	0.000
7.6667	0.000	0.000	8.437	0.000



# Analysis Results

## POC 1



+ Predeveloped    x Mitigated

### Predeveloped Landuse Totals for POC #1

Total Pervious Area: 3.77  
 Total Impervious Area: 0.35

### Mitigated Landuse Totals for POC #1

Total Pervious Area: 1.94  
 Total Impervious Area: 2.18

Flow Frequency Method: Log Pearson Type III 17B

### Flow Frequency Return Periods for Predeveloped. POC #1

Return Period	Flow(cfs)
2 year	0.3216
5 year	0.489468
10 year	0.621004
25 year	0.811994
50 year	0.973321
100 year	1.151877

### Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs)
2 year	0.160859
5 year	0.21374
10 year	0.25423
25 year	0.311982
50 year	0.360068
100 year	0.412746

## Annual Peaks

### Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1949	0.412	0.151
1950	0.443	0.159
1951	0.268	0.150
1952	0.309	0.127
1953	0.365	0.121
1954	0.936	0.156
1955	0.412	0.171
1956	0.247	0.173
1957	0.412	0.213
1958	0.851	0.159

1959	0.280	0.158
1960	0.358	0.174
1961	1.066	0.171
1962	0.317	0.155
1963	0.541	0.156
1964	0.308	0.150
1965	0.167	0.152
1966	0.182	0.122
1967	0.375	0.160
1968	0.344	0.234
1969	1.032	0.141
1970	0.228	0.131
1971	0.372	0.157
1972	0.399	0.161
1973	0.323	0.137
1974	0.529	0.139
1975	0.374	0.131
1976	0.203	0.163
1977	0.187	0.142
1978	0.217	0.121
1979	0.615	0.206
1980	0.304	0.143
1981	0.227	0.147
1982	0.229	0.248
1983	0.418	0.151
1984	0.267	0.172
1985	0.343	0.181
1986	0.673	0.366
1987	0.296	0.256
1988	0.276	0.157
1989	0.336	0.112
1990	0.226	0.158
1991	0.206	0.164
1992	0.304	0.136
1993	0.227	0.138
1994	0.171	0.153
1995	0.198	0.168
1996	0.446	0.231
1997	0.755	0.606
1998	0.341	0.133
1999	0.204	0.153
2000	0.453	0.161
2001	0.136	0.113
2002	0.177	0.156
2003	0.173	0.142
2004	0.398	0.253
2005	0.202	0.158
2006	0.602	0.269
2007	0.507	0.154
2008	0.436	0.503
2009	0.228	0.145

### Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	Mitigated
1	1.0661	0.6064
2	1.0323	0.5027
3	0.9355	0.3663

4	0.8509	0.2686
5	0.7550	0.2559
6	0.6734	0.2529
7	0.6154	0.2475
8	0.6022	0.2338
9	0.5411	0.2311
10	0.5293	0.2128
11	0.5072	0.2063
12	0.4530	0.1806
13	0.4465	0.1740
14	0.4427	0.1726
15	0.4358	0.1723
16	0.4180	0.1712
17	0.4124	0.1709
18	0.4123	0.1678
19	0.4123	0.1643
20	0.3990	0.1633
21	0.3981	0.1607
22	0.3750	0.1607
23	0.3736	0.1599
24	0.3721	0.1588
25	0.3648	0.1585
26	0.3577	0.1584
27	0.3440	0.1578
28	0.3432	0.1576
29	0.3410	0.1571
30	0.3360	0.1571
31	0.3228	0.1563
32	0.3172	0.1557
33	0.3091	0.1555
34	0.3082	0.1548
35	0.3044	0.1538
36	0.3040	0.1533
37	0.2959	0.1531
38	0.2798	0.1522
39	0.2757	0.1509
40	0.2681	0.1506
41	0.2673	0.1504
42	0.2471	0.1498
43	0.2287	0.1471
44	0.2284	0.1451
45	0.2282	0.1432
46	0.2270	0.1421
47	0.2268	0.1420
48	0.2265	0.1409
49	0.2172	0.1391
50	0.2063	0.1378
51	0.2039	0.1372
52	0.2032	0.1358
53	0.2023	0.1333
54	0.1976	0.1314
55	0.1870	0.1310
56	0.1819	0.1272
57	0.1769	0.1223
58	0.1733	0.1209
59	0.1709	0.1206
60	0.1668	0.1135
61	0.1359	0.1125

## Duration Flows

The Facility PASSED

Flow(cfs)	Predev	Mit	Percentage	Pass/Fail
0.1608	2329	2306	99	Pass
0.1690	1894	1537	81	Pass
0.1772	1583	1105	69	Pass
0.1854	1356	916	67	Pass
0.1936	1201	878	73	Pass
0.2018	1048	841	80	Pass
0.2100	914	769	84	Pass
0.2183	829	716	86	Pass
0.2265	737	660	89	Pass
0.2347	660	593	89	Pass
0.2429	600	552	92	Pass
0.2511	554	483	87	Pass
0.2593	519	446	85	Pass
0.2675	484	417	86	Pass
0.2757	446	387	86	Pass
0.2839	416	356	85	Pass
0.2921	377	335	88	Pass
0.3003	352	316	89	Pass
0.3085	329	297	90	Pass
0.3167	305	269	88	Pass
0.3249	276	245	88	Pass
0.3332	247	219	88	Pass
0.3414	219	199	90	Pass
0.3496	190	177	93	Pass
0.3578	173	151	87	Pass
0.3660	150	124	82	Pass
0.3742	134	94	70	Pass
0.3824	114	71	62	Pass
0.3906	102	63	61	Pass
0.3988	91	56	61	Pass
0.4070	82	51	62	Pass
0.4152	69	48	69	Pass
0.4234	55	44	80	Pass
0.4316	51	41	80	Pass
0.4398	46	39	84	Pass
0.4481	40	33	82	Pass
0.4563	38	28	73	Pass
0.4645	33	26	78	Pass
0.4727	31	26	83	Pass
0.4809	27	23	85	Pass
0.4891	22	22	100	Pass
0.4973	22	20	90	Pass
0.5055	20	17	85	Pass
0.5137	17	17	100	Pass
0.5219	17	16	94	Pass
0.5301	16	12	75	Pass
0.5383	15	11	73	Pass
0.5465	13	11	84	Pass
0.5547	13	9	69	Pass
0.5630	13	9	69	Pass
0.5712	11	8	72	Pass
0.5794	11	7	63	Pass
0.5876	11	5	45	Pass

0.5958	11	4	36	Pass
0.6040	10	2	20	Pass
0.6122	10	0	0	Pass
0.6204	9	0	0	Pass
0.6286	8	0	0	Pass
0.6368	8	0	0	Pass
0.6450	8	0	0	Pass
0.6532	8	0	0	Pass
0.6614	8	0	0	Pass
0.6697	8	0	0	Pass
0.6779	6	0	0	Pass
0.6861	6	0	0	Pass
0.6943	6	0	0	Pass
0.7025	6	0	0	Pass
0.7107	6	0	0	Pass
0.7189	6	0	0	Pass
0.7271	6	0	0	Pass
0.7353	6	0	0	Pass
0.7435	6	0	0	Pass
0.7517	6	0	0	Pass
0.7599	5	0	0	Pass
0.7681	5	0	0	Pass
0.7763	5	0	0	Pass
0.7846	5	0	0	Pass
0.7928	5	0	0	Pass
0.8010	5	0	0	Pass
0.8092	5	0	0	Pass
0.8174	5	0	0	Pass
0.8256	5	0	0	Pass
0.8338	5	0	0	Pass
0.8420	5	0	0	Pass
0.8502	5	0	0	Pass
0.8584	4	0	0	Pass
0.8666	4	0	0	Pass
0.8748	4	0	0	Pass
0.8830	4	0	0	Pass
0.8912	4	0	0	Pass
0.8995	4	0	0	Pass
0.9077	4	0	0	Pass
0.9159	4	0	0	Pass
0.9241	4	0	0	Pass
0.9323	4	0	0	Pass
0.9405	3	0	0	Pass
0.9487	3	0	0	Pass
0.9569	3	0	0	Pass
0.9651	3	0	0	Pass
0.9733	3	0	0	Pass

## Water Quality

Water Quality BMP Flow and Volume for POC #1

On-line facility volume: 0.313 acre-feet

On-line facility target flow: 0.4129 cfs.

Adjusted for 15 min: 0.4129 cfs.

Off-line facility target flow: 0.2332 cfs.

Adjusted for 15 min: 0.2332 cfs.



# LID Report

LID Technique	Used for Treatment ?	Total Volume Needs Treatment (ac-ft)	Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Vault 1 POC	<input type="checkbox"/>	505.37			<input type="checkbox"/>	0.00			
Total Volume Infiltrated		505.37	0.00	0.00		0.00	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

## *Model Default Modifications*

Total of 0 changes have been made.

### *PERLND Changes*

No PERLND changes have been made.

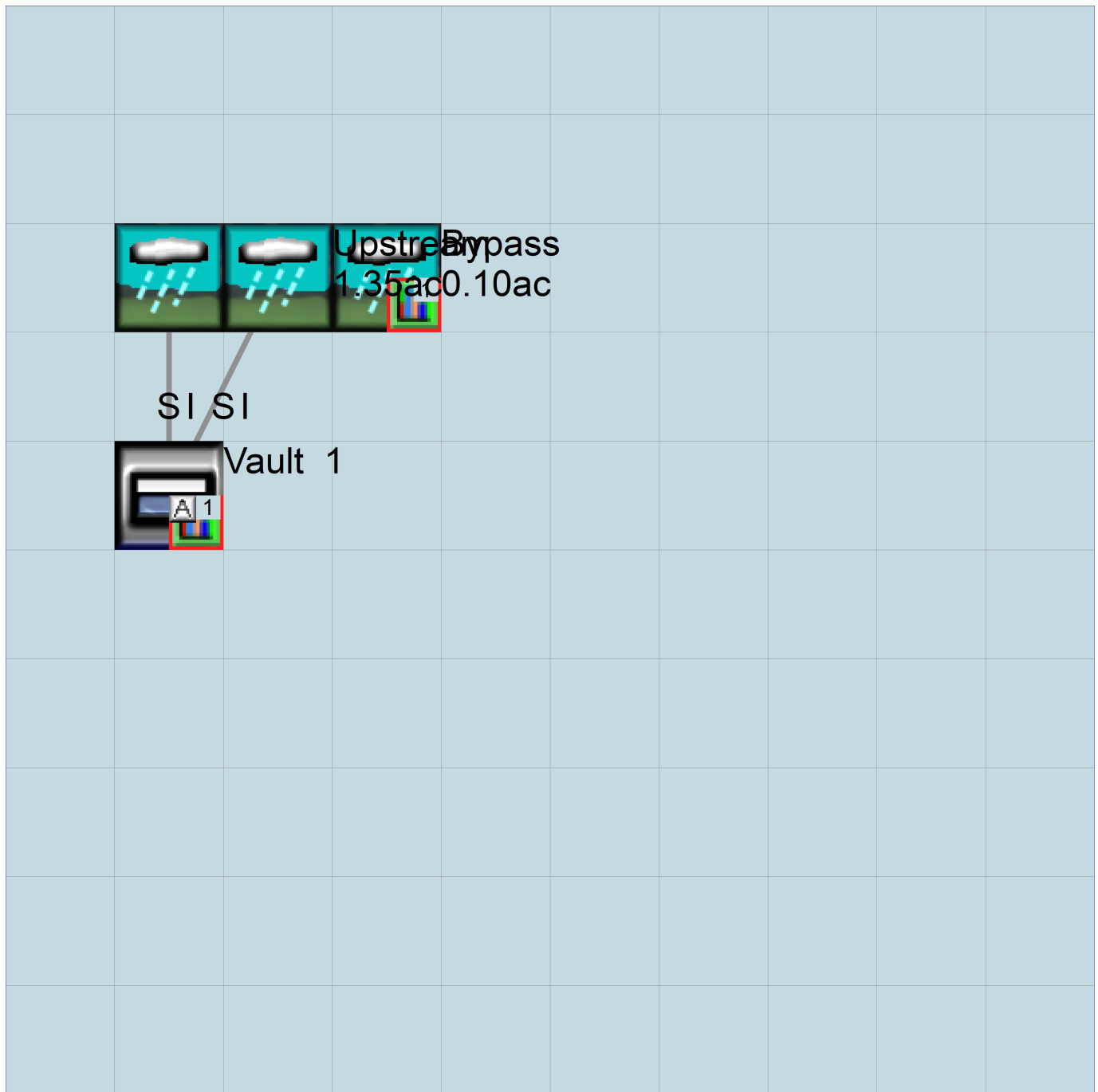
### *IMPLND Changes*

No IMPLND changes have been made.

Appendix  
Predeveloped Schematic



Mitigated Schematic



# Predeveloped UCI File

RUN

```
GLOBAL
  WWHM4 model simulation
  START      1948 10 01      END      2009 09 30
  RUN INTERP OUTPUT LEVEL   3      0
  RESUME     0 RUN         1
  UNIT SYSTEM 1
```

```
FILES
<File> <Un#> <-----File Name----->***
<-ID->                                     ***
WDM      26      Minor East w_Upstream.wdm
MESSU    25      PreMinor East w_Upstream.MES
          27      PreMinor East w_Upstream.L61
          28      PreMinor East w_Upstream.L62
          30      POCMinor East w_Upstream1.dat
END FILES
```

```
OPN SEQUENCE
  INGRP          INDELT 00:15
  PERLND         11
  PERLND         12
  PERLND         17
  IMPLND         4
  IMPLND         5
  COPY           501
  DISPLY         1
  END INGRP
```

END OPN SEQUENCE

```
DISPLY
  DISPLY-INFO1
  # - #<-----Title----->***TRAN PIVL DIG1 FIL1  PYR DIG2 FIL2 YRND
  1   1   Basin 1          MAX          1   2   30   9
  END DISPLY-INFO1
```

END DISPLY

```
COPY
  TIMESERIES
  # - # NPT NMN ***
  1   1   1   1
  501 1   1   1
  END TIMESERIES
```

END COPY

```
GENER
  OPCODE
  #   # OPCD ***
  END OPCODE
  PARM
  #   #           K ***
  END PARM
```

END GENER

```
PERLND
  GEN-INFO
  <PLS ><-----Name----->NBLKS  Unit-systems  Printer ***
  # - #           User  t-series  Engl Metr ***
  #           in  out
  11      C, Forest, Mod      1   1   1   1   27   0
  12      C, Forest, Steep    1   1   1   1   27   0
  17      C, Lawn, Mod        1   1   1   1   27   0
  END GEN-INFO
  *** Section PWATER***
```

```
ACTIVITY
  <PLS > ***** Active Sections *****
  # - # ATMP SNOW PWAT  SED  PST  PWG  PQAL  MSTL  PEST  NITR  PHOS  TRAC  ***
  11      0   0   1   0   0   0   0   0   0   0   0   0
  12      0   0   1   0   0   0   0   0   0   0   0   0
  17      0   0   1   0   0   0   0   0   0   0   0   0
```

END ACTIVITY

PRINT-INFO

<PLS > ***** Print-flags *****														PIVL	PYR	
#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	*****	
11			0	0	4	0	0	0	0	0	0	0	0	0	1	9
12			0	0	4	0	0	0	0	0	0	0	0	0	1	9
17			0	0	4	0	0	0	0	0	0	0	0	0	1	9

END PRINT-INFO

PWAT-PARM1

<PLS > PWATER variable monthly parameter value flags ***														
#	-	#	CSNO	RTOP	UZFG	VCS	VUZ	VNN	VIFW	VIRC	VLE	INFC	HWT	***
11			0	0	0	0	0	0	0	0	0	0	0	
12			0	0	0	0	0	0	0	0	0	0	0	
17			0	0	0	0	0	0	0	0	0	0	0	

END PWAT-PARM1

PWAT-PARM2

<PLS > PWATER input info: Part 2 ***									
#	-	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
11			0	4.5	0.08	400	0.1	0.5	0.996
12			0	4.5	0.08	400	0.15	0.5	0.996
17			0	4.5	0.03	400	0.1	0.5	0.996

END PWAT-PARM2

PWAT-PARM3

<PLS > PWATER input info: Part 3 ***									
#	-	#	***PETMAX	PETMIN	INFEXP	INFILD	DEEPPFR	BASETP	AGWETP
11			0	0	2	2	0	0	0
12			0	0	2	2	0	0	0
17			0	0	2	2	0	0	0

END PWAT-PARM3

PWAT-PARM4

<PLS > PWATER input info: Part 4 ***									
#	-	#	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP	***
11			0.2	0.5	0.35	6	0.5	0.7	
12			0.2	0.3	0.35	6	0.3	0.7	
17			0.1	0.25	0.25	6	0.5	0.25	

END PWAT-PARM4

PWAT-STATE1

<PLS > *** Initial conditions at start of simulation									
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***									
#	-	#	*** CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
11			0	0	0	0	2.5	1	0
12			0	0	0	0	2.5	1	0
17			0	0	0	0	2.5	1	0

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<PLS > <-----Name-----> Unit-systems Printer ***							
#	-	#	User	t-series	Engl	Metr	***
in out							
4			ROOF TOPS/FLAT	1	1	1	27 0
5			DRIVEWAYS/FLAT	1	1	1	27 0

END GEN-INFO

\*\*\* Section IWATER\*\*\*

ACTIVITY

<PLS > ***** Active Sections *****								
#	-	#	ATMP	SNOW	IWAT	SLD	IWG IQAL	***
4			0	0	1	0	0 0	
5			0	0	1	0	0 0	

END ACTIVITY

PRINT-INFO

```

<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL  *****
4   0   0   4   0   0   4   1   9
5   0   0   4   0   0   0   1   9
END PRINT-INFO

```

```

IWAT-PARM1
<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
4   0   0   0   0   0
5   0   0   0   0   0
END IWAT-PARM1

```

```

IWAT-PARM2
<PLS > IWATER input info: Part 2 *****
# - # *** LSUR  SLSUR  NSUR  RETSC
4   400  0.01  0.1  0.1
5   400  0.01  0.1  0.1
END IWAT-PARM2

```

```

IWAT-PARM3
<PLS > IWATER input info: Part 3 *****
# - # ***PETMAX  PETMIN
4   0   0
5   0   0
END IWAT-PARM3

```

```

IWAT-STATE1
<PLS > *** Initial conditions at start of simulation
# - # *** RETS  SURS
4   0   0
5   0   0
END IWAT-STATE1

```

END IMPLND

```

SCHEMATIC
<-Source->          <--Area-->      <-Target->      MBLK   ***
<Name> #           <-factor->      <Name> #        Tbl#   ***
Basin 1***
PERLND 11           2             COPY 501      12
PERLND 11           2             COPY 501      13
PERLND 12           0.77         COPY 501      12
PERLND 12           0.77         COPY 501      13
Upstream***
PERLND 17           0.2          COPY 501      12
PERLND 17           0.2          COPY 501      13
PERLND 11           0.8          COPY 501      12
PERLND 11           0.8          COPY 501      13
IMPLND 4            0.12         COPY 501      15
IMPLND 5            0.23         COPY 501      15

```

\*\*\*\*\*Routing\*\*\*\*\*  
END SCHEMATIC

```

NETWORK
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> #     <Name> # #<-factor->strg <Name> # #     <Name> # #   ***
COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1

```

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> #     <Name> # #<-factor->strg <Name> # #     <Name> # #   ***
END NETWORK

```

```

RCHRES
GEN-INFO
RCHRES Name Nexits Unit Systems Printer ***
# - #<-----><----> User T-series Engl Metr LKFG ***

```





# Mitigated UCI File

RUN

```
GLOBAL
  WWHM4 model simulation
  START      1948 10 01      END      2009 09 30
  RUN INTERP OUTPUT LEVEL   3      0
  RESUME     0 RUN         1
  UNIT SYSTEM                1
END GLOBAL
```

```
FILES
<File> <Un#> <-----File Name----->***
<-ID->                                     ***
WDM      26    Minor East w_Upstream.wdm
MESSU    25    MitMinor East w_Upstream.MES
          27    MitMinor East w_Upstream.L61
          28    MitMinor East w_Upstream.L62
          30    POCMinor East w_Upstreaml.dat
END FILES
```

```
OPN SEQUENCE
  INGRP          INDELT 00:15
  PERLND         13
  IMPLND         1
  IMPLND         4
  IMPLND         5
  IMPLND         8
  PERLND         14
  PERLND         15
  PERLND         17
  PERLND         11
  RCHRES         1
  COPY           1
  COPY           501
  COPY           601
  DISPLY         1
  END INGRP
```

END OPN SEQUENCE

DISPLY

DISPLY-INFO1

