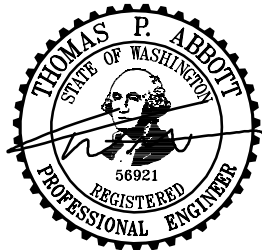


Taylor Lane

Preliminary Drainage Report

Presented to: City of Marysville



1/26/2024



Taylor Lane

Drainage Report

Prepared for
Cornerstone Homes
Contact: Joe Long
13805 Smokey Point Blvd #102
Marysville, WA 98271

Prepared by



Cooper Danby

Approved by
Tom Abbott, PE

Table of Contents

Section	Title	
1	Project Overview	1-1
2	Temporary Erosion and Sediment Control Design	2-1
3	Downstream Analysis	3-1
4	Detention and Water Quality Treatment Design	4-1
5	Conveyance Analysis and Design	5-1
6	Operations and Maintenance Manual	6-1
7	Special Reports and Studies	7-1

Appendices

#	Title
1	Project Overview
2	Temporary Erosion and Sediment Control Design
3	Downstream Analysis
4	Detention and Water Quality Treatment Design
5	Conveyance Analysis and Design
6	Operations and Maintenance Manual
7	Special Reports and Studies

Figures

#	Title
1	Vicinity Map
2	Existing Conditions Map
3	Downstream Analysis Map
4	Predeveloped Hydrology Map
5	Developed Hydrology Map

SECTION 1: PROJECT OVERVIEW

The proposed Taylor Lane project is approximately a 4.66-acre site. The project is a single-family residential development on parcel #00590700013600 and addressed at 4820 83rd Ave NE Marysville 98270. The project proposes to construct 20 new single-family lots and retain the existing single-family home for a total of 21 lots. Emergency and standard access drives along with associated private and public utilities are proposed to serve project development from 49th St NE. Frontage improvements along 49th St NE and 83rd Ave NE consisting of sidewalk improvements to City standards are proposed. See the Vicinity Map in Appendix 1 for visual representation of the subject property.

Existing Site

The parcel #00590700013600 is currently occupied by one single family home with associated outbuildings taking access from 83rd Ave NE. The northeast corner of the site containing the existing home consists of lawn coverage and gravel driveways, while the rest of the site contains dense forest and vegetative coverage. Existing topography generally descends to the south, with some areas of steep slopes, towards a wetland located in the lower-middle region. The project parcel is currently zoned R-6.5 in the City of Marysville.

The proposed development will exist within the bounds of a single stormwater threshold discharge basin as all site runoff from developed surfaces meets within ¼ mile from the project site downstream. Wetland Resources Inc has prepared a Critical Areas Report dated January 2024 on the site. The findings indicate that in the southerly portion of the site, there is a Category II wetland with a 100' buffer. Impacts are planned within portions of these buffer areas, requiring averaging and mitigation. Additionally, stormwater is proposed to be discharged to the wetland area on the parcel via a dispersion trench.

A preliminary Geotechnical Engineering Design Study has been prepared by Earth Solutions NW in January 2024 on the site. Please reference the geotechnical report for detailed soils information. There are a number of steep slopes on the site, as the parcel generally slopes from north to south. Per the Geotechnical Engineer's recommendations, the steep slopes are proposed to be graded and walls constructed to support construction and development activities on the project site. Please see the submitted geotechnical report for addition discussion of the onsite soils and project grading proposal.

Proposed Development

The proposed project will construct 20 new single-family lots and retain the existing home onsite. Emergency and standard access drives will serve the site from 49th St NE, along with associated private and public utilities are proposed to serve project development. Frontage improvements along 49th St NE and 83rd Ave NE consisting of sidewalk improvements to City standards are proposed. Retaining walls will be required along the southern side of most lots bordering the wetland buffer area.

Proposed Drainage System

This project is designed to comply with the 2019 Department of Ecology Stormwater Manual for Western Washington (2019 DOE SWMMWW). Stormwater will be mitigated via a detention vault that is proposed in the southwestern portion of the site. Prior to discharge, a perfilter cartridge filtration unit will be used to treat stormwater runoff to meet basic water quality treatment requirements. The proposed detention vault and water quality treatment system will discharge to the existing onsite wetland area which continues offsite to the south until joining King Creek.

Onsite development will create 2.19 AC of new impervious surfaces which will be collected by the detention vault for mitigation and stormwater quality treatment. This area is considered to be within the Onsite Basin for stormwater modeling. Frontage improvements consisting of new sidewalk and planter along 49th St NE and 83rd Ave NE, referred to as the Frontage Basin, will also be collected and routed to the detention vault to the maximum extent feasible. The runoff from these surfaces unable to be collected due to vertical constraints, approximately 0.02 AC, will be considered to bypass detention and be within the Frontage Bypass Basin.

Erosion/Sedimentation Control

Erosion control measures that will be utilized during construction will include a combination of silt fence, storm drain inlet protection, interceptor swales, and sediment ponds. See Section 2.0 for discussion of how SWPPP Elements are addressed.

Minimum Requirements

Per the 2019 DOE Manual, Minimum Requirements 1-9 apply to the proposed development.