```
# - #<-----Title----->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND
1      Vault 1          MAX          1      2      30      9
```

END DISPLY-INFO1

END DISPLY

COPY

TIMESERIES

```
# - # NPT NMN ***
1      1      1
501    1      1
601    1      1
```

END TIMESERIES

END COPY

GENER

OPCODE

```
#      # OPCD ***
```

END OPCODE

PARM

```
#      #          K ***
```

END PARM

END GENER

PERLND

GEN-INFO

```
<PLS ><-----Name----->NBLKS Unit-systems Printer ***
# - # User t-series Engl Metr ***
          in out ***
13      C, Pasture, Flat      1      1      1      1      27      0
14      C, Pasture, Mod      1      1      1      1      27      0
15      C, Pasture, Steep    1      1      1      1      27      0
17      C, Lawn, Mod         1      1      1      1      27      0
```

11 C, Forest, Mod 1 1 1 1 27 0  
 END GEN-INFO  
 \*\*\* Section PWATER\*\*\*

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*  
 13 0 0 1 0 0 0 0 0 0 0 0 0  
 14 0 0 1 0 0 0 0 0 0 0 0 0  
 15 0 0 1 0 0 0 0 0 0 0 0 0  
 17 0 0 1 0 0 0 0 0 0 0 0 0  
 11 0 0 1 0 0 0 0 0 0 0 0 0

END ACTIVITY

PRINT-INFO

<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*\*\*  
 13 0 0 4 0 0 0 0 0 0 0 0 0 1 9  
 14 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9  
 15 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9  
 17 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9  
 11 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9

END PRINT-INFO

PWAT-PARM1

<PLS > PWATER variable monthly parameter value flags \*\*\*  
 # - # CSNO RTOP UZFG VCS VUZ VNM VIFW VIRC VLE INFC HWT \*\*\*  
 13 0 0 0 0 0 0 0 0 0 0 0  
 14 0 0 0 0 0 0 0 0 0 0 0  
 15 0 0 0 0 0 0 0 0 0 0 0  
 17 0 0 0 0 0 0 0 0 0 0 0  
 11 0 0 0 0 0 0 0 0 0 0 0

END PWAT-PARM1

PWAT-PARM2

<PLS > PWATER input info: Part 2 \*\*\*  
 # - # \*\*\*FOREST LZSN INFILT LSUR SLSUR KVARV AGWRC  
 13 0 4.5 0.06 400 0.05 0.5 0.996  
 14 0 4.5 0.06 400 0.1 0.5 0.996  
 15 0 4.5 0.06 400 0.15 0.5 0.996  
 17 0 4.5 0.03 400 0.1 0.5 0.996  
 11 0 4.5 0.08 400 0.1 0.5 0.996

END PWAT-PARM2

PWAT-PARM3

<PLS > PWATER input info: Part 3 \*\*\*  
 # - # \*\*\*PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP  
 13 0 0 2 2 0 0 0  
 14 0 0 2 2 0 0 0  
 15 0 0 2 2 0 0 0  
 17 0 0 2 2 0 0 0  
 11 0 0 2 2 0 0 0

END PWAT-PARM3

PWAT-PARM4

<PLS > PWATER input info: Part 4 \*\*\*  
 # - # CEPSC UZSN NSUR INTFW IRC LZETP \*\*\*  
 13 0.15 0.4 0.3 6 0.5 0.4  
 14 0.15 0.4 0.3 6 0.5 0.4  
 15 0.15 0.25 0.3 6 0.3 0.4  
 17 0.1 0.25 0.25 6 0.5 0.25  
 11 0.2 0.5 0.35 6 0.5 0.7

END PWAT-PARM4

PWAT-STATE1

<PLS > \*\*\* Initial conditions at start of simulation  
 ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 \*\*\*  
 # - # \*\*\* CEPS SURS UZS IFWS LZS AGWS GWVS  
 13 0 0 0 0 2.5 1 0  
 14 0 0 0 0 2.5 1 0  
 15 0 0 0 0 2.5 1 0

```

17          0          0          0          0          2.5          1          0
11          0          0          0          0          2.5          1          0
END PWAT-STATE1

```

END PERLND

IMPLND

```

GEN-INFO
<PLS ><-----Name----->   Unit-systems   Printer   ***
# - #                           User   t-series  Engr Metr ***
                               in   out      ***
1      ROADS/FLAT                1     1     1    27    0
4      ROOF TOPS/FLAT            1     1     1    27    0
5      DRIVEWAYS/FLAT           1     1     1    27    0
8      SIDEWALKS/FLAT            1     1     1    27    0
END GEN-INFO
*** Section IWATER***

```

```

ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT  SLD  IWG IQAL  ***
1      0    0    1    0    0    0
4      0    0    1    0    0    0
5      0    0    1    0    0    0
8      0    0    1    0    0    0
END ACTIVITY

```

```

PRINT-INFO
<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL  *****
1      0    0    4    0    0    4    1    9
4      0    0    4    0    0    0    1    9
5      0    0    4    0    0    0    1    9
8      0    0    4    0    0    0    1    9
END PRINT-INFO

```

```

IWAT-PARM1
<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
1      0    0    0    0    0
4      0    0    0    0    0
5      0    0    0    0    0
8      0    0    0    0    0
END IWAT-PARM1

```

```

IWAT-PARM2
<PLS > IWATER input info: Part 2 ***
# - # *** LSUR  SLSUR  NSUR  RETSC
1      400    0.01  0.1  0.1
4      400    0.01  0.1  0.1
5      400    0.01  0.1  0.1
8      400    0.01  0.1  0.1
END IWAT-PARM2

```

```

IWAT-PARM3
<PLS > IWATER input info: Part 3 ***
# - # ***PETMAX  PETMIN
1      0          0
4      0          0
5      0          0
8      0          0
END IWAT-PARM3

```