Minimum Requirement #1: Preparation of Stormwater Site Plans

This report along with the preliminary plans satisfies the minimum requirement.

Minimum Requirement #2: Construction Stormwater Pollution Prevention

See Section 2 of this Report for the SWPPP BMP Elements, and the SWPPP (submitted as a separate document) for a complete discussion of erosion control BMP's and their use specific to the site.

Minimum Requirement #3: Source of Pollution

Permanent source control BMPs are not applicable for the subject site since the associated activities for the new residence do not fall within the types of facilities listed within Volume IV of the DOE Manual (Residential developments are not required to implement source control BMP's). BMPs for erosion and sedimentation control will be specified in the Construction Plans and the SWPPP.

Minimum Requirement #4: Preservation of Natural Drainage Systems and Outfalls

Flow from the site will preserve its natural drainage pattern south toward the onsite wetland. Runoff flows towards King Creek, a tributary to Ebey Slough, which then eventually discharges into The Puget Sound.

Minimum Requirement #5: On-Site Stormwater Management

The project proposes BMP T5.13 soils to be underlain within all pervious areas that are disturbed by development. Generally, all other infiltration-related BMPs are infeasible due to the site development area's adjacent location to steep slopes. Per the geotechnical report other dispersion-related BMPs are considered infeasible due to the proximity of the steep slopes to the developed impervious coverage of the site and the lack of available dispersion length. Please see Section 4.5 for additional discussion of Onsite Stormwater Management and Low Impact Development BMP feasibility.

Minimum Requirement #6: Runoff Treatment

Proposed new pollution generating impervious surfaces (PGIS) will exceed the 5,000 SF threshold and thus basic water quality treatment will be provided via a water quality treatment structure that treats stormwater runoff discharging from the stormwater detention vault. A Perfilter cartridge filtration unit is proposed for this purpose. See Section 4.0 for additional discussion regarding proposed stormwater management and water quality treatment measures.

Minimum Requirement #7: Flow Control

The project will exceed the 10,000 SF new plus replaced impervious threshold and is required to provide flow control. A concrete detention vault will be installed in the southwestern portion of the project site, accessible via access easement serving Lot 1. This vault will discharge via dispersion trench at historic, mitigated rates towards the existing onsite wetland area. Please see Section 4.0 for additional flow control modeling and parameters for detention sizing.

Minimum Requirement #8: Wetlands Protection

Wetland Resources Inc has prepared a Critical Areas Report dated January, 2024 on the site. The findings indicate that in the southerly portion of the site, there is a Category II wetland with a 100' buffer. Development is anticipated to impact these buffer areas which will require mitigation and buffer averaging.

Minimum Requirement #9: Operation and Maintenance

See Operations and Maintenance in Section 6 of this report.

SECTION 2: TEMPORARY EROSION AND SEDIMENT CONTROL DESIGN

SWPPP Design Elements

A Construction Stormwater Pollution Prevention Plan (SWPPP) will be provided prior to construction. The SWPPP report is modeled under the guidelines of 1.3.4.2 MR2 of the DOE Manual. Construction SWPPP Elements #1 through #13 are addressed below. These elements will be delineated within future civil plans for the project.

Element #1 – Mark Clearing Limits

All clearing limits will be delineated with high visibility plastic fence and/or silt fence.

Element #2 – Establish Construction Access

Stabilized construction accesses will be installed as shown on the plans.

Element #3 – Control Flow Rates

Detention of construction period runoff will be provided by means of sediment ponds on the site.

Element #4 – Install Sediment Controls

Silt fence, catch basin protection, and the temporary sediment pond will be utilized to contain sediments within the project's clearing limits.

Element #5 – Stabilize Soils

Exposed soils will be stabilized as specified in the Grading and Erosion Control Notes with temporary and permanent seeding, mulching, and plastic covering.

Element #6 – Protect Slopes

Slopes shall be protected as specified under Element #5.

Element #7 – Protect Drain Inlets

Storm drain inlet protection will be utilized to contain sediments within the project's clearing limits.

Element #8 – Stabilize Channels and Outlets

Temporary channels, shall be stabilized with check dams.

Element #9 – Control Pollutants

Pollutants shall be controlled as specified in Volume IV of the 2019 DOE Manual—Source Control BMPs to address potential sources of pollution.

Element #10 – Control De-Watering

There will be no de-watering as a part of this project..

Element #11 – Maintain BMPs

Maintenance of the BMPs is specified within the Construction Sequence and Grading and Erosion Control Notes that will be included in the civil plans associated with the project.

Element #12: Manage the Project

The Grading and Erosion Control Notes specify seasonal work limitations. Maintenance of the BMPs is specified within the Construction Sequence and Grading and Erosion Control Notes.

Element #13: Protect on-site stormwater management BMPs

On-site stormwater management BMPs used for runoff from roofs and other hard surfaces are not feasible due to soil conditions and proposed project density.

SECTION 3: DOWNSTREAM ANALYSIS

Task 1. Study Area Definition and Maps

Snohomish County Bare Earth LiDAR, survey, and 2021 aerial photography were the best topographical references available for the area containing the site. The limits of the downstream analysis extend roughly 0.25 miles beyond the subject property's natural discharge location.

Task 2. Resource Review

All of the resources below have been reviewed for existing and potential issues near the project site:

Adopted Basin Plans

No Adopted Basin Plans were located that include the project site.

Drainage Basin

This site is in the King Creek subbasin, within the Ebey Slough watershed. Discharge from the proposed development will discharge into a wetland area tributary to King Creek.

Floodplain / Floodway (FEMA) maps

Per FEMA Floodplain map #53061C0736F the subject property is not within a floodplain.

Critical Areas Map

There's a Category II wetland in the southern portion of the site. There are steep slopes located within the wetland buffer area near the central portion of the site. See section 7.0 for a complete list of reports prepared for the project including a Critical Areas Report, Wetland critical areas, and geotechnical and slope analysis reports.

Drainage Complaints

No relevant issues were identified near the proposed site.

Road Drainage Problems

No issues were identified near the proposed site.

Soil Survey

Site soils are classified as Tokul gravelly medial loam (0 to 8 percent slopes) which is classified as Hydrologic Soil Group B. Per geotechnical analysis, infiltration is infeasible onsite, which is consistent with NRCS soil mapping data.

Wetland Inventory Maps

Wetlands are identified to be on and immediately adjacent to the south of the project site. Reference the critical areas report submitted with this report for additional information regarding the wetland areas onsite.

Migrating River Studies

Migrating River Studies are not considered applicable to the proposed development.

Section 303d List of Polluted Waters

Washington State Department of Ecology's Water Quality Assessment for Washington contains a listing for King Creek which the project is tributary to. Please refer to Appendix 3 for copies of applicable 303(d) listings.

Water Quality Problems

King creek has a category 5 listing in the DOE Water Quality Assessment Review tools for temperature which does not fall under water quality problems. The development however will improve water quality tributary to the creek via basic treatment device(s).

Stormwater Compliance Plans

Not applicable to the proposed project.

Task 3. Field Inspection/Downstream Analysis

On November 27th, 2023, a Downstream Analysis was performed at the site. The weather consisted of 46°F and clear skies. The following observations were verified during the visit.

The subject property contains impervious areas associated with an existing home, driveways, and associated outbuildings surrounded by lawn area in the northeastern portion of the site. Slopes are generally moderate in the developed portion. The rest of the site consists of steep slopes generally descending south with dense forest/vegetation, and an existing wetland and wetland buffer area in the south-central portion. Frontages along 49th St NE (north) and 83rd Ave NE (east) were observed for potential upstream run-on. 49th St does not produce upstream run-on to the site (Images 1 & 2) while 83rd Ave does yield some upstream flow from the existing road centerline (Image 3).

The site is located within a single threshold discharge area (TDA) since all runoff exits the southern property line and converges prior to exiting the quarter mile boundary of analysis. The flow path is formed where runoff enters Wetland A either onsite or offsite to the south. Flow continues south/southwest through several undeveloped properties until ultimately joining King Creek prior to crossing under 44th St NE and continuing beyond the quarter mile boundary of analysis (Images 4 & 5).

Task 4. Drainage System Description and Problem Descriptions

Based on the information available and all the resources available including visual inspection of the downstream flow path to the ¼-mile boundary, there is no evidence of existing or anticipated downstream drainage problems. All flows are adequately carried through natural channels to the quarter mile buffer of analysis.

Task 5. Mitigation of Existing or Potential Drainage Problems

No evidence of existing or potential problems with downstream drainage conveyance infrastructure was found. Mitigation is not required.

SECTION 4: DETENTION AND WATER QUALITY TREATMENT DESIGN

4.1 Predeveloped Site Hydrology

The pre-developed and developed conditions were modeled in WWHM for the purpose of peak flow determination for direct discharge. Based on the site location, the WWHM used the Seatac Gage with a Precipitation Scale factor of 1.20. For visual representation of the listed basins, see the Predeveloped Hydrology Map.

Onsite Basin:

The predeveloped condition applied to the Onsite Basin results in a forested land cover condition. It includes all disturbed areas onsite. The values as modeled in WWHM are as follows:

Table 1: Predeveloped Conditions: Onsite Basin

Onsite Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, steep	2.84
Total	2.84

Frontage Basin:

The predeveloped condition is applied to the Frontage Basin, which includes area within 49th St NE and 83rd Ave NE to be developed. In the developed condition, this basin will be detained and mitigated in the proposed detention vault. The values as modeled in WWHM are as follows:

Table 2: Predeveloped Conditions: Frontage Basin

Frontage Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Forest, flat	0.47
Total	0.47

Upstream Basin:

In the predeveloped condition, a portion of the pavement within the 83rd Ave NE ROW is tributary to the project site. In the developed condition, this pavement will continue to be tributary to the site. This area has been modeled as upstream flow through in WWHM in both the predeveloped and developed conditions. The values as modeled in WWHM are as follows:

Table 3: Predeveloped Conditions: Upstream Basin

Upstream Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Roads, flat	0.09
Total	0.09

4.2 Developed Site Hydrology

In the developed condition, the proposed apartment project will construct 20 new single family lots with proposed ROW access, stormwater detention and utilities. Frontage improvements along 49th St NE and 83rd Ave NE are proposed.