```

IWAT-STATE1
<PLS > *** Initial conditions at start of simulation
# - # *** RETS  SURS
1      0          0
4      0          0
5      0          0
8      0          0

```

END IWAT-STATE1

END IMPLND

SCHEMATIC

<-Source->	<Name>	#	<--Area-->	<-factor-->	<-Target-->	<Name>	#	MBLK	Tbl#	***
Basin	1	***								***
PERLND	13		0.84		RCHRES	1		2		
PERLND	13		0.84		RCHRES	1		3		
IMPLND	1		0.29		RCHRES	1		5		
IMPLND	4		1		RCHRES	1		5		
IMPLND	5		0.45		RCHRES	1		5		
IMPLND	8		0.09		RCHRES	1		5		
Upstream	***									
PERLND	17		0.2		RCHRES	1		2		
PERLND	17		0.2		RCHRES	1		3		
PERLND	11		0.8		RCHRES	1		2		
PERLND	11		0.8		RCHRES	1		3		
IMPLND	4		0.12		RCHRES	1		5		
IMPLND	5		0.23		RCHRES	1		5		
Bypass	***									
PERLND	14		0.06		COPY	501		12		
PERLND	14		0.06		COPY	601		12		
PERLND	14		0.06		COPY	501		13		
PERLND	14		0.06		COPY	601		13		
PERLND	15		0.04		COPY	501		12		
PERLND	15		0.04		COPY	601		12		
PERLND	15		0.04		COPY	501		13		
PERLND	15		0.04		COPY	601		13		

\*\*\*\*\*Routing\*\*\*\*\*

PERLND	13		0.84		COPY	1		12		
IMPLND	1		0.29		COPY	1		15		
IMPLND	4		1		COPY	1		15		
IMPLND	5		0.45		COPY	1		15		
IMPLND	8		0.09		COPY	1		15		
PERLND	13		0.84		COPY	1		13		
PERLND	17		0.2		COPY	1		12		
PERLND	11		0.8		COPY	1		12		
IMPLND	4		0.12		COPY	1		15		
IMPLND	5		0.23		COPY	1		15		
PERLND	17		0.2		COPY	1		13		
PERLND	11		0.8		COPY	1		13		
RCHRES	1		1		COPY	501		16		

END SCHEMATIC

NETWORK

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***					
<Name>	#	<Name>	#	<-factor-->	strg	<Name>	#	<Name>	#	***			
COPY	501	OUTPUT	MEAN	1	1	48.4		DISPLY	1	INPUT	TIMSER	1	

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***		
<Name>	#	<Name>	#	<-factor-->	strg	<Name>	#	<Name>	#	***

END NETWORK

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit	Systems	Printer	***
#	-	#	<----->	<---->	User T-series	Engl Metr LKFG
					in out	***
1	Vault	1		1	1	28 0 1

END GEN-INFO

\*\*\* Section RCHRES\*\*\*

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*

#	-	#	HYFG	ADFG	CNFG	HTFG	SDFG	GQFG	OXFG	NUFG	PKFG	PHFG	***



2.666667	0.114050	0.304132	0.166351
2.750000	0.114050	0.313636	0.168931
2.833333	0.114050	0.323140	0.171471
2.916667	0.114050	0.332645	0.173974
3.000000	0.114050	0.342149	0.176442
3.083333	0.114050	0.351653	0.200365
3.166667	0.114050	0.361157	0.211668
3.250000	0.114050	0.370661	0.220867
3.333333	0.114050	0.380165	0.228965
3.416667	0.114050	0.389669	0.236348
3.500000	0.114050	0.399174	0.243217
3.583333	0.114050	0.408678	0.249690
3.666667	0.114050	0.418182	0.255845
3.750000	0.114050	0.427686	0.261736
3.833333	0.114050	0.437190	0.267403
3.916667	0.114050	0.446694	0.272876
4.000000	0.114050	0.456198	0.278179
4.083333	0.114050	0.465702	0.283330
4.166667	0.114050	0.475207	0.288344
4.250000	0.114050	0.484711	0.293236
4.333333	0.114050	0.494215	0.298014
4.416667	0.114050	0.503719	0.302689
4.500000	0.114050	0.513223	0.307268
4.583333	0.114050	0.522727	0.311758
4.666667	0.114050	0.532231	0.316165
4.750000	0.114050	0.541736	0.320494
4.833333	0.114050	0.551240	0.324751
4.916667	0.114050	0.560744	0.328938
5.000000	0.114050	0.570248	0.333061
5.083333	0.114050	0.579752	0.337122
5.166667	0.114050	0.589256	0.341125
5.250000	0.114050	0.598760	0.345072
5.333333	0.114050	0.608264	0.348966
5.416667	0.114050	0.617769	0.352810
5.500000	0.114050	0.627273	0.356605
5.583333	0.114050	0.636777	0.360354
5.666667	0.114050	0.646281	0.364058
5.750000	0.114050	0.655785	0.371405
5.833333	0.114050	0.665289	0.387120
5.916667	0.114050	0.674793	0.407049
6.000000	0.114050	0.684298	0.429897
6.083333	0.114050	0.693802	0.486276
6.166667	0.114050	0.703306	0.526002
6.250000	0.114050	0.712810	0.564033
6.333333	0.114050	0.722314	0.601523
6.416667	0.114050	0.731818	0.638785
6.500000	0.114050	0.741322	0.675888
6.583333	0.114050	0.750826	1.067637
6.666667	0.114050	0.760331	1.768617
6.750000	0.114050	0.769835	2.641403
6.833333	0.114050	0.779339	3.593799
6.916667	0.114050	0.788843	4.531930
7.000000	0.114050	0.798347	5.366192
7.083333	0.114050	0.807851	6.028618
7.166667	0.114050	0.817355	6.496551
7.250000	0.114050	0.826860	6.820942
7.333333	0.114050	0.836364	7.225527
7.416667	0.114050	0.845868	7.548185
7.500000	0.114050	0.855372	7.856654
7.583333	0.114050	0.864876	8.152668

END FTABLE 1  
 END FTABLES

EXT SOURCES

<-Volume->	<Member>	SsysSgap<--Mult-->	Tran	<-Target vols>	<-Grp>	<-Member->	***			
<Name>	#	<Name>	#	tem	strg<-factor->	strg	<Name>	#	#	***
WDM	2	PREC	ENGL	1.2	PERLND	1	999	EXTNL	PREC	
WDM	2	PREC	ENGL	1.2	IMPLND	1	999	EXTNL	PREC	
WDM	1	EVAP	ENGL	0.76	PERLND	1	999	EXTNL	PETINP	
WDM	1	EVAP	ENGL	0.76	IMPLND	1	999	EXTNL	PETINP	

END EXT SOURCES

EXT TARGETS

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Volume->	<Member>	Tsys	Tgap	Amd	***	
<Name>	#	<Name>	#	#<-factor->	strg	<Name>	#	<Name>	tem	strg	strg***
RCHRES	1	HYDR	RO	1 1	1	WDM	1000	FLOW	ENGL	REPL	
RCHRES	1	HYDR	STAGE	1 1	1	WDM	1001	STAG	ENGL	REPL	
COPY	1	OUTPUT	MEAN	1 1	48.4	WDM	701	FLOW	ENGL	REPL	
COPY	501	OUTPUT	MEAN	1 1	48.4	WDM	801	FLOW	ENGL	REPL	
COPY	601	OUTPUT	MEAN	1 1	48.4	WDM	901	FLOW	ENGL	REPL	

END EXT TARGETS

MASS-LINK

<Volume>	<-Grp>	<-Member->	<--Mult-->	<Target>	<-Grp>	<-Member->	***	
<Name>	#	<Name>	#	<Name>	#	<Name>	#	***
MASS-LINK		2						
PERLND	PWATER	SURO	0.083333	RCHRES	INFLOW	IVOL		
END MASS-LINK		2						
MASS-LINK		3						
PERLND	PWATER	IFWO	0.083333	RCHRES	INFLOW	IVOL		
END MASS-LINK		3						
MASS-LINK		5						
IMPLND	IWATER	SURO	0.083333	RCHRES	INFLOW	IVOL		
END MASS-LINK		5						
MASS-LINK		12						
PERLND	PWATER	SURO	0.083333	COPY	INPUT	MEAN		
END MASS-LINK		12						
MASS-LINK		13						
PERLND	PWATER	IFWO	0.083333	COPY	INPUT	MEAN		
END MASS-LINK		13						
MASS-LINK		15						
IMPLND	IWATER	SURO	0.083333	COPY	INPUT	MEAN		
END MASS-LINK		15						
MASS-LINK		16						
RCHRES	ROFLOW			COPY	INPUT	MEAN		
END MASS-LINK		16						

END MASS-LINK

END RUN

## *Disclaimer*

### *Legal Notice*

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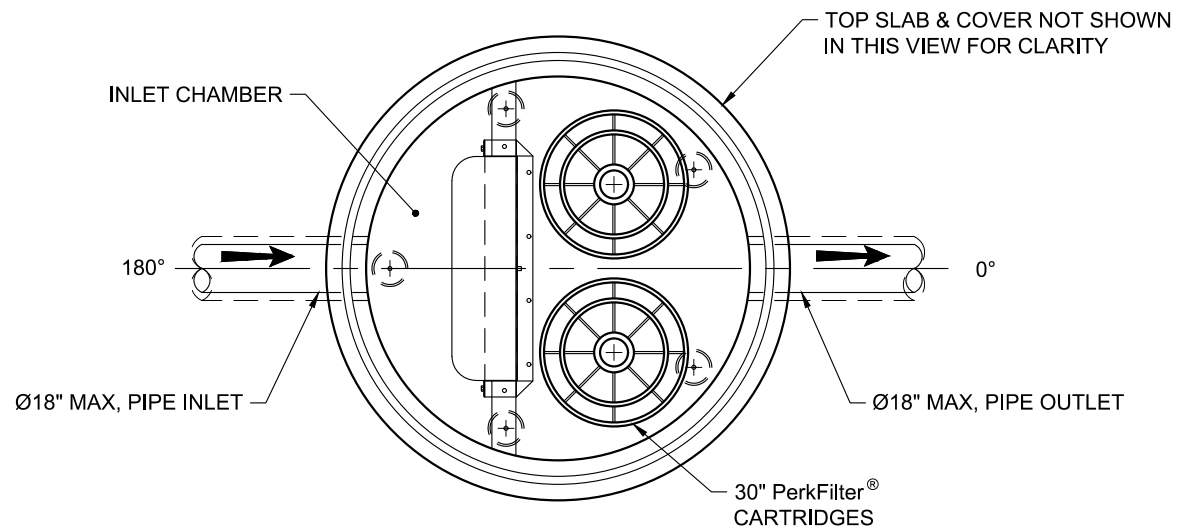
SITE SPECIFIC DATA					MINIMUM DEPTH	
Structure ID	-	Outlet Pipe Size	Minimum Rim to Outlet Depth			
Treatment Flow Rate (gpm/cfs)	-	Ø6"	5.67'			
Peak Flow Rate (cfs)	-	Ø8"	5.92'			
Cartridge Quantity	-	Ø10"	6.17'			
Rim Elevation	-	Ø12"	6.42'			
		Ø15"	6.67'			
		Ø18"	6.92'			

Pipe Data	Pipe Location	Pipe Size	Pipe Type	Invert Elevation
Inlet	-	-	-	-
Outlet	-	-	-	-

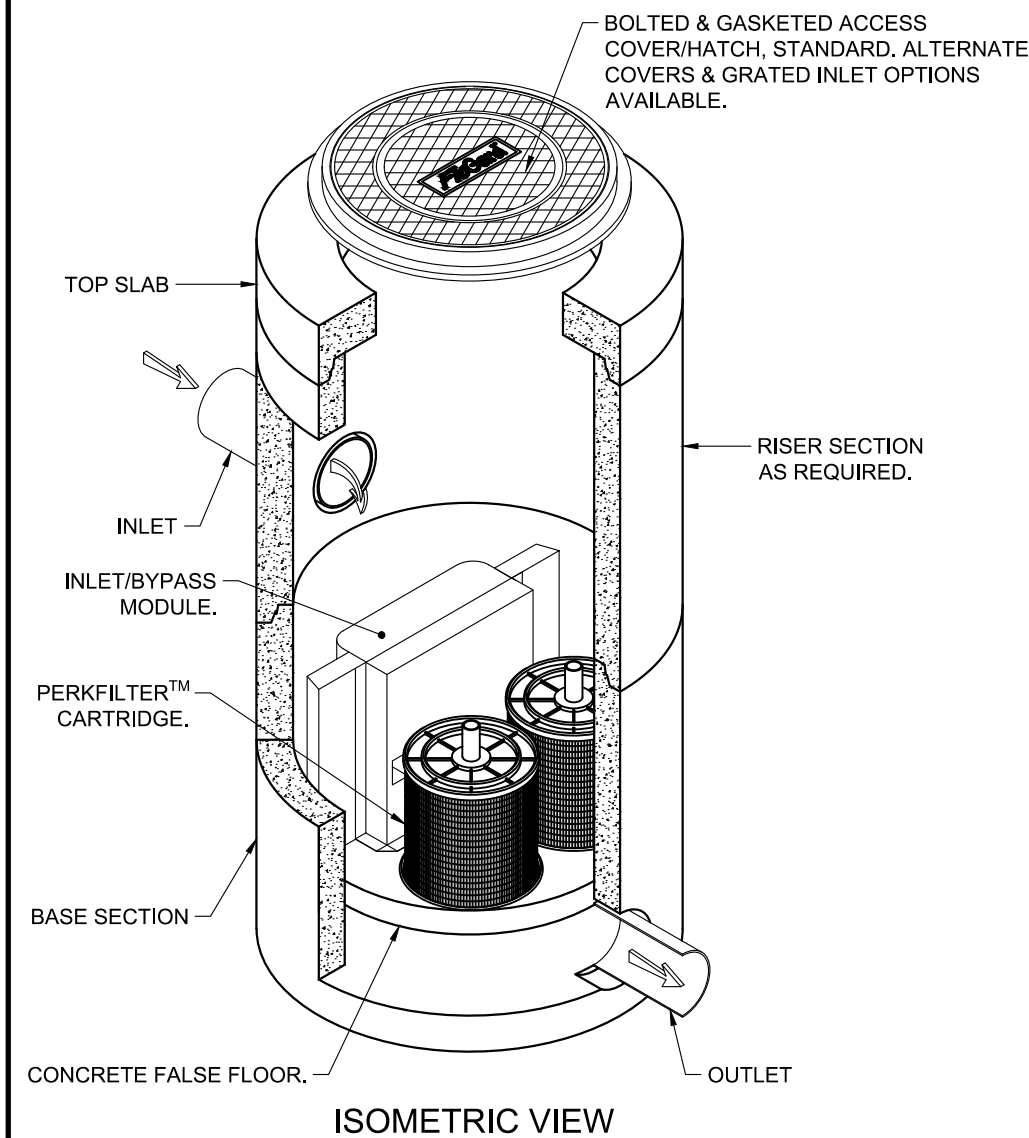
Notes:  
-

PERFORMANCE SPECIFICATIONS	
Peak Treatment Capacities: <sup>1</sup>	
Max. Cartridge Quantity	2
NJDEP 80% Removal, 75 micron	68 gpm / 0.151 cfs
WA Ecology GULD - Basic & Phosphorus	34 gpm / 0.075 cfs
Max. Bypass Capacity	3.62 cfs

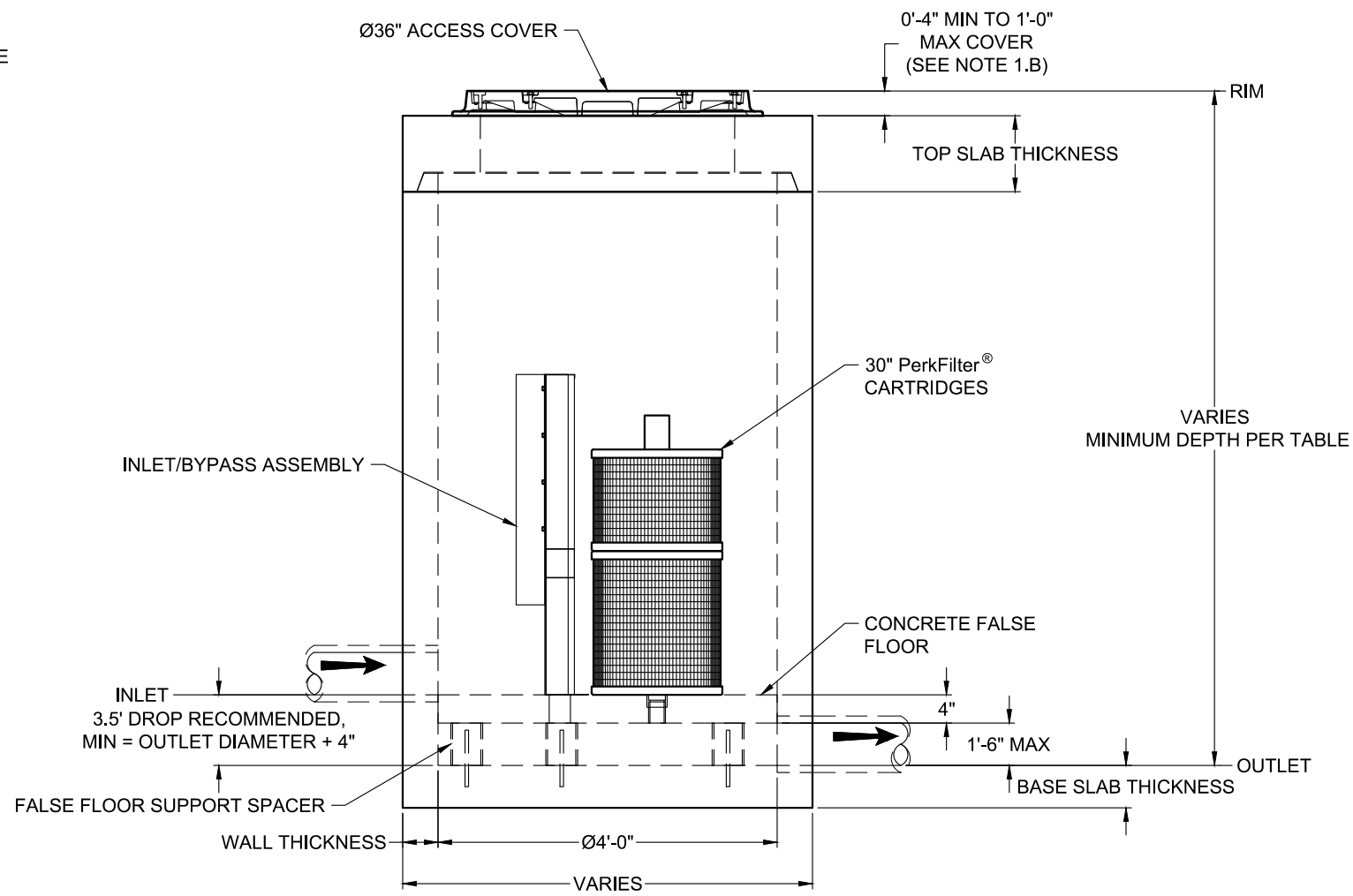
1. Contact Oldcastle for alternative treatment and peak flow capacities.



PLAN VIEW



ISOMETRIC VIEW



ELEVATION VIEW

NOTES:

- DESIGN LOADINGS:
  - AASHTO HS-20-44 (WITH IMPACT)
  - DESIGN SOIL COVER: 1'-0" MAXIMUM
  - ASSUMED WATER TABLE: BELOW INVERT.
  - LATERAL EARTH PRESSURE: 45 PCF (DRAINED)
  - LATERAL LIVE LOAD SURCHARGE: 80 PSF (APPLIED TO 8'-0" BELOW GRADE)
  - NO LATERAL SURCHARGE FROM ADJACENT BUILDINGS, WALLS, PIERS, OR FOUNDATIONS.
- CONCRETE 28-DAY MINIMUM COMPRESSIVE STRENGTH: 5,000 PSI MINIMUM.
- REINFORCING: REBAR, ASTM A615/A706, GRADE 60
- CEMENT: ASTM C150
- REQUIRED ALLOWABLE SOIL BEARING CAPACITY: 2,500 PSF
- REFERENCE STANDARD:
  - ASTM C 478
  - ASTM C 497
- THIS STRUCTURE IS DESIGNED TO THE PARAMETERS NOTED HEREIN. ENGINEER-OF-RECORD SHALL VERIFY THAT NOTED PARAMETERS MEET OR EXCEED PROJECT REQUIREMENTS. IF DESIGN PARAMETERS ARE INCORRECT, REVIEWING ENGINEER/AUTHORITY SHALL NOTIFY OLDCASTLE INFRASTRUCTURE UPON REVIEW OF THIS SUBMITTAL.
- OVERSIZED HOLES TO ACCOMMODATE SPECIFIC PIPE TYPE MUST BE CONCENTRIC TO PIPE ID. AFTER PIPES ARE INSTALLED, ALL ANNULAR SPACES SHALL BE FILLED WITH A MINIMUM OF 3,000 PSI CONCRETE FOR FULL THICKNESS OF PRECAST WALLS. PIPES ARE TO BE FLUSH WITH THE INSIDE SURFACE OF THE CONCRETE STRUCTURE.
- CONTRACTOR RESPONSIBLE TO VERIFY ALL SIZES, LOCATIONS, AND ELEVATIONS OF OPENINGS.
- CONTRACTOR RESPONSIBLE TO ENSURE ADEQUATE BEARING SURFACE IS PROVIDED (I.E. COMPACTED AND LEVEL PER PROJECT SPECIFICATIONS).
- SECTION HEIGHTS, SLAB/WALL THICKNESSES, AND KEYWAYS ARE SUBJECT TO CHANGE AS REQUIRED FOR SITE REQUIREMENTS AND/OR DUE TO PRODUCT AVAILABILITY AND PRODUCTION FACILITY CONSTRAINTS.
- FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT OLDCASTLE INFRASTRUCTURE.
- MAXIMUM PICK WEIGHTS:
  - TOP SLAB: XX,XXX LBS
  - RISER: XX,XXX LBS
  - BASE: XX,XXX LBS\* (\* COMBINED WEIGHT OF BASE INCLUDES FALSE FLOOR, AND PRODUCT INTERNALS.)
- INTERNALS SHALL CONSIST OF CARTRIDGES, INLET/BYPASS ASSEMBLIES, FALSE FLOOR AND FALSE FLOOR SUPPORT SPACERS.



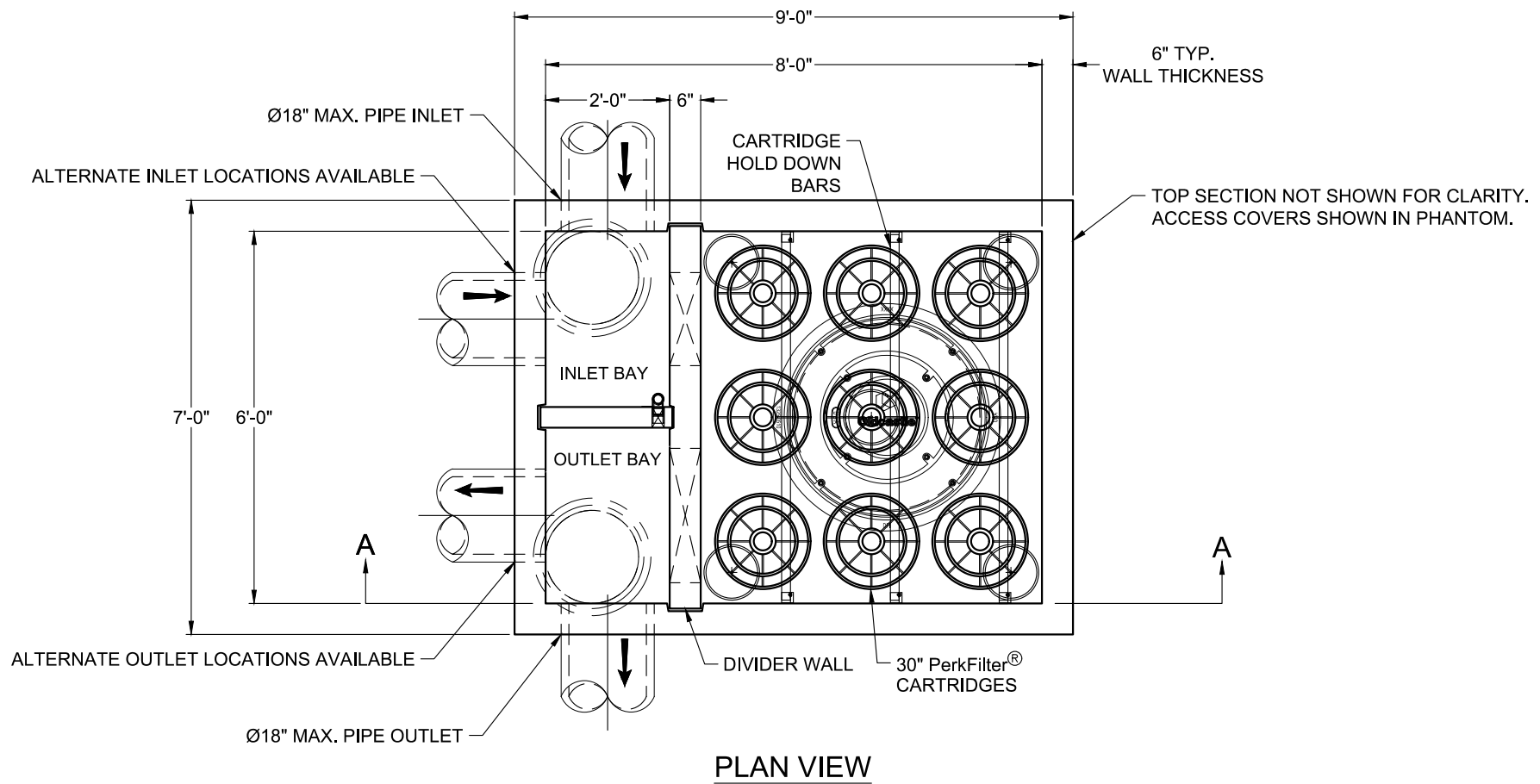
Ph: 800.579.8819 | www.oldcastleinfrastructure.com/stormwater  
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PerkFilter® Manhole (STANDARD)  
 Ø48" with 30" Cartridges

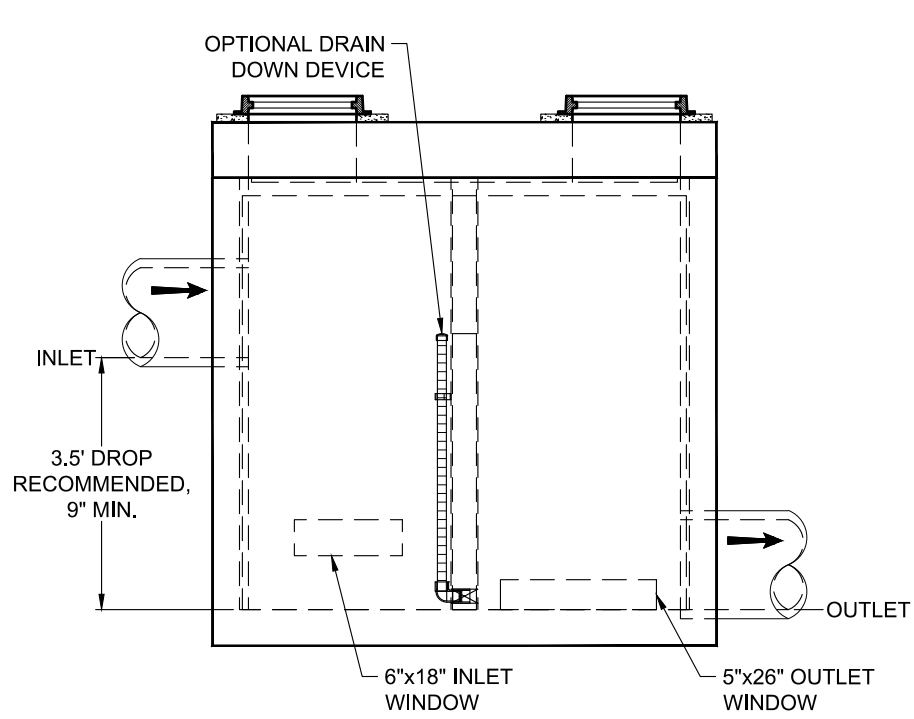
CUSTOMER	-	SHEET	1 OF 1
PROJECT NAME	-	REVISION	-
SHEET NAME	Specifier Drawing	REV DATE	-
	PFMH-48-30		



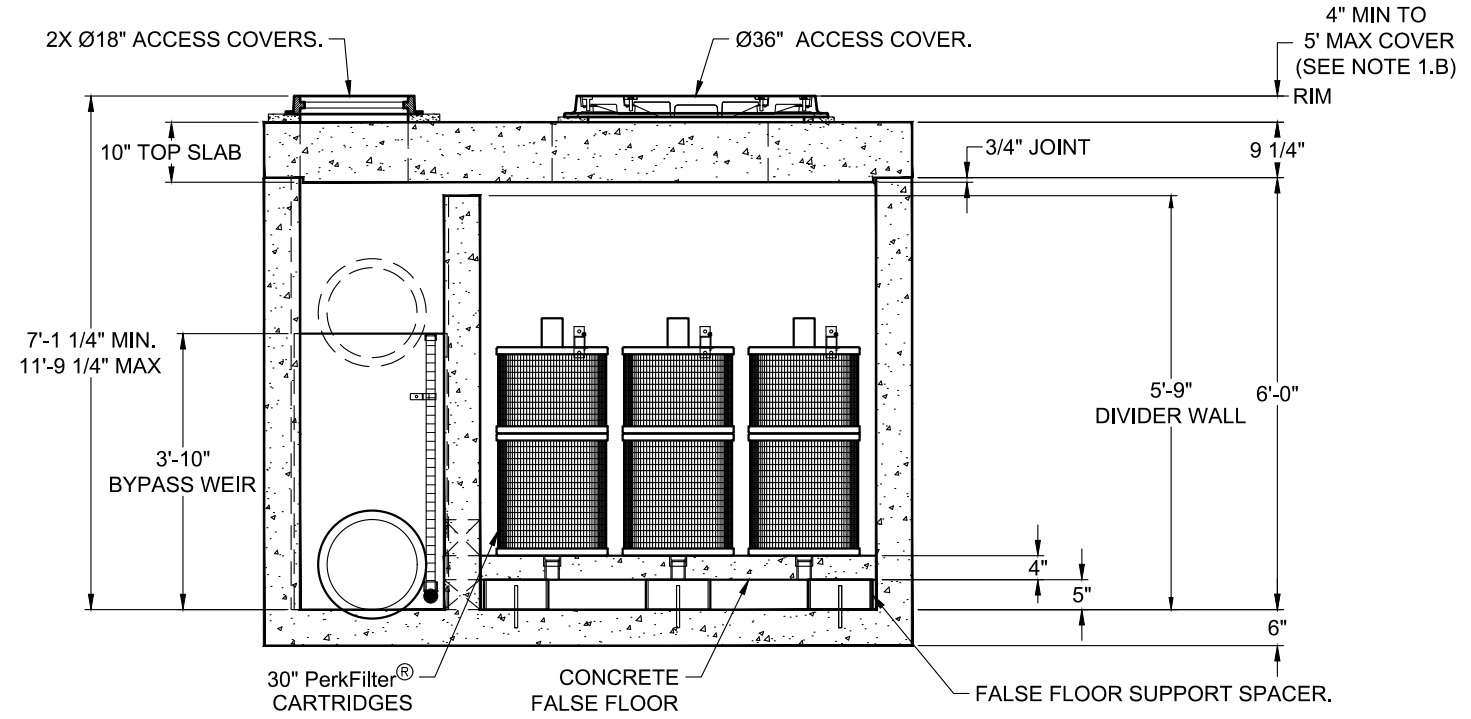
SITE SPECIFIC DATA				
Structure ID	-			
Treatment Flow Rate (gpm/cfs)	-			
Peak Flow Rate (cfs)	-			
Rim Elevation	-			
Pipe Data	Pipe Location	Pipe Size	Pipe Type	Invert Elevation
Inlet 1	-	-	-	-
Inlet 2	-	-	-	-
Outlet	-	-	-	-
Notes: -				
PERFORMANCE SPECIFICATIONS				
Peak Treatment Capacities: <sup>1</sup>				
Max. Cartridge Quantity	9			
NJDEP 80% Removal, 75 micron	0.602 cfs			
WA Ecology GULD - Basic & Phosphorus	0.341 cfs			
Max. Bypass Capacity	18.3 cfs			
<sup>1</sup> Contact Oldcastle for alternative treatment and peak flow capacities.				



PLAN VIEW



LEFT END VIEW



SECTION A-A

- NOTES:**
- DESIGN LOADINGS:
    - AASHTO HS-20-44 (WITH IMPACT)
    - DESIGN SOIL COVER: 5'-0" MAXIMUM
    - ASSUMED WATER TABLE: BELOW BASE OF PRECAST (ENGINEER-OF-RECORD TO CONFIRM SITE WATER TABLE ELEVATION)
    - LATERAL EARTH PRESSURE: 45 PCF (DRAINED)
    - LATERAL LIVE LOAD SURCHARGE: 80 PSF (APPLIED TO 8'-0" BELOW GRADE)
    - NO LATERAL SURCHARGE FROM ADJACENT BUILDINGS, WALLS, PIERS, OR FOUNDATIONS.
  - CONCRETE 28-DAY MINIMUM COMPRESSIVE STRENGTH: 5,000 PSI MINIMUM.
  - REINFORCING: REBAR, ASTM A615/A706, GRADE 60
  - CEMENT: ASTM C150
  - REQUIRED ALLOWABLE SOIL BEARING CAPACITY: 2,500 PSF
  - REFERENCE STANDARD:
    - ASTM C890
    - ASTM C913
    - ACI 318-14
  - THIS STRUCTURE IS DESIGNED TO THE PARAMETERS NOTED HEREIN. ENGINEER-OF-RECORD SHALL VERIFY THAT NOTED PARAMETERS MEET OR EXCEED PROJECT REQUIREMENTS. IF DESIGN PARAMETERS ARE INCORRECT, REVIEWING ENGINEER/AUTHORITY SHALL NOTIFY OLDCASTLE INFRASTRUCTURE UPON REVIEW OF THIS SUBMITTAL.
  - OVERSIZED HOLES TO ACCOMMODATE SPECIFIC PIPE TYPE MUST BE CONCENTRIC TO PIPE ID. AFTER PIPES ARE INSTALLED, ALL ANNULAR SPACES SHALL BE FILLED WITH A MINIMUM OF 3,000 PSI CONCRETE FOR FULL THICKNESS OF PRECAST WALLS. PIPES ARE TO BE FLUSH WITH THE INSIDE SURFACE OF THE CONCRETE STRUCTURE.
  - CONTRACTOR RESPONSIBLE TO VERIFY ALL SIZES, LOCATIONS, AND ELEVATIONS OF OPENINGS.
  - CONTRACTOR RESPONSIBLE TO ENSURE ADEQUATE BEARING SURFACE IS PROVIDED (I.E. COMPACTED AND LEVEL PER PROJECT SPECIFICATIONS).
  - SECTION HEIGHTS, SLAB/WALL THICKNESSES, AND KEYWAYS ARE SUBJECT TO CHANGE AS REQUIRED FOR SITE REQUIREMENTS AND/OR DUE TO PRODUCT AVAILABILITY AND PRODUCTION FACILITY CONSTRAINTS.
  - FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT OLDCASTLE INFRASTRUCTURE.
  - MAXIMUM PICK WEIGHTS:
    - TOP: XX,XXX LBS
    - BASE: XX,XXX LBS\*
 (\* COMBINED WEIGHT OF BASE INCLUDES DIVIDER WALLS, FALSE FLOOR, AND PRODUCT INTERNALS.)
  - INTERNALS SHALL CONSIST OF CARTRIDGES, WEIR WALL, FALSE FLOOR, FALSE FLOOR SUPPORT SPACERS, AND DIVIDER WALL.



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PerkFilter® Vault (STANDARD)		
6'x8' With 30" Cartridges		
CUSTOMER	-	
PROJECT NAME	-	
SHEET NAME	REVISION	SHEET
Specifier Drawing	-	1 OF 1
PFV-68-30	REV DATE	-





PROJECT INFORMATION			
Contact Name:		Company:	
Email:		Phone:	
Date:	<input type="checkbox"/> Rush Request	<input type="checkbox"/> Site Plan Attached	
Project Name:			
Location:		Regulatory Jurisdiction:	
Type of Site:	<input type="checkbox"/> Residential	<input type="checkbox"/> Commercial	<input type="checkbox"/> Mixed Use <input type="checkbox"/> Industrial <input type="checkbox"/> Other
Treatment Technology:	<input type="checkbox"/> Bioretention/Biofiltration	<input type="checkbox"/> Media Filtration	<input type="checkbox"/> Hydrodynamic Separation

## DESIGN SPECIFICATIONS

### Sizing Methodology:

Water Quality Flow    
  Water Quality Volume    
  Downstream of Detention

### Site Information:

Structure ID	Drainage Area ○ sf   ○ acres	Percent Impervious (%)	Runoff Coefficient	WQ Flow Rate or Volume	Peak Flow Rate
				○ cfs/cf   ○ gpm/gal	○ cfs   ○ gpm

### Additional Information (if available):

Water Quality		Water Quality	
On-Line BMP	Off-Line BMP	On-Line BMP	Off-Line BMP
24 hour Volume (ac-ft) <input type="text" value="0.0833"/>		24 hour Volume (ac-ft) <input type="text" value="0.3130"/>	
Standard Flow Rate (cfs) <input type="text" value="0.1103"/>	Standard Flow Rate (cfs) <input type="text" value="0.0623"/>	Standard Flow Rate (cfs) <input type="text" value="0.4129"/>	Standard Flow Rate (cfs) <input type="text" value="0.2332"/>

### Note Special Conditions:

Shallow Application  
 High Groundwater  
 Limited Drop (PerkFilter Only) \_\_\_\_\_

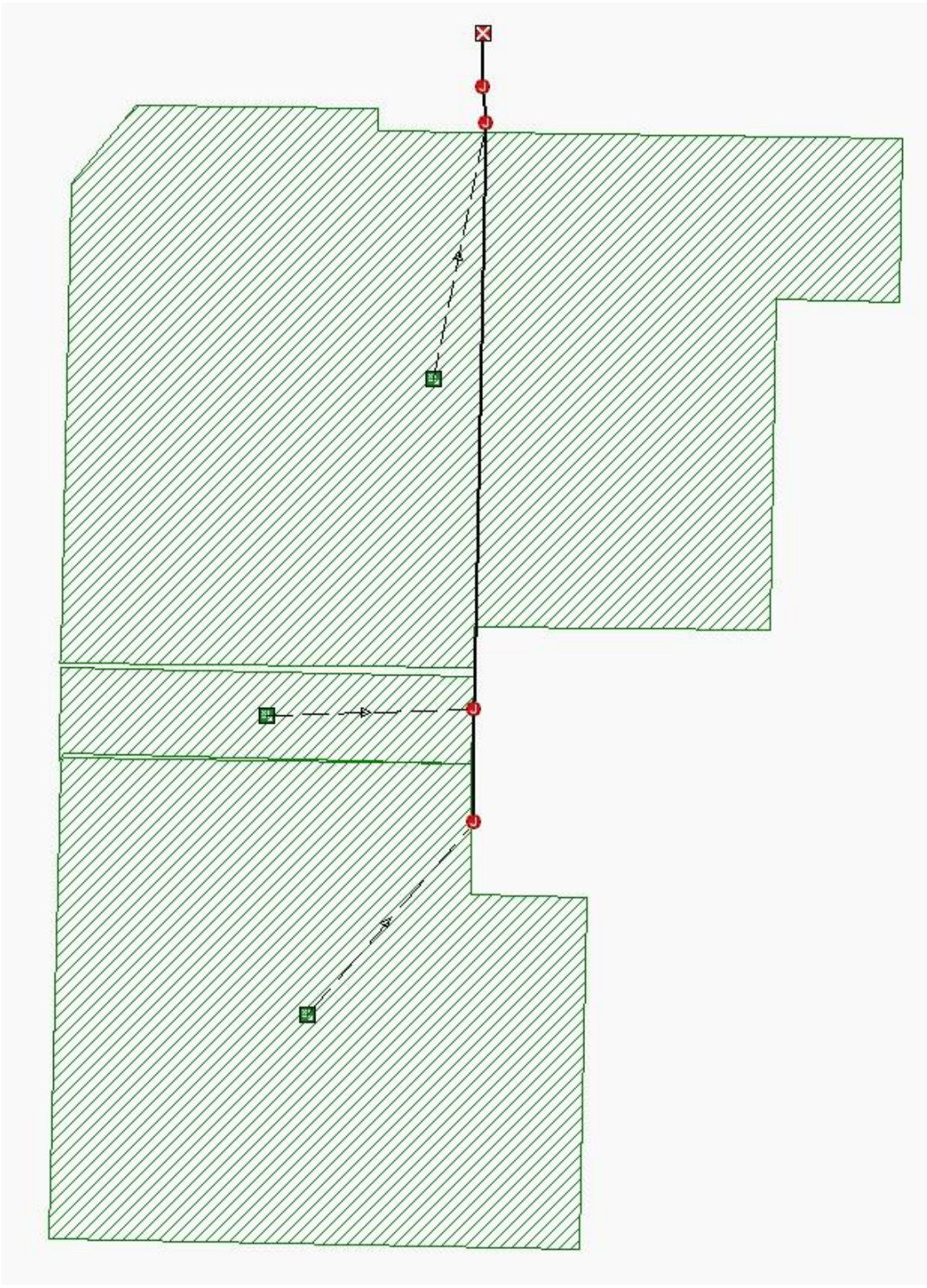
### Notes:

## **Appendix 5: Conveyance Analysis**

1. Conveyance Basins Map
2. SSA Model Layout 1
3. SSA Model Results 1
4. SSA Model Layout 2
5. SSA Model Results 2
6. Manning's Analysis – Vault 1 Discharge
7. Manning's Analysis – Vault 2 Discharge



SSA MODEL LAYOUT 1



## Project Description

File Name ..... West Vault SSA.SPF

## Project Options

Flow Units ..... CFS  
Elevation Type ..... Elevation  
Hydrology Method ..... Rational  
Time of Concentration (TOC) Method ..... User-Defined  
Link Routing Method ..... Kinematic Wave  
Enable Overflow Ponding at Nodes ..... YES  
Skip Steady State Analysis Time Periods ..... NO

## Analysis Options

Start Analysis On ..... 00:00:00      0:00:00  
End Analysis On ..... 00:00:00      0:00:00  
Start Reporting On ..... 00:00:00      0:00:00  
Antecedent Dry Days ..... 0      days  
Runoff (Dry Weather) Time Step ..... 0 01:00:00      days hh:mm:ss  
Runoff (Wet Weather) Time Step ..... 0 00:05:00      days hh:mm:ss  
Reporting Time Step ..... 0 00:05:00      days hh:mm:ss  
Routing Time Step ..... 30      seconds

## Number of Elements

	Qty
Rain Gages .....	0
Subbasins.....	3
Nodes.....	5
<i>Junctions</i> .....	4
<i>Outfalls</i> .....	1
<i>Flow Diversions</i> .....	0
<i>Inlets</i> .....	0
<i>Storage Nodes</i> .....	0
Links.....	4
<i>Channels</i> .....	0
<i>Pipes</i> .....	4
<i>Pumps</i> .....	0
<i>Orifices</i> .....	0
<i>Weirs</i> .....	0
<i>Outlets</i> .....	0
Pollutants .....	0
Land Uses .....	0

## Rainfall Details

Return Period..... 50 year(s)

## Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Runoff Coefficient	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	Sub-1	0.46	0.5800	1.28	0.74	0.34	2.04	0 00:10:00
2	Sub-2	0.06	0.6400	0.75	0.48	0.03	0.35	0 00:05:00
3	Sub-3	0.32	0.5900	0.75	0.45	0.14	1.71	0 00:05:00



## Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft <sup>2</sup> )	Peak Inflow (cfs)	Max HGL Elevation (ft)	Max Surcharge Depth (ft)	Min Freeboard (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	CB-12	Junction	348.80	352.24	348.80	352.24	10.00	3.09	349.43	0.00	2.81	0 00:00	0.00	0.00
2	CB-13	Junction	351.68	354.33	351.68	354.33	10.00	2.04	352.25	0.00	2.08	0 00:00	0.00	0.00
3	CB-14	Junction	351.81	354.33	351.81	354.33	10.00	1.71	352.38	0.00	1.95	0 00:00	0.00	0.00
4	PERKFILTER	Junction	346.40	352.16	346.40	352.16	10.00	3.09	349.32	0.00	2.84	0 00:00	0.00	0.00
5	Out-01	Outfall	346.35					3.09	347.13					

# Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length (ft)	Inlet Invert Elevation (ft)	Outlet Invert Elevation (ft)	Average Slope (%)	Diameter or Height (in)	Manning's Roughness	Peak Flow (cfs)	Design Flow Capacity (cfs)	Peak Flow/Design Flow Ratio	Peak Flow Velocity (ft/sec)	Peak Flow Depth (ft)	Peak Flow Depth/Total Depth Ratio	Total Time Reported (min)	Surcharged Condition
1	Link-01	Pipe	PERKFILTER	Out-01	7.00	346.40	346.35	0.7100	12.000	0.0120	3.09	3.26	0.95	4.72	0.78	0.78	0.00	Calculated
2	Pipe - (137)	Pipe	CB-14	CB-13	26.44	351.81	351.68	0.5000	12.000	0.0120	1.69	2.73	0.62	4.31	0.57	0.57	0.00	Calculated
3	Pipe - (139)	Pipe	CB-13	CB-12	128.78	351.68	348.80	2.2300	12.000	0.0120	2.01	5.77	0.35	6.76	0.41	0.41	0.00	Calculated
4	Pipe - (48)	Pipe	CB-12	PERKFILTER	8.49	348.80	348.69	1.2500	12.000	0.0120	3.09	4.32	0.72	5.97	0.63	0.63	0.00	Calculated

# Subbasin Hydrology

## Subbasin : Sub-1

### Input Data

Area (ac) ..... 0.46  
Weighted Runoff Coefficient ..... 0.58

### Runoff Coefficient

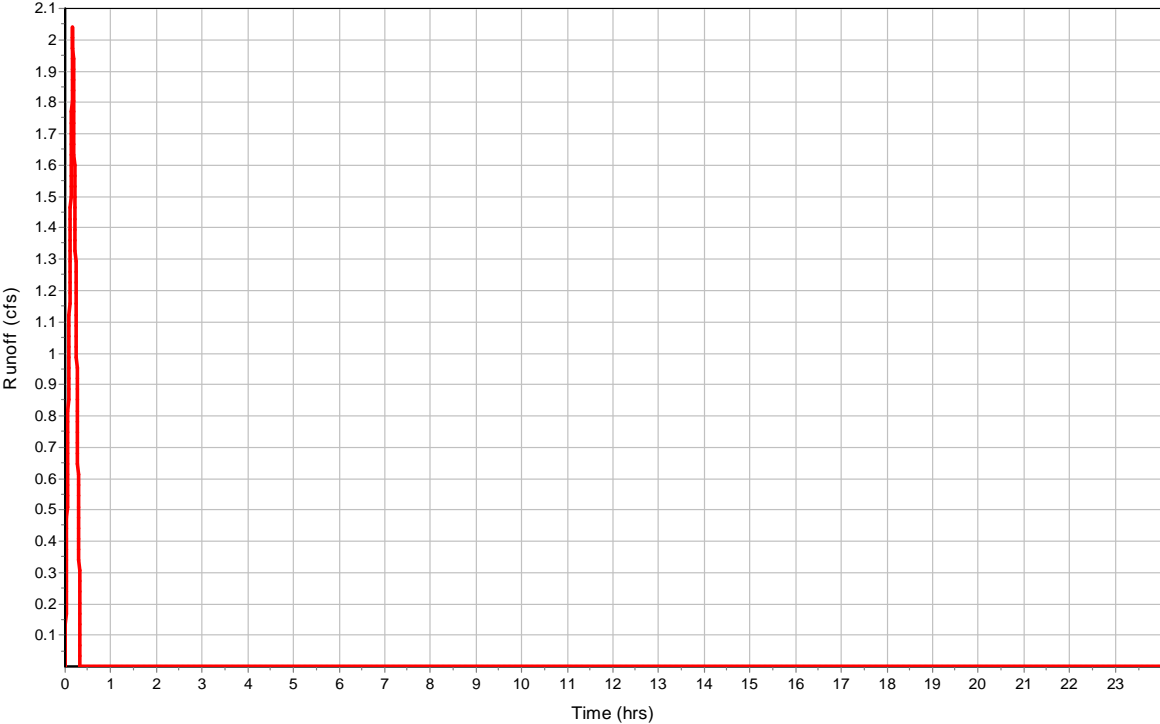
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.33	-	0.72
-	0.13	-	0.24
Composite Area & Weighted Runoff Coeff.	0.46		0.58

### Subbasin Runoff Results

Total Rainfall (in) ..... 1.28  
Total Runoff (in) ..... 0.74  
Peak Runoff (cfs) ..... 2.04  
Rainfall Intensity ..... 7.654  
Weighted Runoff Coefficient ..... 0.58  
Time of Concentration (days hh:mm:ss) ..... 0 00:10:00

Subbasin : Sub-1

Runoff Hydrograph



**Subbasin : Sub-2**

**Input Data**

Area (ac) ..... 0.06  
Weighted Runoff Coefficient ..... 0.64

**Runoff Coefficient**

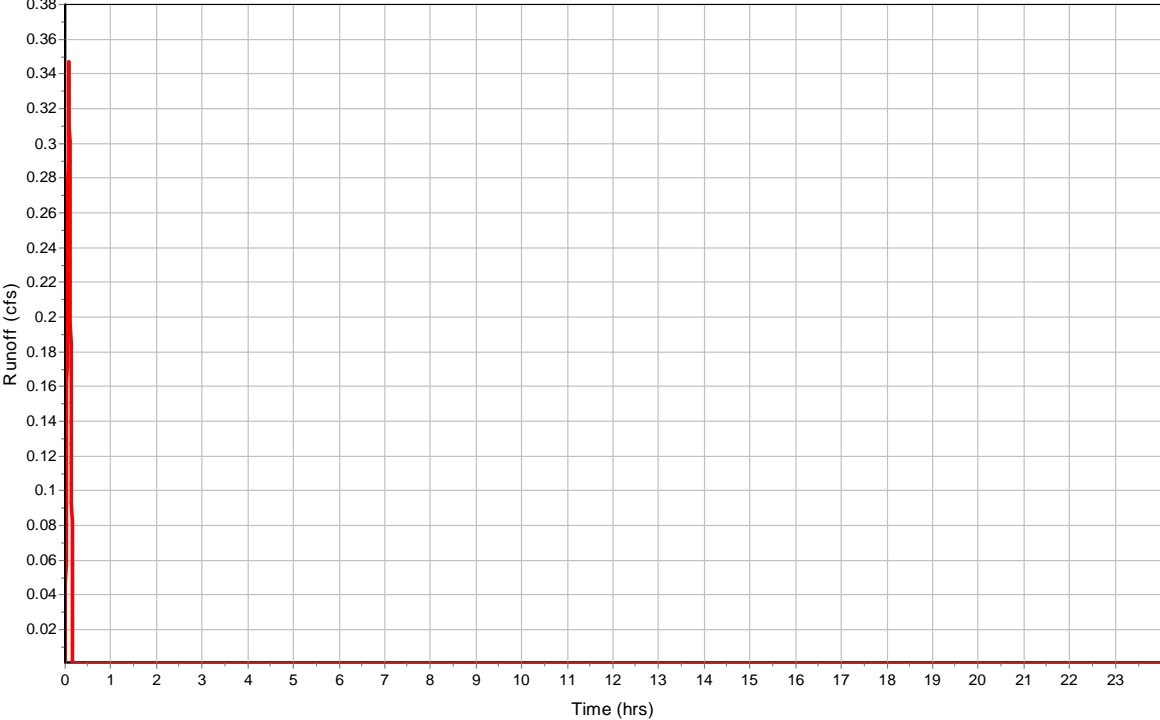
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.05	-	0.72
-	0.01	-	0.24
Composite Area & Weighted Runoff Coeff.	0.06		0.64

**Subbasin Runoff Results**

Total Rainfall (in) ..... 0.75  
Total Runoff (in) ..... 0.48  
Peak Runoff (cfs) ..... 0.35  
Rainfall Intensity ..... 9.041  
Weighted Runoff Coefficient ..... 0.64  
Time of Concentration (days hh:mm:ss) ..... 0 00:05:00

Subbasin : Sub-2

Runoff Hydrograph



**Subbasin : Sub-3**

**Input Data**

Area (ac) ..... 0.32  
Weighted Runoff Coefficient ..... 0.59

**Runoff Coefficient**

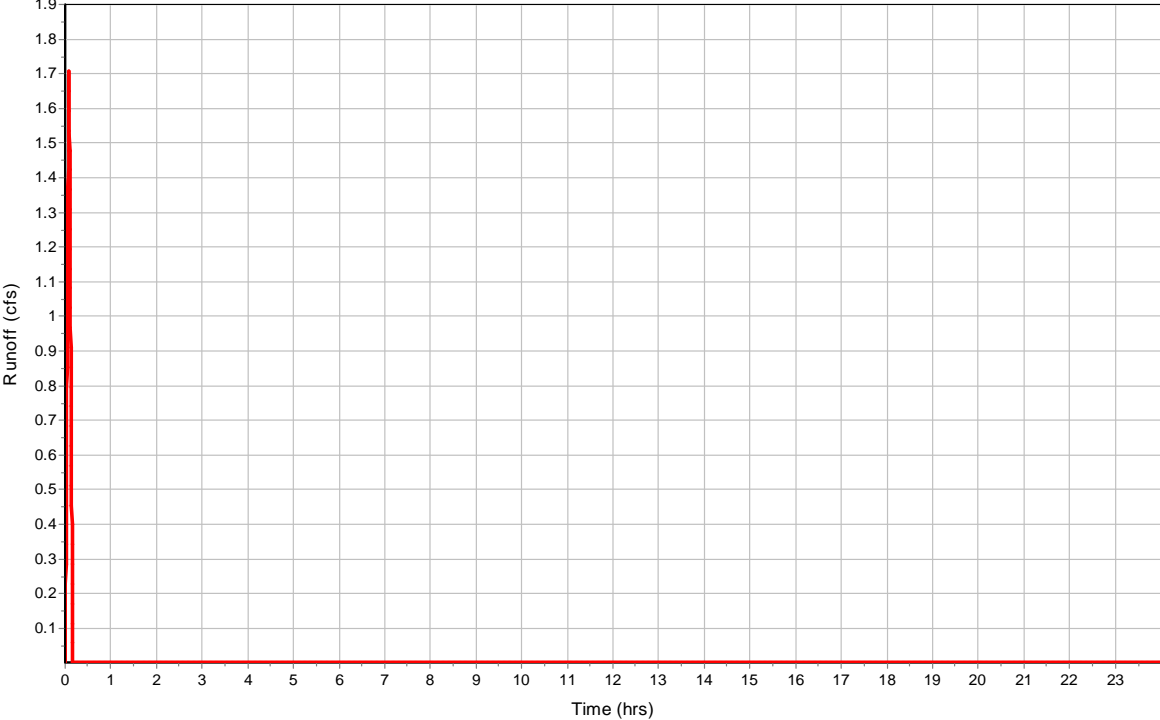
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.23	-	0.72
-	0.09	-	0.24
Composite Area & Weighted Runoff Coeff.	0.32		0.59

**Subbasin Runoff Results**

Total Rainfall (in) ..... 0.75  
Total Runoff (in) ..... 0.45  
Peak Runoff (cfs) ..... 1.71  
Rainfall Intensity ..... 9.041  
Weighted Runoff Coefficient ..... 0.59  
Time of Concentration (days hh:mm:ss) ..... 0 00:05:00

Subbasin : Sub-3

Runoff Hydrograph





## Junction Input

SN Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft <sup>2</sup> )	Minimum Pipe Cover (in)
1 CB-12	348.80	352.24	3.44	348.80	0.00	352.24	0.00	10.00	29.22
2 CB-13	351.68	354.33	2.65	351.68	0.00	354.33	0.00	10.00	19.83
3 CB-14	351.81	354.33	2.52	351.81	0.00	354.33	0.00	10.00	18.28
4 PERKFILTER	346.40	352.16	5.76	346.40	0.00	352.16	0.00	10.00	29.59

## Junction Results

SN Element ID	Peak Inflow	Peak Lateral Inflow	Max HGL Elevation Attained	Max HGL Depth Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Average HGL Elevation Attained	Average HGL Depth Attained	Time of Max HGL Occurrence	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
	(cfs)	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(days hh:mm)	(days hh:mm)	(ac-in)	(min)
1 CB-12	3.09	2.04	349.43	0.63	0.00	2.81	348.81	0.01	0 00:05	0 00:00	0.00	0.00
2 CB-13	2.04	0.35	352.25	0.57	0.00	2.08	351.68	0.00	0 00:05	0 00:00	0.00	0.00
3 CB-14	1.71	1.71	352.38	0.57	0.00	1.95	351.81	0.00	0 00:05	0 00:00	0.00	0.00
4 PERKFILTER	3.09	0.00	349.32	2.92	0.00	2.84	348.70	2.30	0 00:05	0 00:00	0.00	0.00

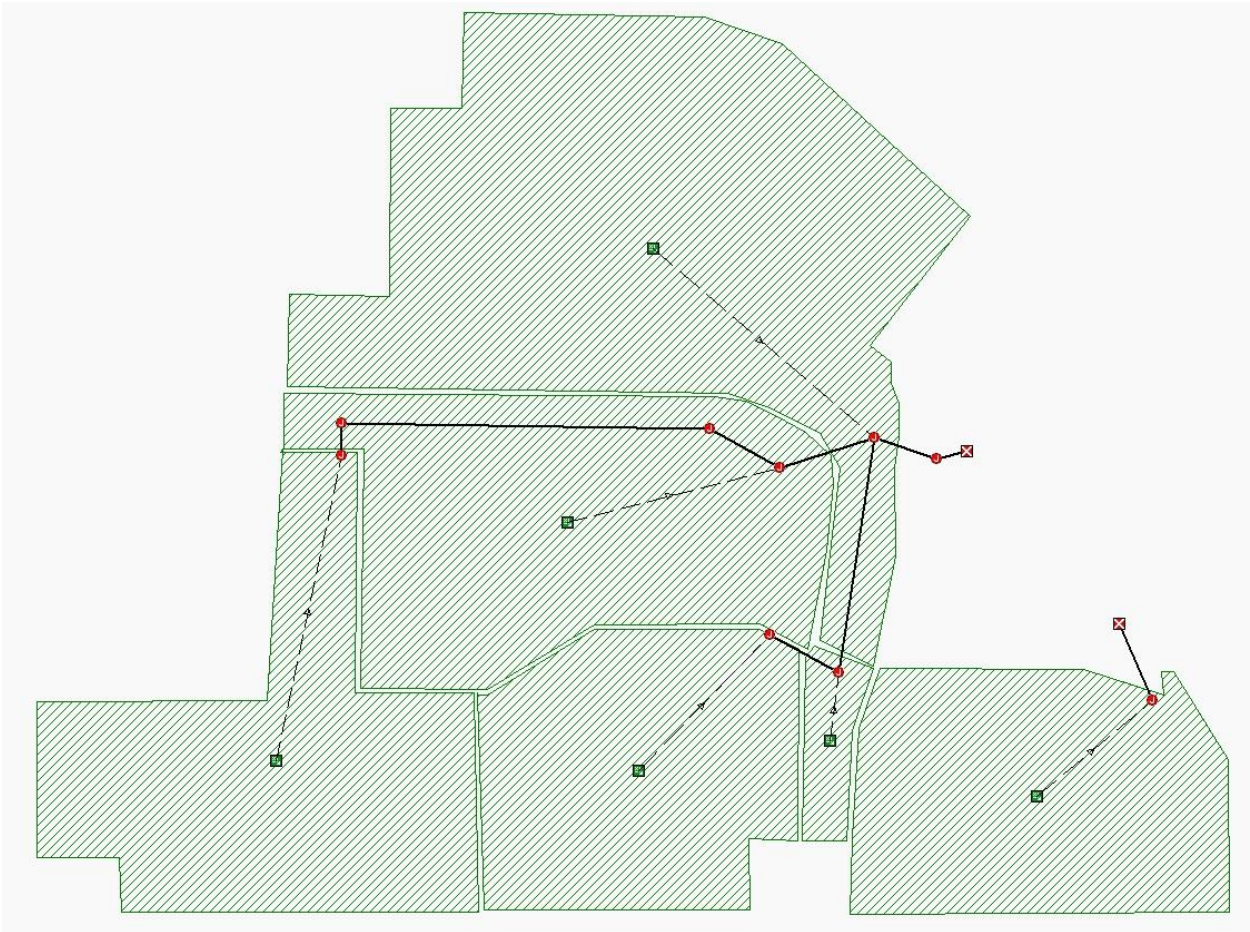
## Pipe Input

SN Element ID	Length	Inlet Invert Elevation	Inlet Invert Offset	Outlet Invert Elevation	Outlet Invert Offset	Total Drop	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1 Link-01	7.00	346.40	0.00	346.35	0.00	0.05	0.7100	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
2 Pipe - (137)	26.44	351.81	0.00	351.68	0.00	0.13	0.5000	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
3 Pipe - (139)	128.78	351.68	0.00	348.80	0.00	2.88	2.2300	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
4 Pipe - (48)	8.49	348.80	0.00	348.69	2.29	0.11	1.2500	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1

## Pipe Results

SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/Total Depth Ratio	Total Time Surcharged	Froude Number	Reported Condition
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)		
1 Link-01	3.09	0 00:05	3.26	0.95	4.72	0.02	0.78	0.78	0.00		Calculated
2 Pipe - (137)	1.69	0 00:05	2.73	0.62	4.31	0.10	0.57	0.57	0.00		Calculated
3 Pipe - (139)	2.01	0 00:05	5.77	0.35	6.76	0.32	0.41	0.41	0.00		Calculated
4 Pipe - (48)	3.09	0 00:05	4.32	0.72	5.97	0.02	0.63	0.63	0.00		Calculated

SSA MODEL LAYOUT 2



## Project Description

File Name ..... East Vault SSA.SPF

## Project Options

Flow Units ..... CFS  
Elevation Type ..... Elevation  
Hydrology Method ..... Rational  
Time of Concentration (TOC) Method ..... User-Defined  
Link Routing Method ..... Kinematic Wave  
Enable Overflow Ponding at Nodes ..... YES  
Skip Steady State Analysis Time Periods ..... NO

## Analysis Options

Start Analysis On ..... 00:00:00      0:00:00  
End Analysis On ..... 00:00:00      0:00:00  
Start Reporting On ..... 00:00:00      0:00:00  
Antecedent Dry Days ..... 0      days  
Runoff (Dry Weather) Time Step ..... 0 01:00:00      days hh:mm:ss  
Runoff (Wet Weather) Time Step ..... 0 00:05:00      days hh:mm:ss  
Reporting Time Step ..... 0 00:05:00      days hh:mm:ss  
Routing Time Step ..... 30      seconds

## Number of Elements

	Qty
Rain Gages .....	0
Subbasins.....	6
Nodes.....	11
<i>Junctions</i> .....	9
<i>Outfalls</i> .....	2
<i>Flow Diversions</i> .....	0
<i>Inlets</i> .....	0
<i>Storage Nodes</i> .....	0
Links.....	9
<i>Channels</i> .....	0
<i>Pipes</i> .....	9
<i>Pumps</i> .....	0
<i>Orifices</i> .....	0
<i>Weirs</i> .....	0
<i>Outlets</i> .....	0
Pollutants .....	0
Land Uses .....	0

## Rainfall Details

Return Period..... 50 year(s)

## Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Runoff Coefficient	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ac-in)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	Sub-4	0.54	0.5800	0.75	0.44	0.24	2.83	0 00:05:00