In compliance with the 2019 DOE Manual, all runoff from onsite developed/disturbed surfaces will be collected, treated, and discharged directly to existing/historic flow paths or will bypass detention and be mitigated within the proposed flow control system.

Onsite Basin:

The developed Onsite Basin includes the onsite disturbed area, including all lot areas. The maximum lot impervious value (65%) per zoning code was utilized in determine proposed site impervious coverage. In the developed condition, the Onsite Basin has been modeled using WWHM with the following areas and ground cover designations:

Table 4: Developed Conditions: Onsite Basin

Onsite Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Roof, flat	1.04
Roads, flat	0.36
Driveway, flat	0.35
Sidewalks, flat	0.07
Pasture, flat	0.75
Pasture, mod	0.27
Total	2.84

Frontage Basin:

The developed Frontage Basin is comprised of area within 49th St NE and 83rd Ave NE to be developed for ROW improvements. The Frontage Basin was modeled using WWHM with the following areas and ground cover designations:

Table 5: Developed Conditions: Frontage Basin

Frontage Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Roads, flat	0.25
Sidewalk, flat	0.10
Pasture, flat	0.10
Total	0.45

Frontage Bypass Basin:

The developed Frontage Bypass Basin is comprised of 83rd Ave NE pavement that cannot be collected due to topographical reasons. The Frontage Bypass Basin was modeled using WWHM with the following areas and ground cover designations:

Table 6: Developed Conditions: Frontage Bypass Basin

Frontage Bypass Basin	
<u>Ground Cover</u>	<u>Area (acre)</u>
Road, flat	0.02
Total	0.02

Upstream Basin:

In the predeveloped condition, a portion of the pavement within the 83rd Ave NE ROW is tributary to the project site. In the developed condition, this pavement will continue to be tributary to the site. This area has been modeled as upstream flow through in WWHM in both the predeveloped and developed conditions. The values as modeled in WWHM are as follows:

Table 7: Predeveloped Conditions: Upstream Basin

Upstream Basin	
Ground Cover	Area (acre)
Roads, flat	0.09
Total	0.09

4.3 Detention Facility Design

The proposed detention vault facility used for mitigating developed condition flows was designed in compliance with the 2019 DOE requirements to model hydrologic conditions and detention in a continuous runoff model (WWHM2012) where the following evaluation parameters are employed:

“Flow duration is computed by counting the number of flow values that exceed a specified flow level. The specified flow levels used by WWHM in the flow duration analysis are listed below.

1. 50% of the 2-year predevelopment peak flow.
2. 100% of the 2-year predevelopment peak flow.
3. 100% of the 50-year predevelopment peak flow.

There are three criteria by which flow duration values are compared:

1. *If the postdevelopment flow duration values exceed any of the predevelopment flow levels between 50% and 100% of the 2-year predevelopment peak flow values (100 Percent Threshold) then the flow duration requirement has not been met.*
2. *If the postdevelopment flow duration values exceed any of the predevelopment flow levels between 100% of the 2-year and 100% of the 50-year predevelopment peak flow values more than 10 percent of the time (110 Percent Threshold) then the flow duration requirement has not been met.*
3. *If more than 50 percent of the flow duration levels exceed the 100 percent threshold then the flow duration requirement has not been met.”*

Detention Vault Facility

The proposed cast in place concrete detention facility detains, and releases collected storm water runoff from the Onsite Basin, Upstream Basin and Frontage Basin. The facility is located in Tract 997. Flows from the developed basins are collected and conveyed to the detention vault via a proposed network of catch basins and storm water conveyance pipes. Detailed WWHM output is provided in Appendix 4. A summary of the detailed statistics and inputs used for modeling the system in WWHM2012 can be found below.

Table 8: Detention Vault Design Summary

Detention Vault	
Live Storage Bottom Area (modeled)	6,300 SF
Live Storage Bottom Area (provided)	6,300 SF
Number of Cells	3
Cell Dimensions	(3 x 23.3' x 90)
Begin Live Storage Elevation	336.0
Riser Height	6.00'
Volume (modeled)	37,800 CF
Volume (provided)	37,800 CF

See table below for the flow rates and water surface elevations by storm event for the detention vault.