2	Sub-5	1.11	0.4200	1.97	0.83	0.92	2.76	0 00:20:00
3	Sub-6	0.06	0.6400	0.75	0.48	0.03	0.35	0 00:05:00
4	Sub-7	0.90	0.5800	1.67	0.97	0.87	3.48	0 00:15:00
5	Sub-8	0.49	0.5900	1.28	0.75	0.37	2.21	0 00:10:00
6	Sub-9	0.69	0.5900	0.75	0.45	0.31	3.68	0 00:05:00

## Node Summary

SN	Element ID	Element Type	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Initial Water Elevation (ft)	Surcharge Elevation (ft)	Ponded Area (ft <sup>2</sup> )	Peak Inflow (cfs)	Max HGL Elevation (ft)	Max Surcharge Depth (ft)	Min Freeboard (ft)	Time of Peak Flooding Occurrence (days hh:mm)	Total Flooded Volume (ac-in)	Total Time Flooded (min)
1	CB-2A	Junction	340.50	345.69	340.50	345.69	10.00	3.48	341.15	0.00	4.54	0 00:00	0.00	0.00
2	CB-3	Junction	344.10	347.75	344.10	347.75	10.00	8.42	345.09	0.00	2.66	0 00:00	0.00	0.00
3	CB-4	Junction	346.69	350.15	346.69	350.15	10.00	2.76	347.44	0.00	2.70	0 00:00	0.00	0.00
4	CB-4A	Junction	346.85	350.14	346.85	350.14	10.00	2.76	347.61	0.00	2.53	0 00:00	0.00	0.00
5	CB-5	Junction	344.89	347.97	344.89	347.97	10.00	3.94	345.41	0.00	2.57	0 00:00	0.00	0.00
6	CB-6	Junction	345.50	348.56	345.50	348.56	10.00	2.77	346.01	0.00	2.54	0 00:00	0.00	0.00
7	CB-7	Junction	350.90	353.42	350.90	353.42	10.00	2.83	351.33	0.00	2.09	0 00:00	0.00	0.00
8	CB-8	Junction	351.60	354.12	351.60	354.12	10.00	2.83	351.99	0.00	2.14	0 00:00	0.00	0.00
9	Out-1Pipe - (116)	Junction	340.50	349.30	340.50	349.30	10.00	8.42	343.39	0.00	5.91	0 00:00	0.00	0.00
10	Out-1Pipe - (112)	Outfall	340.45					8.42	341.34					
11	Out-1Pipe - (89)	Outfall	340.29					3.47	340.94					



## Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length (ft)	Inlet Invert Elevation (ft)	Outlet Invert Elevation (ft)	Average Slope (%)	Diameter or Height (in)	Manning's Roughness	Peak Flow (cfs)	Design Flow Capacity (cfs)	Peak Flow/Design Flow Ratio	Peak Flow Velocity (ft/sec)	Peak Flow Depth (ft)	Peak Flow Depth/Total Depth Ratio	Total Time Reported (min)	Surcharged Condition
1	Link-02	Pipe	Out-1Pipe - (116)	Out-1Pipe - (112)	4.00	340.50	340.45	1.2500	18.000	0.0120	8.42	12.72	0.66	7.69	0.89	0.59	0.00	Calculated
2	Pipe - (116)	Pipe	CB-3	Out-1Pipe - (116)	26.00	344.10	342.80	5.0000	18.000	0.0120	8.42	25.45	0.33	12.92	0.59	0.40	0.00	Calculated
3	Pipe - (38)	Pipe	CB-7	CB-6	148.75	350.90	345.50	3.6300	12.000	0.0120	2.77	7.35	0.38	8.96	0.42	0.43	0.00	Calculated
4	Pipe - (39)	Pipe	CB-8	CB-7	13.06	351.60	350.90	5.3600	12.000	0.0120	2.83	8.94	0.32	10.09	0.39	0.39	0.00	Calculated
5	Pipe - (56)	Pipe	CB-5	CB-3	40.03	344.29	344.10	0.4700	18.000	0.0120	3.94	15.99	0.25	7.50	0.51	0.34	0.00	Calculated
6	Pipe - (57)	Pipe	CB-4	CB-3	95.81	346.69	344.60	2.1800	12.000	0.0120	2.75	5.70	0.48	7.21	0.49	0.49	0.00	Calculated
7	Pipe - (58)	Pipe	CB-4A	CB-4	26.44	346.85	346.69	0.6100	12.000	0.0120	2.76	3.00	0.92	4.34	0.75	0.75	0.00	Calculated
8	Pipe - (59)	Pipe	CB-6	CB-5	32.33	345.50	344.89	1.8800	12.000	0.0120	2.77	5.29	0.52	6.82	0.51	0.51	0.00	Calculated
9	Pipe - (89)	Pipe	CB-2A	Out-1Pipe - (89)	33.41	340.50	340.29	0.6300	18.000	0.0120	3.47	9.02	0.38	4.78	0.65	0.43	0.00	Calculated

# Subbasin Hydrology

## Subbasin : Sub-4

### Input Data

Area (ac) ..... 0.54  
Weighted Runoff Coefficient ..... 0.58

### Runoff Coefficient

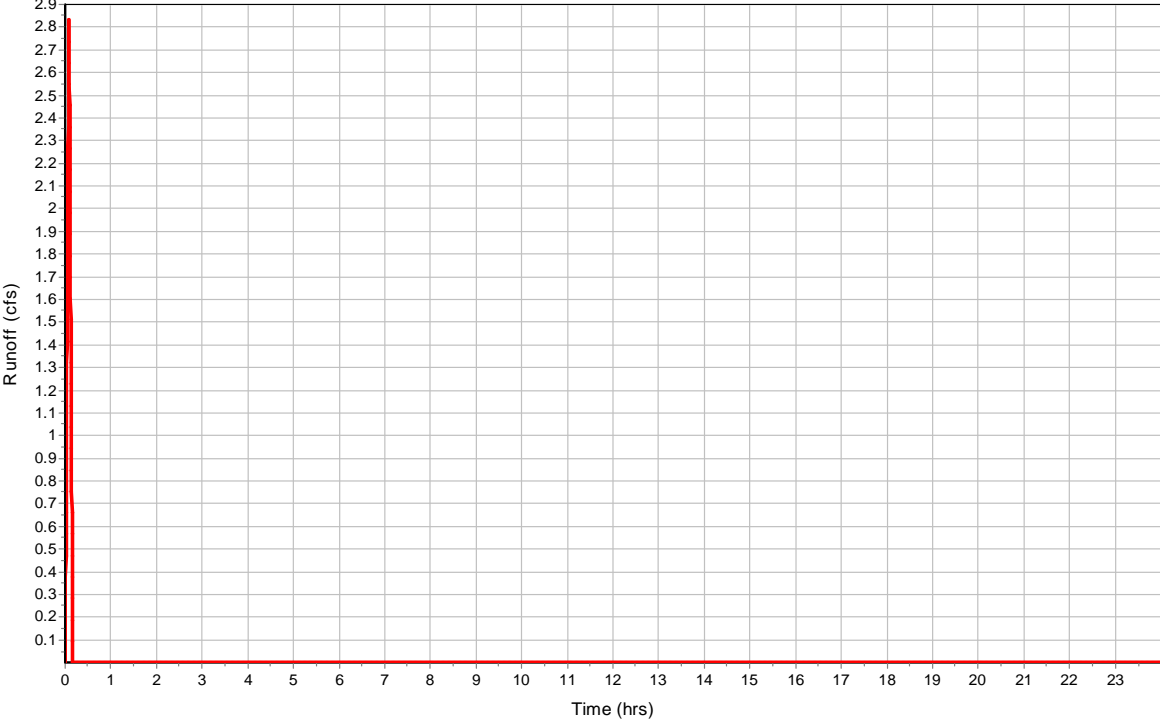
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.38	-	0.72
-	0.16	-	0.24
Composite Area & Weighted Runoff Coeff.	0.54		0.58

### Subbasin Runoff Results

Total Rainfall (in) ..... 0.75  
Total Runoff (in) ..... 0.44  
Peak Runoff (cfs) ..... 2.83  
Rainfall Intensity ..... 9.041  
Weighted Runoff Coefficient ..... 0.58  
Time of Concentration (days hh:mm:ss) ..... 0 00:05:00

Subbasin : Sub-4

Runoff Hydrograph



**Subbasin : Sub-5**

**Input Data**

Area (ac) ..... 1.11  
Weighted Runoff Coefficient ..... 0.42

**Runoff Coefficient**

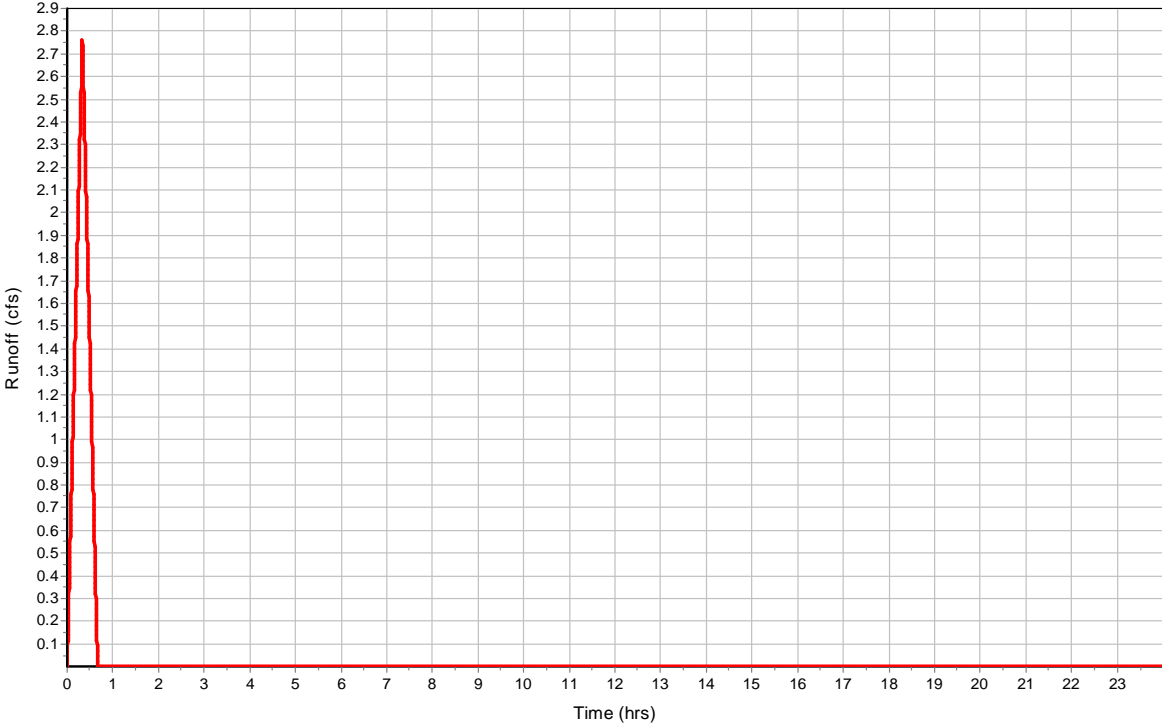
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.41	-	0.72
-	0.7	-	0.24
Composite Area & Weighted Runoff Coeff.	1.11		0.42

**Subbasin Runoff Results**

Total Rainfall (in) ..... 1.97  
Total Runoff (in) ..... 0.83  
Peak Runoff (cfs) ..... 2.76  
Rainfall Intensity ..... 5.921  
Weighted Runoff Coefficient ..... 0.42  
Time of Concentration (days hh:mm:ss) ..... 0 00:20:00

Subbasin : Sub-5

Runoff Hydrograph



**Subbasin : Sub-6**

**Input Data**

Area (ac) ..... 0.06  
Weighted Runoff Coefficient ..... 0.64

**Runoff Coefficient**

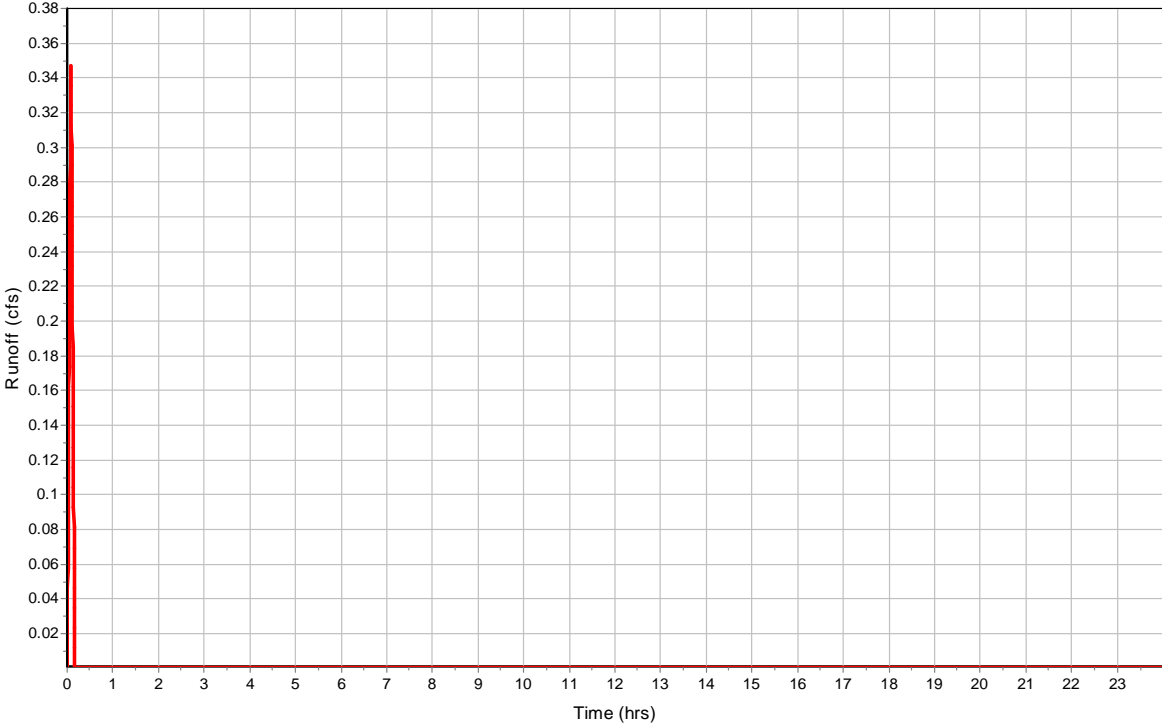
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.05	-	0.72
-	0.01	-	0.24
Composite Area & Weighted Runoff Coeff.	0.06		0.64

**Subbasin Runoff Results**

Total Rainfall (in) ..... 0.75  
Total Runoff (in) ..... 0.48  
Peak Runoff (cfs) ..... 0.35  
Rainfall Intensity ..... 9.041  
Weighted Runoff Coefficient ..... 0.64  
Time of Concentration (days hh:mm:ss) ..... 0 00:05:00

Subbasin : Sub-6

Runoff Hydrograph



**Subbasin : Sub-7**

**Input Data**

Area (ac) ..... 0.9  
Weighted Runoff Coefficient ..... 0.58

**Runoff Coefficient**

Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.63	-	0.72
-	0.27	-	0.24
Composite Area & Weighted Runoff Coeff.	0.9		0.58

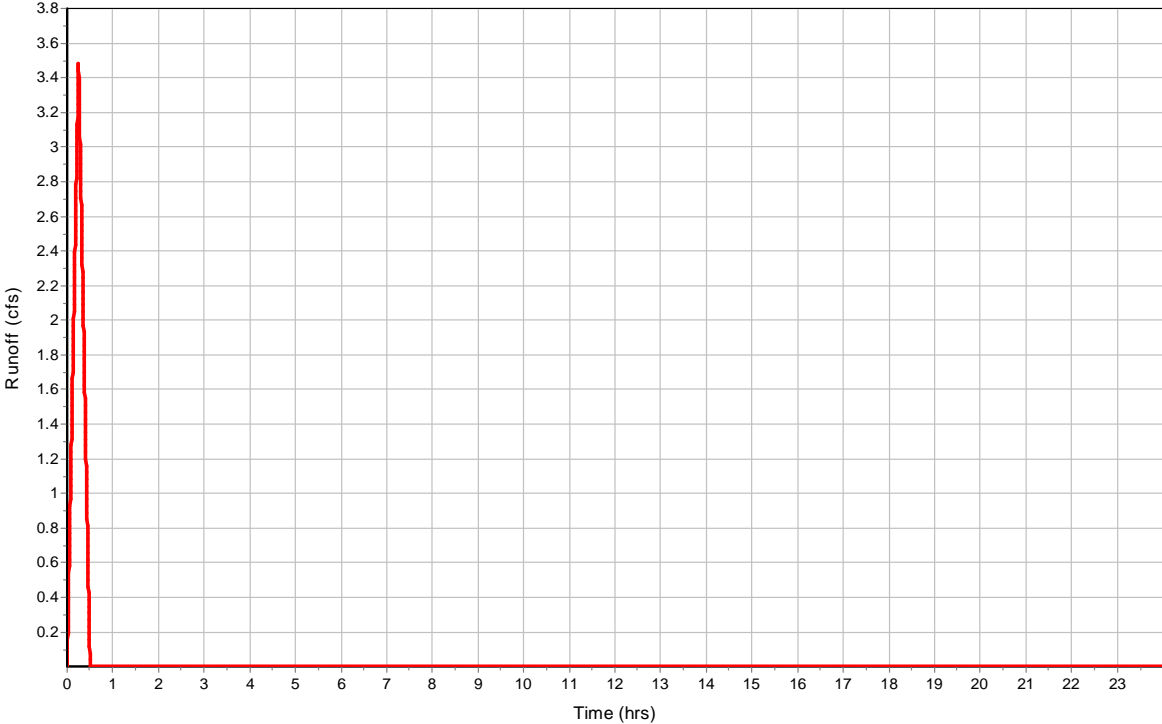
**Subbasin Runoff Results**

Total Rainfall (in) ..... 1.67  
Total Runoff (in) ..... 0.97  
Peak Runoff (cfs) ..... 3.48  
Rainfall Intensity ..... 6.665  
Weighted Runoff Coefficient ..... 0.58  
Time of Concentration (days hh:mm:ss) ..... 0 00:15:00



Subbasin : Sub-7

Runoff Hydrograph



**Subbasin : Sub-8**

**Input Data**

Area (ac) ..... 0.49  
Weighted Runoff Coefficient ..... 0.59

**Runoff Coefficient**

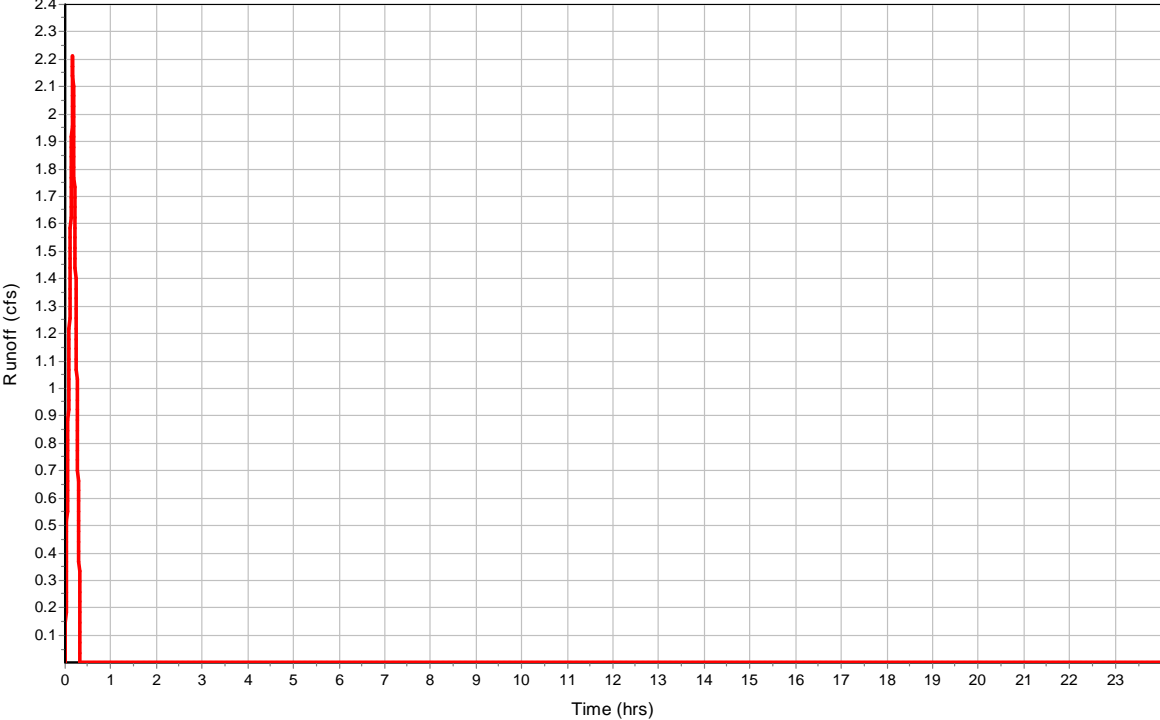
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.36	-	0.72
-	0.13	-	0.24
Composite Area & Weighted Runoff Coeff.	0.49		0.59

**Subbasin Runoff Results**

Total Rainfall (in) ..... 1.28  
Total Runoff (in) ..... 0.75  
Peak Runoff (cfs) ..... 2.21  
Rainfall Intensity ..... 7.654  
Weighted Runoff Coefficient ..... 0.59  
Time of Concentration (days hh:mm:ss) ..... 0 00:10:00

Subbasin : Sub-8

Runoff Hydrograph



**Subbasin : Sub-9**

**Input Data**

Area (ac) ..... 0.69  
Weighted Runoff Coefficient ..... 0.59

**Runoff Coefficient**

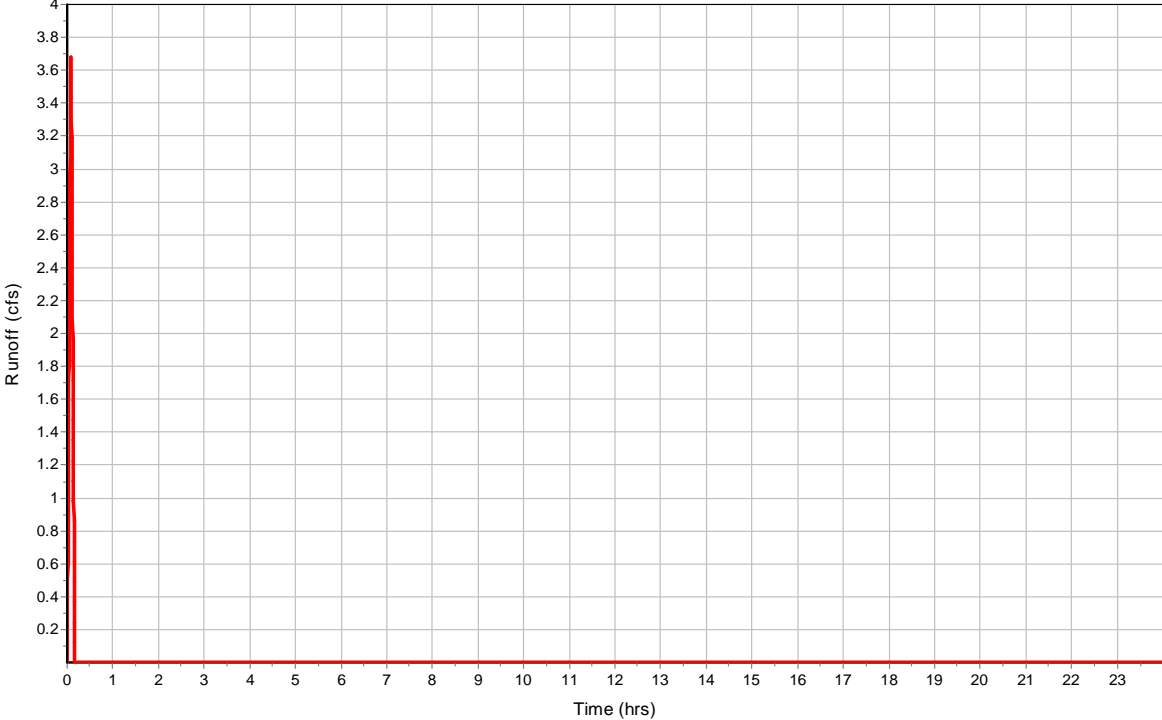
Soil/Surface Description	Area (acres)	Soil Group	Runoff Coeff.
-	0.5	-	0.72
-	0.19	-	0.24
Composite Area & Weighted Runoff Coeff.	0.69		0.59

**Subbasin Runoff Results**

Total Rainfall (in) ..... 0.75  
Total Runoff (in) ..... 0.45  
Peak Runoff (cfs) ..... 3.68  
Rainfall Intensity ..... 9.041  
Weighted Runoff Coefficient ..... 0.59  
Time of Concentration (days hh:mm:ss) ..... 0 00:05:00

Subbasin : Sub-9

Runoff Hydrograph



## Junction Input

SN Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Surcharge Elevation (ft)	Surcharge Depth (ft)	Ponded Area (ft <sup>2</sup> )	Minimum Pipe Cover (in)
1 CB-2A	340.50	345.69	5.19	340.50	0.00	345.69	0.00	10.00	44.25
2 CB-3	344.10	347.75	3.65	344.10	0.00	347.75	0.00	10.00	25.80
3 CB-4	346.69	350.15	3.46	346.69	0.00	350.15	0.00	10.00	29.47
4 CB-4A	346.85	350.14	3.29	346.85	0.00	350.14	0.00	10.00	27.46
5 CB-5	344.89	347.97	3.08	344.89	0.00	347.97	0.00	10.00	24.96
6 CB-6	345.50	348.56	3.06	345.50	0.00	348.56	0.00	10.00	24.70
7 CB-7	350.90	353.42	2.52	350.90	0.00	353.42	0.00	10.00	18.28
8 CB-8	351.60	354.12	2.52	351.60	0.00	354.12	0.00	10.00	18.28
9 Out-1Pipe - (116)	340.50	349.30	8.80	340.50	0.00	349.30	0.00	10.00	60.00

## Junction Results

SN Element ID	Peak Inflow	Peak Lateral Inflow	Max HGL Elevation Attained	Max HGL Depth Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Average HGL Elevation Attained	Average HGL Depth Attained	Time of Max HGL Occurrence	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
	(cfs)	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(days hh:mm)	(days hh:mm)	(ac-in)	(min)
1 CB-2A	3.48	3.48	341.15	0.65	0.00	4.54	340.51	0.01	0 00:15	0 00:00	0.00	0.00
2 CB-3	8.42	3.68	345.09	0.99	0.00	2.66	344.61	0.51	0 00:20	0 00:00	0.00	0.00
3 CB-4	2.76	0.35	347.44	0.75	0.00	2.70	346.70	0.01	0 00:20	0 00:00	0.00	0.00
4 CB-4A	2.76	2.76	347.61	0.76	0.00	2.53	346.86	0.01	0 00:20	0 00:00	0.00	0.00
5 CB-5	3.94	2.21	345.41	0.52	0.00	2.57	344.90	0.01	0 00:05	0 00:00	0.00	0.00
6 CB-6	2.77	0.00	346.01	0.51	0.00	2.54	345.50	0.00	0 00:05	0 00:00	0.00	0.00
7 CB-7	2.83	0.00	351.33	0.43	0.00	2.09	350.90	0.00	0 00:05	0 00:00	0.00	0.00
8 CB-8	2.83	2.83	351.99	0.39	0.00	2.14	351.60	0.00	0 00:05	0 00:00	0.00	0.00
9 Out-1Pipe - (116)	8.42	0.00	343.39	2.89	0.00	5.91	342.81	2.31	0 00:05	0 00:00	0.00	0.00

## Pipe Input

SN Element ID	Length	Inlet Invert Elevation	Inlet Invert Offset	Outlet Invert Elevation	Outlet Invert Offset	Total Drop	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1 Link-02	4.00	340.50	0.00	340.45	0.00	0.05	1.2500	CIRCULAR	18.000	18.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
2 Pipe - (116)	26.00	344.10	0.00	342.80	2.30	1.30	5.0000	CIRCULAR	18.000	18.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
3 Pipe - (38)	148.75	350.90	0.00	345.50	0.00	5.40	3.6300	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
4 Pipe - (39)	13.06	351.60	0.00	350.90	0.00	0.70	5.3600	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
5 Pipe - (56)	40.03	344.29	-0.60	344.10	0.00	0.19	0.4700	CIRCULAR	18.000	18.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
6 Pipe - (57)	95.81	346.69	0.00	344.60	0.50	2.09	2.1800	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
7 Pipe - (58)	26.44	346.85	0.00	346.69	0.00	0.16	0.6100	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
8 Pipe - (59)	32.33	345.50	0.00	344.89	0.00	0.61	1.8800	CIRCULAR	12.000	12.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1
9 Pipe - (89)	33.41	340.50	0.00	340.29	0.00	0.21	0.6300	CIRCULAR	18.000	18.000	0.0120	0.5000	0.5000	0.0000	0.00	No	1



## Pipe Results

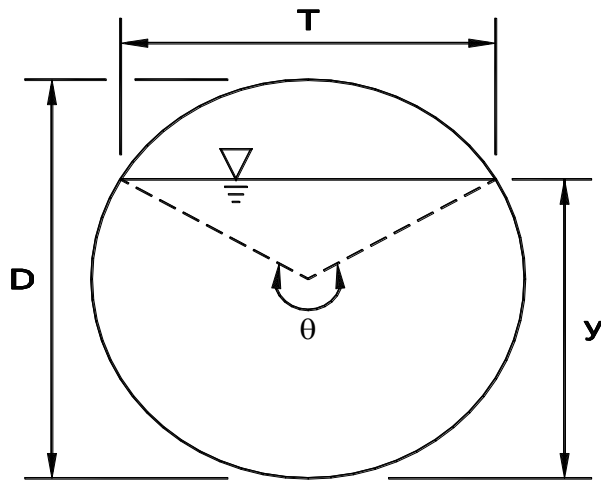
SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged	Froude Number	Reported Condition
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)		
1 Link-02	8.42	0 00:05	12.72	0.66	7.69	0.01	0.89	0.59	0.00		Calculated
2 Pipe - (116)	8.42	0 00:05	25.45	0.33	12.92	0.03	0.59	0.40	0.00		Calculated
3 Pipe - (38)	2.77	0 00:05	7.35	0.38	8.96	0.28	0.42	0.43	0.00		Calculated
4 Pipe - (39)	2.83	0 00:05	8.94	0.32	10.09	0.02	0.39	0.39	0.00		Calculated
5 Pipe - (56)	3.94	0 00:05	15.99	0.25	7.50	0.09	0.51	0.34	0.00		Calculated
6 Pipe - (57)	2.75	0 00:20	5.70	0.48	7.21	0.22	0.49	0.49	0.00		Calculated
7 Pipe - (58)	2.76	0 00:20	3.00	0.92	4.34	0.10	0.75	0.75	0.00		Calculated
8 Pipe - (59)	2.77	0 00:05	5.29	0.52	6.82	0.08	0.51	0.51	0.00		Calculated
9 Pipe - (89)	3.47	0 00:15	9.02	0.38	4.78	0.12	0.65	0.43	0.00		Calculated

<b>Open Channel Flow Calculator For Circular Pipes</b>	<b>Land Development Consultants, Inc.</b>	
	14201 NE 200th St. Ste. 100 Woodinville, WA 98072	Tel: (425) 806-1869 Fax: (425) 482-2893

Project Name: Minor Property  
 Description: Detention Vault 1 Discharge

Project No.: 23-0011  
 Date: 3/27/2024  
 Calc. By: CJD

Pipe Diameter (D) = 12 in  
 Pipe Slope (S) = 0.50 %  
**Flow Depth (y) = 0.34 ft**  
 Flowrate (Q) = 0.91 cfs  
 Mannings Coeff. (n) = 0.009  
 Theta Angle ( $\theta$ ) = 2.49 rad  
 Wetted Area (A) = 0.24 ft<sup>2</sup>  
 Wet. Perimeter (P) = 1.25 ft  
 Hydraulic Radius (R) = 0.19 ft  
 Top Width (T) = 0.95 ft  
 Flow Velocity = 3.86 fps



**Formulas:**

Theta Angle ( $\theta$ ):  
 If  $y \geq r$ :  $\theta = 2\pi - 2a \cos\left(\frac{y-r}{r}\right)$  where: r = Pipe Radius  
 If  $y \leq r$ :  $\theta = 2a \cos\left(\frac{r-y}{r}\right)$  where: r = Pipe Radius

Wetted Area (A):  $A = \frac{1}{8}(\theta - \sin \theta)d^2$

Wetted Perimeter (P):  $P = \frac{1}{2}\theta d$

Hydraulic Radius (R):  $R = \frac{A}{P}$

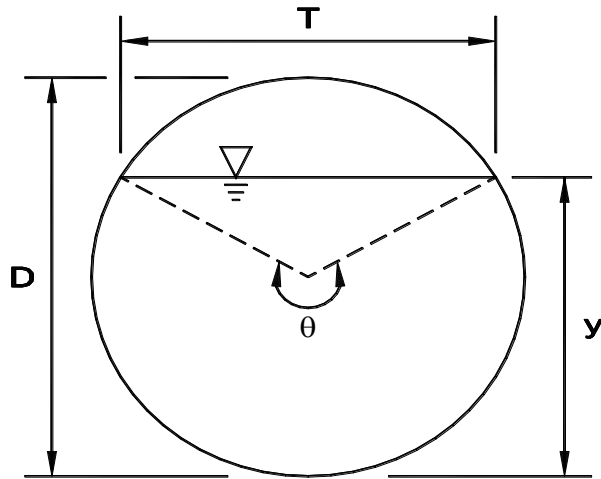
Top Width (T):  $T = \sin\left(\frac{\theta}{2}\right)d$

<b>Open Channel Flow Calculator For Circular Pipes</b>	<b>Land Development Consultants, Inc.</b>	
	14201 NE 200th St. Ste. 100 Woodinville, WA 98072	Tel: (425) 806-1869 Fax: (425) 482-2893

Project Name: Minor Property  
Description: Detention Vault 2 Discharge

Project No.: 23-0011  
Date: 3/27/2024  
Calc. By: CJD

Pipe Diameter (D) = 18 in  
Pipe Slope (S) = 0.50 %  
**Flow Depth (y) = 0.57 ft**  
Flowrate (Q) = 3.34 cfs  
Mannings Coeff. (n) = 0.009  
Theta Angle ( $\theta$ ) = 2.67 rad  
Wetted Area (A) = 0.62 ft<sup>2</sup>  
Wet. Perimeter (P) = 2.00 ft  
Hydraulic Radius (R) = 0.31 ft  
Top Width (T) = 1.46 ft  
Flow Velocity = 5.37 fps



**Formulas:**

Theta Angle ( $\theta$ ):  
If  $y \geq r$ :  $\theta = 2\pi - 2a \cos\left(\frac{y-r}{r}\right)$  where: r = Pipe Radius  
If  $y \leq r$ :  $\theta = 2a \cos\left(\frac{r-y}{r}\right)$  where: r = Pipe Radius

Wetted Area (A):  $A = \frac{1}{8}(\theta - \sin \theta)d^2$

Wetted Perimeter (P):  $P = \frac{1}{2}\theta d$

Hydraulic Radius (R):  $R = \frac{A}{P}$

Top Width (T):  $T = \sin\left(\frac{\theta}{2}\right)d$

## **Appendix 6: Operations and Maintenance Manual**

1. Operations and Maintenance Manual

**Table V-A.2: Maintenance Standards - Infiltration (continued)**

Maintenance Component	Defect	Conditions When Maintenance Is Needed	Results Expected When Maintenance Is Performed
		(A percolation test pit or test of facility indicates facility is only working at 90% of its designed capabilities. Test every 2 to 5 years. If two inches or more sediment is present, remove).	
Filter Bags (if applicable)	Filled with Sediment and Debris	Sediment and debris fill bag more than 1/2 full.	Filter bag is replaced or system is redesigned.
Rock Filters	Sediment and Debris	By visual inspection, little or no water flows through filter during heavy rain storms.	Gravel in rock filter is replaced.
Side Slopes of Pond	Erosion	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>
Emergency Overflow Spillway and Berms over 4 feet in height.	Tree Growth	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>
	Piping	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>
Emergency Overflow Spillway	Rock Missing	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>
	Erosion	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>
Pre-settling Ponds and Vaults	Facility or sump filled with Sediment and/or debris	6" or designed sediment trap depth of sediment.	Sediment is removed.

**Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)**

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Storage Area	Plugged Air Vents	One-half of the cross section of a vent is blocked at any point or the vent is damaged.	Vents open and functioning.
	Debris and Sediment	Accumulated sediment depth exceeds 10% of the diameter of the storage area for 1/2 length of storage vault or any point depth exceeds 15% of diameter. (Example: 72-inch storage tank would require cleaning when sediment reaches depth of 7 inches for more than 1/2 length of tank.)	All sediment and debris removed from storage area.
	Joints Between Tank/Pipe Section	Any openings or voids allowing material to be transported into facility. (Will require engineering analysis to determine structural stability).	All joint between tank/pipe sections are sealed.
	Tank Pipe Bent Out of Shape	Any part of tank/pipe is bent out of shape more than 10% of its design shape. (Review required by engineer to determine structural stability).	Tank/pipe repaired or replaced to design.
	Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab	Cracks wider than 1/2-inch and any evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determines that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or any evidence of soil particles entering the vault through the walls.	Vault replaced or repaired to design specifications and is structurally sound. No cracks more than 1/4-inch wide at the joint of the inlet/outlet pipe.

**Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults) (continued)**

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Manhole	Cover Not in Place	Cover is missing or only partially in place. Any open manhole requires maintenance.	Manhole is closed.
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread (may not apply to self-locking lids).	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. Intent is to keep cover from sealing off access to maintenance.	Cover can be removed and reinstalled by one maintenance person.
	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, misalignment, not securely attached to structure wall, rust, or cracks.	Ladder meets design standards. Allows maintenance person safe access.
Catch Basins	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>

**Table V-A.4: Maintenance Standards - Control Structure/Flow Restrictor**

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
General	Trash and Debris (Includes Sediment)	Material exceeds 25% of sump depth or 1 foot below orifice plate.	Control structure orifice is not blocked. All trash and debris removed.
	Structural Damage	Structure is not securely attached to manhole wall. Structure is not in upright position (allow up to 10% from plumb). Connections to outlet pipe are not watertight and show signs of rust. Any holes - other than designed holes - in the structure.	Structure securely attached to wall and outlet pipe. Structure in correct position. Connections to outlet pipe are water tight; structure repaired or replaced and works as designed. Structure has no holes other than designed holes.
Cleanout Gate	Damaged or Missing	Cleanout gate is not watertight or is missing. Gate cannot be moved up and down by one maintenance person. Chain/rod leading to gate is missing or damaged. Gate is rusted over 50% of its surface area.	Gate is watertight and works as designed. Gate moves up and down easily and is watertight. Chain is in place and works as designed. Gate is repaired or replaced to meet design standards.
Orifice Plate	Damaged or Missing	Control device is not working properly due to missing, out of place, or bent orifice plate.	Plate is in place and works as designed.
	Obstructions	Any trash, debris, sediment, or vegetation blocking the plate.	Plate is free of all obstructions and works as designed.
Overflow Pipe	Obstructions	Any trash or debris blocking (or having the potential of blocking) the overflow pipe.	Pipe is free of all obstructions and works as designed.
Manhole	See <a href="#">Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)</a>	See <a href="#">Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)</a>	See <a href="#">Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)</a>
Catch Basin	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>	See <a href="#">Table V-A.5: Maintenance Standards - Catch Basins</a>

**Table V-A.5: Maintenance Standards - Catch Basins**

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is performed
General	Trash & Debris	Trash or debris which is located immediately in front of the catch basin opening or is blocking inletting capacity of the basin by more than 10%. Trash or debris (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of six inches clearance from the debris surface to the invert of the lowest pipe. Trash or debris in any inlet or outlet pipe blocking more than 1/3 of its height. Dead animals or vegetation that could generate odors that could cause complaints or dangerous gases (e.g., methane).	No Trash or debris located immediately in front of catch basin or on grate opening. No trash or debris in the catch basin. Inlet and outlet pipes free of trash or debris. No dead animals or vegetation present within the catch basin.
	Sediment	Sediment (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of 6 inches clearance from the sediment surface to the invert of the lowest pipe.	No sediment in the catch basin
	Structure Damage to Frame and/or Top Slab	Top slab has holes larger than 2 square inches or cracks wider than 1/4 inch. (Intent is to make sure no material is running into basin). Frame not sitting flush on top slab, i.e., separation of more than 3/4 inch of the frame from the top slab. Frame not securely attached	Top slab is free of holes and cracks. Frame is sitting flush on the riser rings or top slab and firmly attached.
	Fractures or Cracks in Basin Walls/ Bottom	Maintenance person judges that structure is unsound. Grout fillet has separated or cracked wider than 1/2 inch and longer than 1 foot at the joint of any inlet/outlet pipe or any evidence of soil particles entering catch basin through cracks.	Basin replaced or repaired to design standards. Pipe is regouted and secure at basin wall.
	Settlement/ Mis-alignment	If failure of basin has created a safety, function, or design problem.	Basin replaced or repaired to design standards.
	Vegetation	Vegetation growing across and blocking more than 10% of the basin opening. Vegetation growing in inlet/outlet pipe joints that is more than six inches tall and less than six inches apart.	No vegetation blocking opening to basin. No vegetation or root growth present.
	Contamination and Pollution	See <a href="#">Table V-A.1: Maintenance Standards - Detention Ponds</a>	No pollution present.
Catch Basin Cover	Cover Not in Place	Cover is missing or only partially in place. Any open catch basin requires maintenance.	Cover/grate is in place, meets design standards, and is secured
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread.	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. (Intent is keep cover from sealing off access to maintenance.)	Cover can be removed by one maintenance person.
Ladder	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, not securely attached to basin wall, misalignment, rust, cracks, or sharp edges.	Ladder meets design standards and allows maintenance person safe access.
Metal Grates (If Applicable)	Grate opening Unsafe	Grate with opening wider than 7/8 inch.	Grate opening meets design standards.
	Trash and Debris	Trash and debris that is blocking more than 20% of grate surface inletting capacity.	Grate free of trash and debris.
	Damaged or Missing.	Grate missing or broken member(s) of the grate.	Grate is in place, meets the design standards, and is installed and aligned with the flow path.

**Table V-A.13: Maintenance Standards - Sand Filters (Above Ground/Open) (continued)**

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