Table 9: Flow Rates and Water Surface Elevations by Storm Event

Storm Event	Predeveloped Rate (cfs)	Mitigated Rates (cfs)	Water Surface Elevation (ft)
2-Year	0.1952	0.1053	338.52
10-Year	0.4076	0.1723	340.25
50-Year	0.6837	0.2484	340.67
100-Year	0.8320	0.2869	341.01

4.4 Water Quality Treatment

Perkfilter

Water quality treatment for the mitigated site is accomplished through a Perkfilter structure located downstream of the detention vault. A summary of design criteria is provided below:

Table 10: Perkfilter Design Summary

60" Ø Perkfilter Manhole	
Tributary Area	3.38 AC
Tributary PGIS Area	1.05 AC
Water Quality Flow Rate (2 yr mitigated peak)	0.1053 cfs
WQ Treatment Capacity	0.1130 cfs
Number of Cartridges	3
Cartridge Height	12"+18"
Internal Drop	3.5'
Peak Flow Rate	0.2869 cfs
Peak Flow Storm Event	100-year

4.5 Onsite Stormwater Management

The project does not meet the LID performance standard and minimum requirements 1-9 are required for the project but choose to implement List #2 to evaluate low impact design. The following BMP's below are assessed for implementation:

Lawn and Landscaped Areas:

1. *Post-Construction Soil Quality and Depth*
 - BMP T5.13 soils will be applied to all permeable and landscaped areas in developed condition.
 - i. **Conclusion: Feasible**

Roofs:

1. *Downspout Full Infiltration per BMP T5.10A or Downspout Full Dispersion per BMP T5.30*
 - Infiltration is not feasible on site due to the lack of infiltratable soils which has been confirmed by testing found in the geotechnical report and thus BMP T5.10A is infeasible. Due to site specific constraints including building locations as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site per BMP T5.30.
 - i. **Conclusion: Infeasible**
2. *Bioretention*
 - Due to spatial constraints provided by the development footprint and infiltration infeasibility as confirmed by testing in the geotechnical report, a bioretention facility cannot be designed to provide the required horizontally projected surface area.
 - i. **Conclusion: Infeasible**
3. *Downspout Dispersion per BMP T5.10B.*
 - Due to site specific constraints including building location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path length to disperse on site.
 - i. **Conclusion: Infeasible**
4. *Perforated Stub-Out Connections per BMP T5.10C.*
 - No stub-out connections will be implemented in the design as soils are not suitable for infiltration as well as the site's proximity to steep slopes.
 - i. **Conclusion: Infeasible**

Other Hard Surfaces:

1. *Full Dispersion per BMP T5.30*
 - Due to site specific constraints including building as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path to disperse on site.
 - i. **Conclusion: Infeasible**

2. *BMP T5.15 Permeable Pavement*
 - Infiltration is not feasible on site per the Geotechnical Engineer, which has been confirmed by testing found in the geotechnical report.
 - i. **Conclusion: Infeasible**

3. *Bioretention*
 - Due to spatial constraints provide by the development footprint and infiltration infeasibility as confirmed by testing in the geotechnical report, a bioretention facility cannot be designed to provide the required horizontally projected surface area.
 - i. **Conclusion: Infeasible**

4. *Sheet Flow Dispersion or Concentrated Flow Dispersion in accordance with BMP T5.12 or BMP T5.11*
 - Due to site specific constraints including building location as well as the proximity of slopes and walls to the developed site improvements, there is inadequate flow path length to disperse on site.
 - i. **Conclusion: Infeasible**

SECTION 5: CONVEYANCE DESIGN

The stormwater conveyance system is comprised of a network of open/closed grate catch basins, buried pipe, a concrete detention vault and a Perfilter water quality unit. Catch basins have been located such that each section of storm drainage pipe may adequately convey associated tributary area flows.

A fully prepared conveyance capacity analysis of the proposed pipes onsite will be prepared at a future submittal.

SECTION 6: OPERATIONS AND MAINTENANCE MANUAL

The proposed storm drainage system consists of buried pipes, catch basins, a detention vault, and a perfilter water quality treatment structure. These facilities will require periodic maintenance and inspection. Inspection and maintenance procedures are contained on the following pages.