	Flow Spreader	Flow spreader uneven or clogged so that flows are not uniformly distributed across sand filter.	Spreader leveled and cleaned so that flows are spread evenly over sand filter.
	Damaged Pipes	Any part of the piping that is crushed or deformed more than 20% or any other failure to the piping.	Pipe repaired or replaced.

**Table V-A.14: Maintenance Standards - Sand Filters (Below Ground/Enclosed)**

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Below Ground Vault.	Sediment Accumulation on Sand Media Section	Sediment depth exceeds 1/2-inch.	No sediment deposits on sand filter section that which would impede permeability of the filter section.
	Sediment Accumulation in Pre-Settling Portion of Vault	Sediment accumulation in vault bottom exceeds the depth of the sediment zone plus 6-inches.	No sediment deposits in first chamber of vault.
	Trash/Debris Accumulation	Trash and debris accumulated in vault, or pipe inlet/outlet, floatables and non-floatables.	Trash and debris removed from vault and inlet/outlet piping.
	Sediment in Drain Pipes/Cleanouts	When drain pipes, cleanouts become full with sediment and/or debris.	Sediment and debris removed.
	Short Circuiting	When seepage/flow occurs along the vault walls and corners. Sand eroding near inflow area.	Sand filter media section re-laid and compacted along perimeter of vault to form a semi-seal. Erosion protection added to dissipate force of incoming flow and curtail erosion.
	Damaged Pipes	Inlet or outlet piping damaged or broken and in need of repair.	Pipe repaired and/or replaced.
	Access Cover Damaged/Not Working	Cover cannot be opened, corrosion/deformation of cover. Maintenance person cannot remove cover using normal lifting pressure.	Cover repaired to proper working specifications or replaced.
	Ventilation	Ventilation area blocked or plugged	Blocking material removed or cleared from ventilation area. A specified % of the vault surface area must provide ventilation to the vault interior (see design specifications).
	Vault Structure Damaged; Includes Cracks in Walls, Bottom, Damage to Frame and/or Top Slab.	Cracks wider than 1/2-inch or evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determine that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or evidence of soil particles entering through the cracks.	Vault replaced or repairs made so that vault meets design specifications and is structurally sound. Vault repaired so that no cracks exist wider than 1/4-inch at the joint of the inlet/outlet pipe.
	Baffles/Internal walls	Baffles or walls corroding, cracking, warping and/or showing signs of failure as determined by maintenance/inspection person.	Baffles repaired or replaced to specifications.
Access Ladder Damaged	Ladder is corroded or deteriorated, not functioning properly, not securely attached to structure wall, missing rungs, cracks, and misaligned.	Ladder replaced or repaired to specifications, and is safe to use as determined by inspection personnel.	

**Table V-A.15: Maintenance Standards - Manufactured Media Filters**

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Below Ground	Sediment Accumulation on Media.	Sediment depth exceeds 0.25-inches.	No sediment deposits which would impede permeability of the



**Table V-A.15: Maintenance Standards - Manufactured Media Filters (continued)**

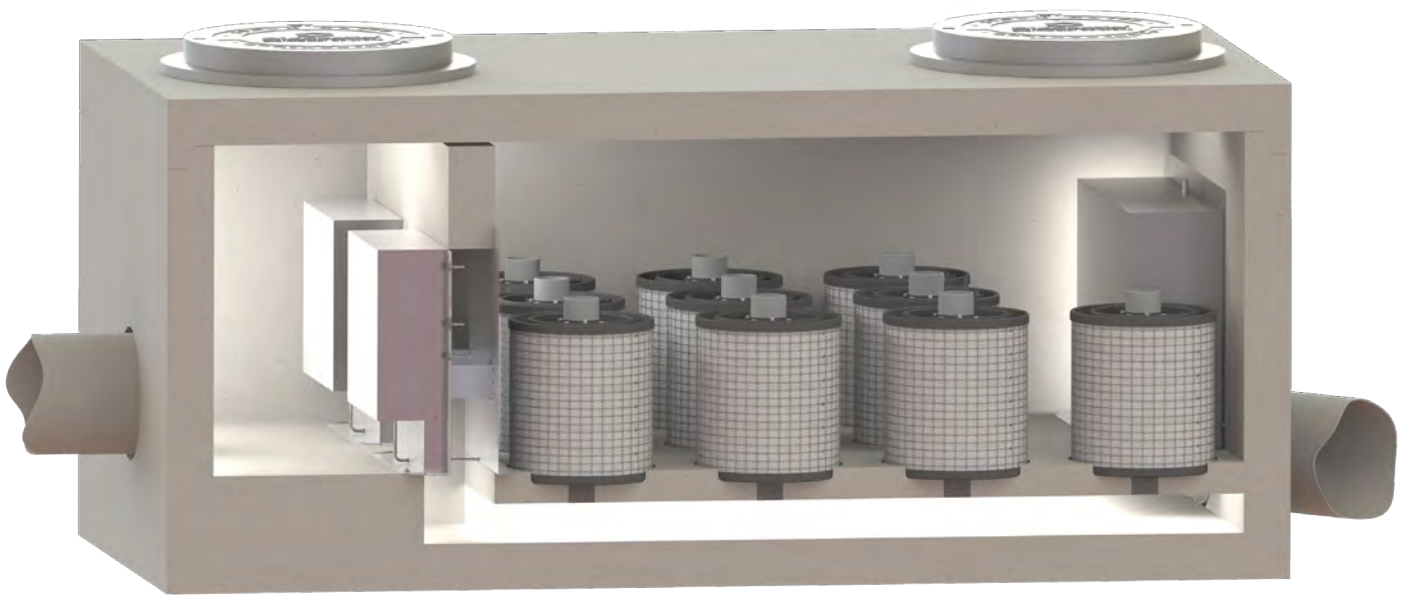
Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Vault			compost media.
	Sediment Accumulation in Vault	Sediment depth exceeds 6-inches in first chamber.	No sediment deposits in vault bottom of first chamber.
	Trash/Debris Accumulation	Trash and debris accumulated on compost filter bed.	Trash and debris removed from the compost filter bed.
	Sediment in Drain Pipes/Clean-Outs	When drain pipes, clean-outs, become full with sediment and/or debris.	Sediment and debris removed.
	Damaged Pipes	Any part of the pipes that are crushed or damaged due to corrosion and/or settlement.	Pipe repaired and/or replaced.
	Access Cover Damaged/Not Working	Cover cannot be opened; one person cannot open the cover using normal lifting pressure, corrosion/deformation of cover.	Cover repaired to proper working specifications or replaced.
	Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab	Cracks wider than 1/2-inch or evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determine that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or evidence of soil particles entering through the cracks.	Vault replaced or repairs made so that vault meets design specifications and is structurally sound. Vault repaired so that no cracks exist wider than 1/4-inch at the joint of the inlet/outlet pipe.
	Baffles	Baffles corroding, cracking warping, and/or showing signs of failure as determined by maintenance/inspection person.	Baffles repaired or replaced to specifications.
Access Ladder Damaged	Ladder is corroded or deteriorated, not functioning properly, not securely attached to structure wall, missing rungs, cracks, and misaligned.	Ladder replaced or repaired and meets specifications, and is safe to use as determined by inspection personnel.	
Below Ground Cartridge Type	Media	Drawdown of water through the media takes longer than 1 hour, and/or overflow occurs frequently.	Media cartridges replaced.
	Short Circuiting	Flows do not properly enter filter cartridges.	Filter cartridges replaced.

# PERKFILTER<sup>®</sup>

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## Submittal Package

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**1 - Submittal Drawing**

**2 - Supplemental Details**

**3 - Features & Benefits**

**4 - Inspection & Maintenance**

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# SECTION 1

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## **Submittal Drawing**

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# SECTION 2

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## **Supplemental Details**

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# SECTION 3

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## **Features & Benefits**



# STORM WATER

## MEDIA Filtration



### Cartridge Filtration Proven to Reduce Pollutant Loading in Runoff from Urban Developments

#### Flexible Configurations

Available in vaults, manholes and catch basins with variable inlet/outlet locations.

#### Superior Flow Rates

High-efficiency treatment in a compact footprint.

#### Field & Laboratory Tested

Removal of Total Suspended Solids (TSS) and phosphorus.

#### Internal High-Flow Bypass

Integrated bypass system reduces construction costs by eliminating the need for a separate bypass structure.

#### Integral Pre-Treatment

Pre-treatment chamber prolongs media lifespan by removing gross pollutants.

#### Modular Cartridge Construction

Simple design provides for efficient media replacement and cartridge handling.

#### Washington State Department of Ecology

- TAPE/GULD for basic treatment (TSS) and phosphorus treatment

#### New Jersey Department of Environmental Protection

- NJCAT certified 80% TSS removal rate

#### Maryland Department of the Environment

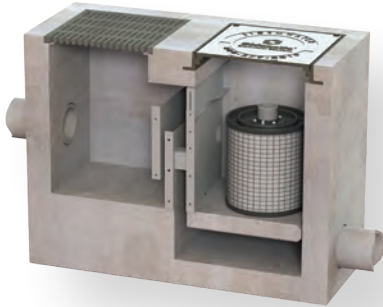
- Approved structural practice for 80% TSS removal

#### Virginia Department of Environmental Quality

- BMP clearinghouse 50% credit for phosphorus reduction

Impervious surfaces and other urban and suburban landscapes generate a variety of contaminants that can enter stormwater, polluting downstream receiving waters. The PerkFilter is a stormwater treatment device that utilizes a wide variety of proprietary media to treat specific pollutants of concern.

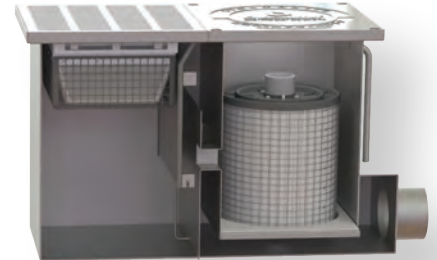
Captures and Retains Target Pollutants, Reducing the Total Downstream Discharge Load



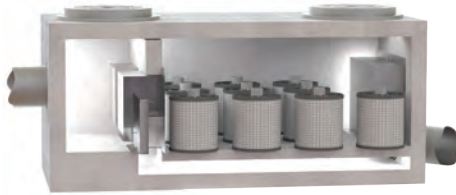
Concrete Catch Basin



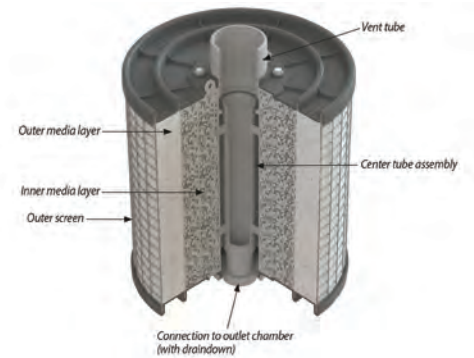
Manhole Configuration



Steel Catch Basin



Vault Configuration



STANDARD CARTRIDGE CAPACITIES		
Cartridge Height Inches	Treatment Capacity (gpm) at Media Surface Loading Rate of:	
	1.5 gpm/ft <sup>2</sup> **	2.5 gpm/ft <sup>2</sup> **
12	6.8	12
18	10.2	18
*24	13.6	24
*30	17	30

\*Standard cartridge heights are 12 & 18 inches.

24 & 30 inch cartridge use modular stacks.

\*\*Depending on regulatory sizing.

## APPLICATIONS

Typical installation locations include:

- Drop inlets or vaults in commercial or residential developments
- Industrial applications
- Pre- or post-treatment for retention/detention systems

## PERFORMANCE

Field-tested removal efficiencies of:

- Total Suspended Solids > 80%
- Total Phosphorus > 60%



(800) 579-8819

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# SECTION 4

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## **Inspection & Maintenance**

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# PERKFILTER®

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## Inspection and Maintenance Guide

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# PerkFilter Media Filtration System

## Description

The PerkFilter is a stormwater treatment device used to remove pollutants from urban runoff. Impervious surfaces and other urban and suburban landscapes generate a variety of contaminants that can enter stormwater and pollute downstream receiving waters. The PerkFilter is a media-filled cartridge filtration device designed to capture and retain sediment, gross solids, metals, nutrients, hydrocarbons, and trash and debris. As with any stormwater treatment system, the PerkFilter requires periodic maintenance to sustain optimum system performance.

## Function

The PerkFilter is a water quality treatment system consisting of three chambers: an inlet chamber, a filter cartridge treatment chamber, and an outlet chamber (Figure 1). Stormwater runoff enters the inlet chamber through an inlet pipe, curb opening, or grated inlet. Gross solids are settled out and floating trash and debris are trapped in the inlet chamber. Pretreated flow is then directed to the treatment chamber through an opening in the baffle wall between the inlet chamber and treatment chamber. The treatment chamber contains media-filled filter cartridges (Figure 2) that use physical and chemical processes to remove pollutants. During a storm event, runoff pools in the treatment chamber before passing radially through the cylindrical cartridges from the outside surface, through the media for treatment, and into the center of the cartridge. At the center of the cartridge is a center tube assembly designed to distribute the hydraulic load evenly across the surface of the filter cartridge and control the treatment flow rate. The center tube assembly discharges treated flow through the false floor and into the outlet chamber. A draindown feature built into each cartridge allows the treatment chamber to dewater between storm events.

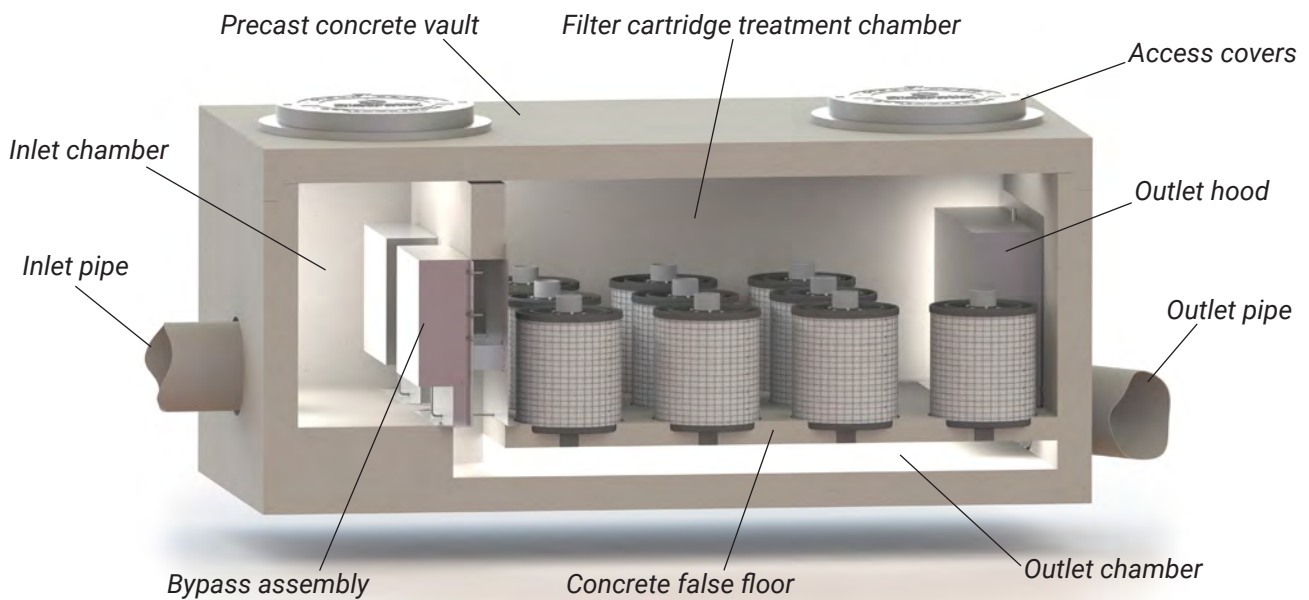
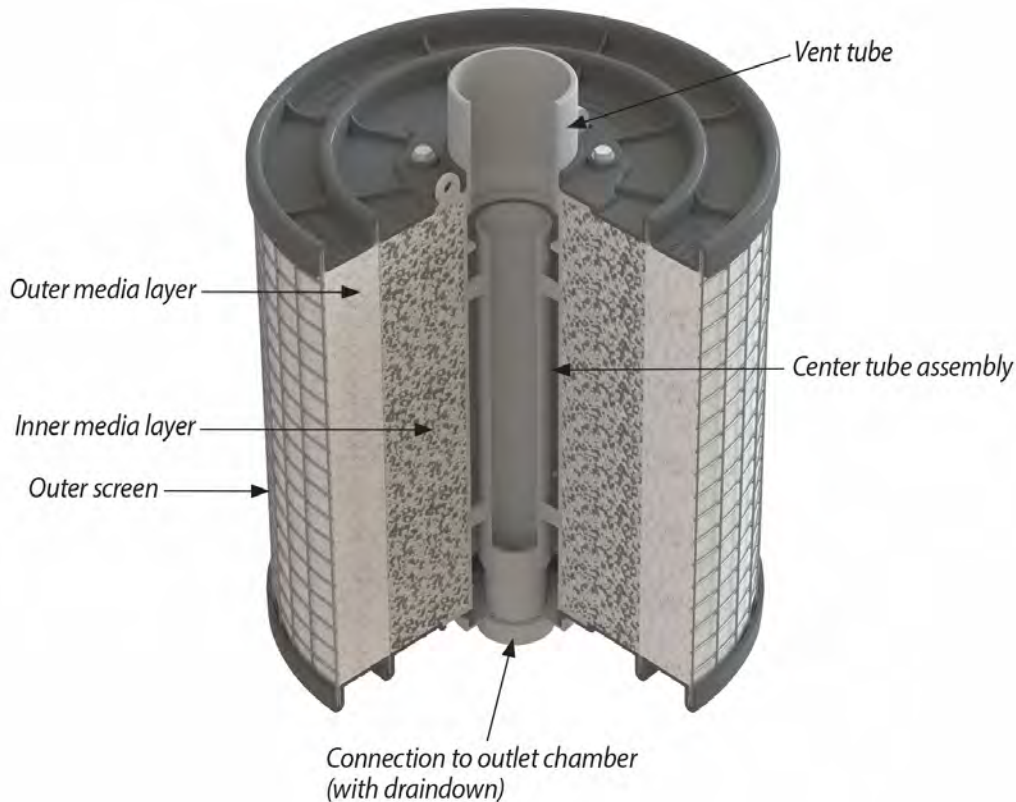


Figure 1. Schematic of the PerkFilter System.

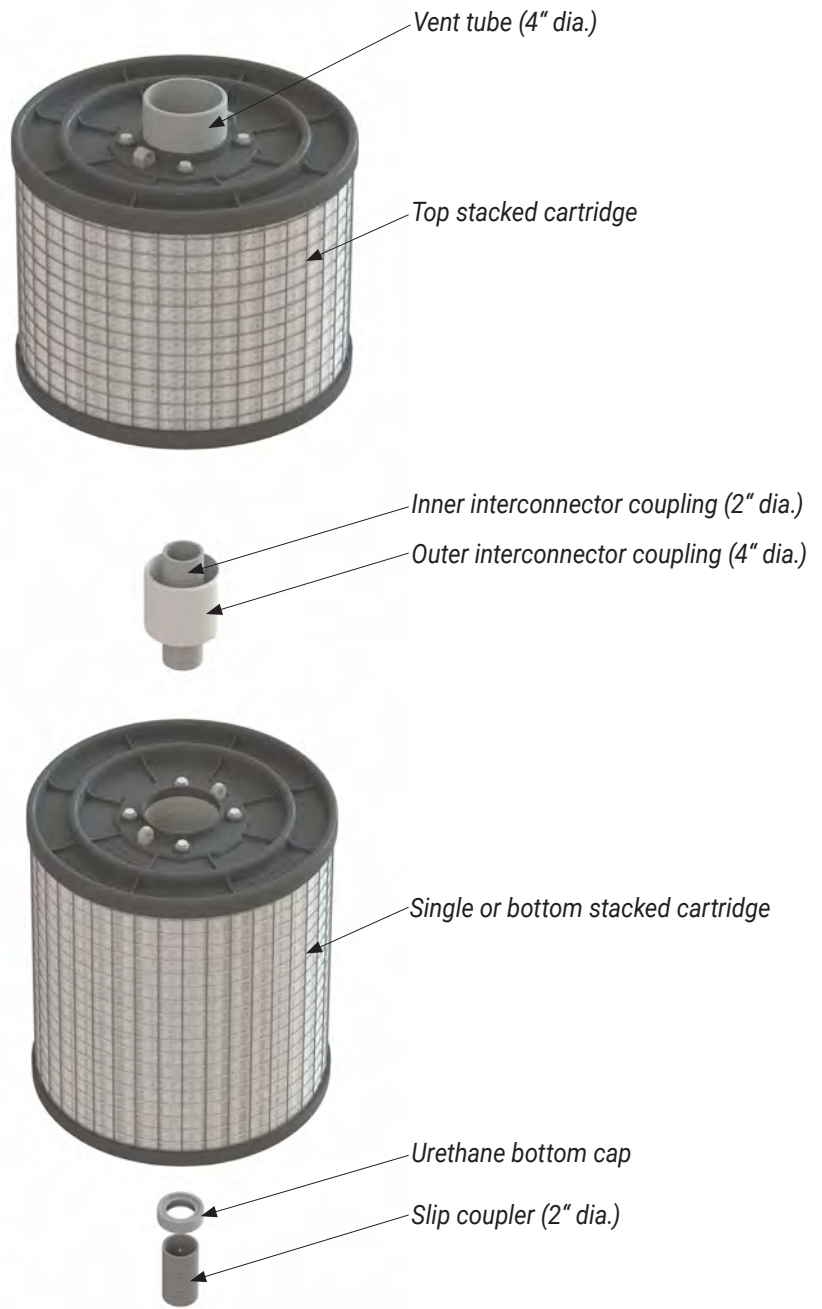