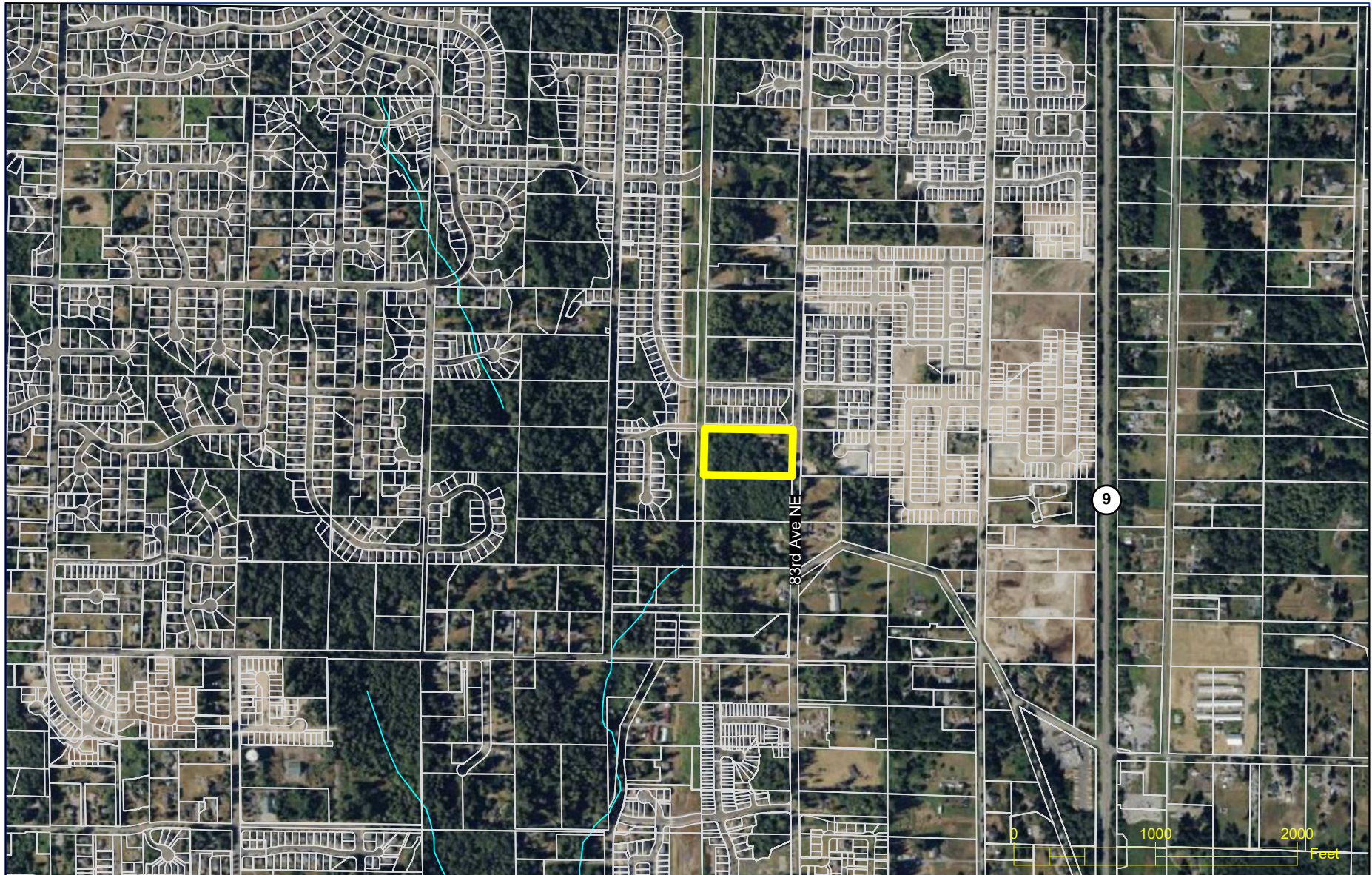
SECTION 7: SPECIAL REPORTS AND STUDIES

The following studies were conducted in preparation of this Report:

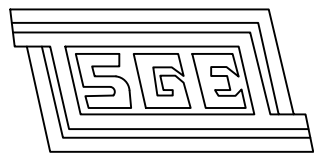
- Critical Areas Report, Wetland Resources, January 2024
- Geotechnical Report, Earth Solutions NW, January 2024