All PerkFilter systems include a high flow bypass assembly to divert flow exceeding the treatment capacity of the filter cartridges around the treatment chamber. The bypass assembly routes peak flow from the inlet chamber directly to the outlet chamber, bypassing the treatment chamber to prevent sediment and other captured pollutants from being scoured and re-entrained by high flow. Treated flow and bypass flow merge in the outlet chamber for discharge by a single outlet pipe.



## Configuration

The PerkFilter structure may consist of a vault, manhole, or catch basin configuration. Catch basin units may be fabricated from concrete or steel. Internal components including the PerkFilter cartridges are manufactured from durable plastic and stainless steel components and hardware. All cartridges are 18 inches in diameter and are available in two heights: 12-inch and 18-inch. Cartridges may be used alone or may be stacked (Figure 3) to provide 24-inch and 30-inch combinations. The capacity of each cartridge or cartridge combination is dictated by the allowable operating rate of the media and the outer surface area of the cartridge. Thus, taller cartridges have greater treatment capacity than shorter cartridges but they also require more hydraulic drop across the system. Cartridges may be filled with a wide variety of media but the standard mix is composed of zeolite, perlite and carbon (ZPC).

Access to an installed PerkFilter system is typically provided by ductile iron castings or hatch covers. The location and number of access appurtenances is dependent on the size and configuration of the system.



**Figure 3. Schematic of stacked cartridges and connector components.**

## Maintenance Overview

State and local regulations require all stormwater management systems to be inspected on a periodic basis and maintained as necessary to ensure performance and protect downstream receiving waters. Maintenance prevents excessive pollutant buildup that can limit system performance by reducing the operating capacity and increasing the potential for scouring of pollutants during periods of high flow.

## Inspection and Maintenance Frequency

The PerkFilter should be inspected on a periodic basis, typically twice per year, and maintained as required. Initially, inspections of a new system should be conducted more frequently to help establish an appropriate site-specific inspection frequency. The maintenance frequency will be driven by the amount of runoff and pollutant loading encountered by a given system. In most cases, the optimum maintenance interval will be one to three years. Inspection and maintenance activities should be performed only during dry weather periods.

## Inspection Equipment

The following equipment is helpful when conducting PerkFilter inspections:

- Recording device (pen and paper form, voice recorder, iPad, etc.)
- Suitable clothing (appropriate footwear, gloves, hardhat, safety glasses, etc.)
- Traffic control equipment (cones, barricades, signage, flagging, etc.)
- Socket and wrench for bolt-down access covers
- Manhole hook or pry bar
- Flashlight
- Tape measure
- Measuring stick or sludge sampler
- Long-handled net (optional)

## Inspection Procedures

PerkFilter inspections are visual and may be conducted from the ground surface without entering the unit. To complete an inspection, safety measures including traffic control should be deployed before the access covers are removed. Once the covers have been removed, the following items should be checked and recorded (see form provided at the end of this document) to determine whether maintenance is required:

- Inspect the internal components and note whether there are any broken or missing parts. In the unlikely event that internal parts are broken or missing, contact Oldcastle Infrastructure at (800) 579-8819 to determine appropriate corrective action.
  - Note whether the inlet pipe is blocked or obstructed. The outlet pipe is covered by a removable outlet hood and cannot be observed without entering the unit.
  - Observe, quantify and record the accumulation of floating trash and debris in the inlet chamber. The significance of accumulated floating trash and debris is a matter of judgment. A long-handled net may be used to retrieve the bulk of trash and debris at the time of inspection if full maintenance due to accumulation of floating oils or settled sediment is not yet warranted.
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- Observe, quantify and record the accumulation of oils in the inlet chamber. The significance of accumulated floating oils is a matter of judgment. However, if there is evidence of an oil or fuel spill, immediate maintenance by appropriate certified personnel is warranted.
- Observe, quantify and record the average accumulation of sediment in the inlet chamber and treatment chamber. A calibrated dipstick, tape measure, or sludge sampler may be used to determine the amount of accumulated sediment in each chamber. The depth of sediment may be determined by calculating the difference between the measurement from the rim of the PerkFilter to the top of the accumulated sediment and the measurement from the rim of the PerkFilter to the bottom of the PerkFilter structure. Finding the top of the accumulated sediment below standing water takes some practice and a light touch, but increased resistance as the measuring device is lowered toward the bottom of the unit indicates the top of the accumulated sediment.
- Finally, observe, quantify and record the amount of standing water in the treatment chamber around the cartridges. If standing water is present, do not include the depth of sediment that may have settled out below the standing water in the measurement.

## **Maintenance Triggers**

Maintenance should be scheduled if any of the following conditions are identified during the inspection:

- Internal components are broken or missing.
- Inlet piping is obstructed.
- The accumulation of floating trash and debris that cannot be retrieved with a net and/or oil in the inlet chamber is significant.
- There is more than 6" of accumulated sediment in the inlet chamber.
- There is more than 4" of accumulated sediment in the treatment chamber.
- There is more than 4" of standing water in the treatment chamber more than 24 hours after end of rain event.
- A hazardous material release (e.g. automotive fluids) is observed or reported.
- The system has not been maintained for 3 years (wet climates) to 5 years (dry climates).

## **Maintenance Equipment**

The following equipment is helpful when conducting PerkFilter maintenance:

- Suitable clothing (appropriate footwear, gloves, hardhat, safety glasses, etc.)
- Traffic control equipment (cones, barricades, signage, flagging, etc.)
- Socket and wrench for bolt-down access covers
- Manhole hook or pry bar
- Confined space entry equipment, if needed
- Flashlight
- Tape measure
- 9/16" socket and wrench to remove hold-down struts and filter cartridge tops
- Replacement filter cartridges
- Vacuum truck with water supply and water jet

Contact Oldcastle Infrastructure at (800) 579-8819 for replacement filter cartridges. A lead time of four weeks is recommended.

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## Maintenance Procedures

Maintenance should be conducted during dry weather when no flow is entering the system. Confined space entry is necessary to maintain vault and manhole PerkFilter configurations. Only personnel that are OSHA Confined Space Entry trained and certified may enter underground structures. Confined space entry is not required for catch basin PerkFilter configurations. Once safety measures such as traffic control are deployed, the access covers may be removed and the following activities may be conducted to complete maintenance:

- Remove floating trash, debris and oils from the water surface in the inlet chamber using the extension nozzle on the end of the boom hose of the vacuum truck. Continue using the vacuum truck to completely dewater the inlet chamber and evacuate all accumulated sediment from the inlet chamber. Some jetting may be required to fully remove sediment. The inlet chamber does not need to be refilled with water after maintenance is complete. The system will fill with water when the next storm event occurs.
  - Remove the hold-down strut from each row of filter cartridges and then remove the top of each cartridge (the top is held on by four 9/16" bolts) and use the vacuum truck to evacuate the spent media. When empty, the spent cartridges may be easily lifted off their slip couplers and removed from the vault. The couplers may be left inserted into couplings cast into the false floor to prevent sediment and debris from being washed into the outlet chamber during washdown.
  - Once all the spent cartridges have been removed from the structure, the vacuum truck may be used to evacuate all accumulated sediment from the treatment chamber. Some jetting may be required to fully remove sediment. Take care not to wash sediment and debris through the openings in the false floor and into the outlet chamber. All material removed from the PerkFilter during maintenance including the spent media must be disposed of in accordance with local, state, and/or federal regulations. In most cases, the material may be handled in the same manner as disposal of material removed from sumped catch basins or manholes.
  - Place a fresh cartridge in each cartridge position using the existing slip couplers and urethane bottom caps. If the vault is equipped with stacked cartridges, the existing outer and inner interconnector couplers must be used between the stacked cartridges to provide hydraulic connection. Transfer the existing vent tubes from the spent cartridges to the fresh cartridges. Finally, refit the struts to hold the fresh cartridges in place.
  - Securely replace access covers, as appropriate.
  - Make arrangements to return the empty spent cartridges to Oldcastle Infrastructure.
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# PerkFilter Inspection and Maintenance Log

Location \_\_\_\_\_

**Structure Configuration and Size:**

Inspection Date \_\_\_\_\_

- Vault \_\_\_\_feet x \_\_\_\_feet
- Manhole \_\_\_\_feet diameter
- Catch Basin \_\_\_\_feet x \_\_\_\_feet

**Number and Height of Cartridge Stacks:**

**Media Type:**

Count\_\_\_\_each  12"  18"  24"  30"

ZPC  Perlite  Other \_\_\_\_\_

***Condition of Internal Components***

Notes:

- Good
- Damaged
- Missing

***Inlet or Outlet Blockage or Obstruction***

Notes:

- Yes
- No

***Floating Trash and Debris***

Notes:

- Significant
- Not Significant

***Floating Oils***

Notes:

- Significant
- Not Significant
- Spill

***Sediment Depth in Inlet Chamber***

Notes:

Inches of Sediment: \_\_\_\_\_

***Sediment Depth in Treatment Chamber***

Notes:

Inches of Sediment: \_\_\_\_\_

***Standing Water in Treatment Chamber***

Notes:

Inches of Standing Water: \_\_\_\_\_

***Maintenance Required***

- Yes - Schedule Maintenance
- No - Inspect Again in \_\_\_\_\_ Months

# PERKFILTER®

## OUR MARKETS



**BUILDING  
STRUCTURES**



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**TRANSPORTATION**