Appendix 1: Project Overview

1. Vicinity Map
2. Existing Conditions Map
3. Proposed Development Map



CORNERSTONE HOMES
 TAYLOR LANE
 VICINITY MAP

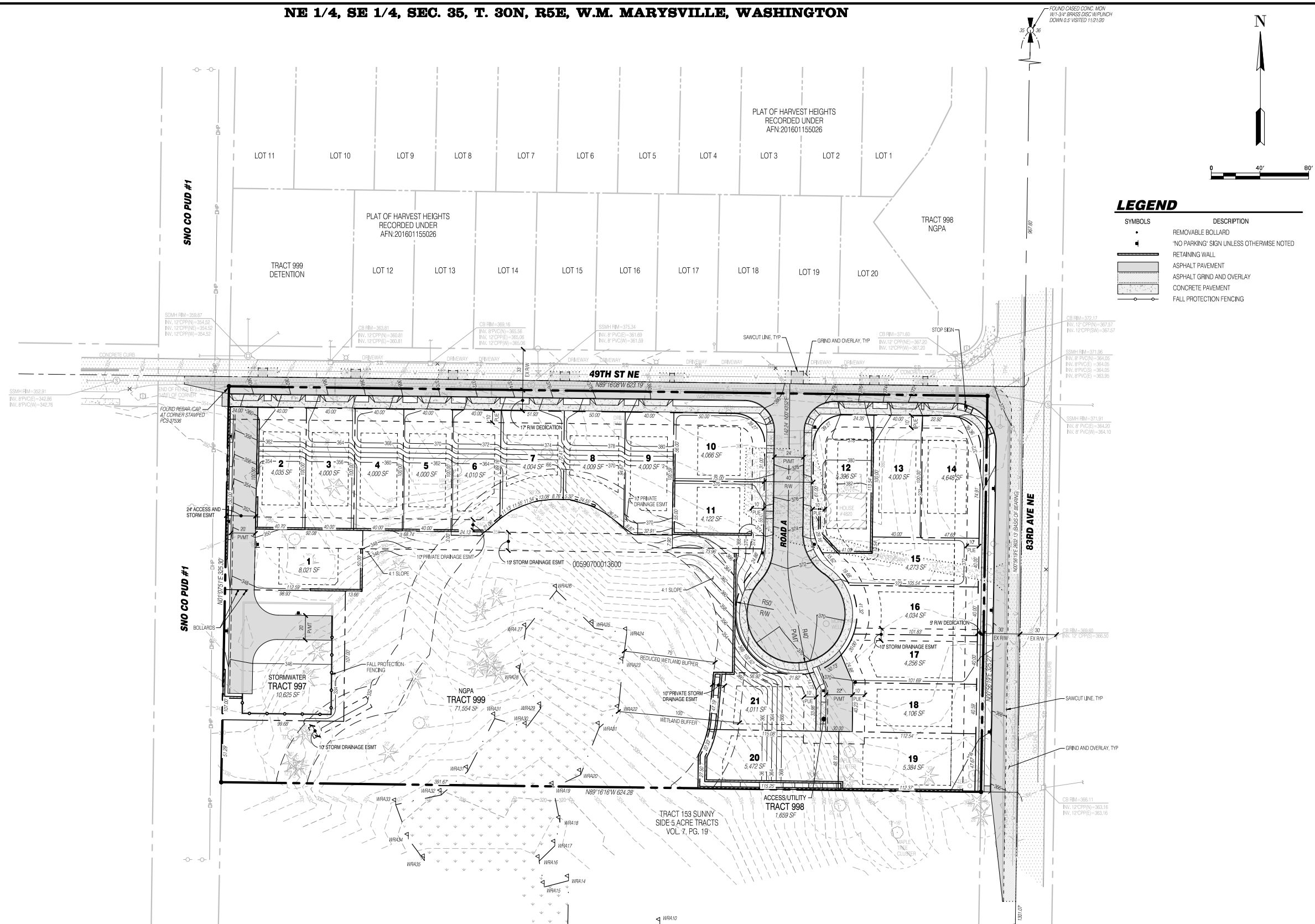


Solid Ground Engineering

8105 166th Ave NE
 Redmond, WA 98052

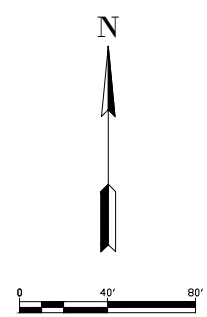
JOB NUMBER:	23-0009	DATE:	1-15-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

NE 1/4, SE 1/4, SEC. 35, T. 30N, R5E, W.M. MARYSVILLE, WASHINGTON



LEGEND

SYMBOLS	DESCRIPTION
(Symbol: Dashed line with dots)	REMOVABLE BOLLARD
(Symbol: Dashed line)	'NO PARKING' SIGN UNLESS OTHERWISE NOTED
(Symbol: Solid line)	RETAINING WALL
(Symbol: Dotted line)	ASPHALT PAVEMENT
(Symbol: Horizontal lines)	ASPHALT GRIND AND OVERLAY
(Symbol: Vertical lines)	CONCRETE PAVEMENT
(Symbol: Dashed line with triangles)	FALL PROTECTION FENCING



PROJECT INFORMATION

TAX PARCELS: 0059070013600
 SITE ADDRESS: 4820 83RD AVE NE, MARYSVILLE
 SITE AREA: 202,889 SF 4.66 AC
 PROPOSED ZONING: R6.5
 PROPOSED LAND USE: DETACHED SINGLE FAMILY HOMES
 FUTURE LAND USE: URBAN LOW RESIDENTIAL (PER COMP PLAN)
 PROPOSED LOTS: 21 LOTS
 BUILDING SETBACKS: 20' FRONT
 9' SIDE / 11' WHEN ADJACENT TO RW
 20' REAR
 10' NGPA
 WATER: CITY OF MARYSVILLE
 SEWER: CITY OF MARYSVILLE
 POWER: SNOHOMISH COUNTY PUD
 GAS: CASCADE NATURAL GAS
 TELEPHONE: COMCAST
 CABLE: COMCAST
 SCHOOL DISTRICT: LAKE STEVENS SCHOOL DISTRICT #4
 FIRE DISTRICT: MARYSVILLE FIRE DISTRICT RFA

SOIL TYPE AND VEGETATIVE COVER

SOILS: TILL SOILS
 VEGETATIVE COVER: PRIMARILY FORESTED

DENSITY CALCULATIONS

ZONE (DENSITY): R-6.5
 GROSS AREA: 202,888 SF (4.66 AC)
 FLAT 20% DEDUCTION: 40,578 SF (0.93 AC)
 NET AREA: 162,310 SF (3.73 AC)
 ALLOWABLE DENSITY: 3.73 x 6.5 DU/AC = 24.25 = 24 LOTS
 LOTS PROPOSED: 20 NEW + 1 EXISTING = 21 LOTS

UTILITY NOTE

THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO ANY CONSTRUCTION. AGENCIES INVOLVED SHALL BE NOTIFIED WITHIN A REASONABLE TIME PRIOR TO THE START OF CONSTRUCTION.

SURVEY DISCLAIMER

THE TOPOGRAPHIC SURVEY WAS PERFORMED BY PACIFIC COAST SURVEYS. SOLID GROUND ENGINEERING ASSUMES NO LIABILITY AS TO THE ACCURACY AND COMPLETENESS OF THIS DATA. ANY DISCREPANCIES FOUND BETWEEN WHAT IS SHOWN ON THE PLANS AND WHAT IS NOTED IN THE FIELD SHOULD BE BROUGHT IMMEDIATELY TO THE ATTENTION OF THE ENGINEER.



ENGINEER'S STAMP

REVISIONS

#	DATE	DESCRIPTION

Solid Ground Engineering
 8105 166th Ave NE
 Redmond, WA 98052

PRELIMINARY PLAT MAP

KEYSTONE LAND, LLC.
TAYLOR LANE
MARYSVILLE, WA

DRAWN BY: JAP
 CHECKED BY: TPA
 DATE: 12-05-23
 JURISDICTION: CITY OF MARYSVILLE
 JOB NUMBER: 23-0009

PP-01
3 OF 8

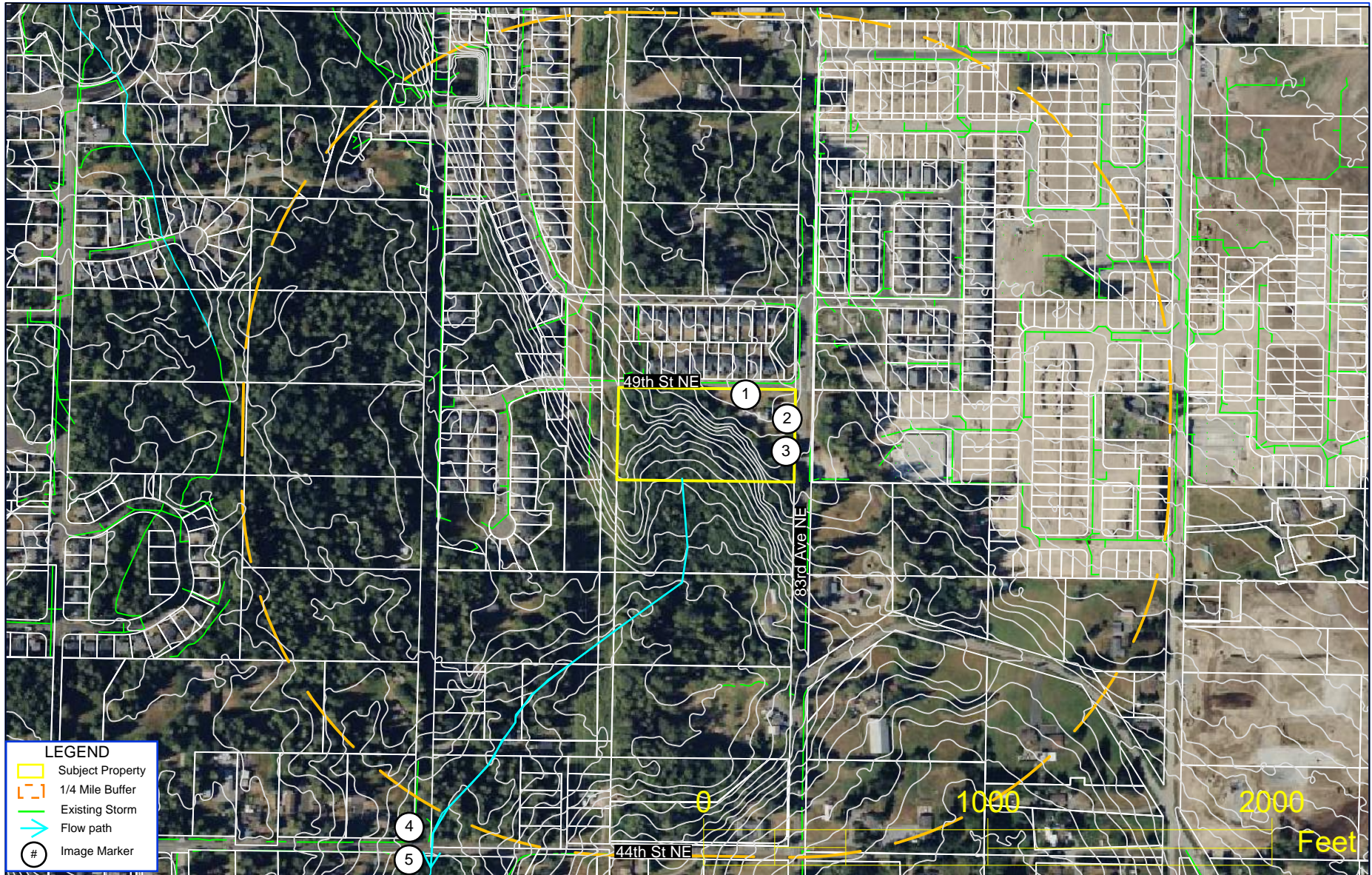
C:\Users\jap\OneDrive\Documents\811\Ground Engineering\23-0009-PP-01.dwg

Appendix 2: Temporary Erosion and Sediment Control Design

1. TESC Plans (to be provided in later submittal)

Appendix 3: Downstream Analysis

1. Downstream Analysis Map
2. Downstream Analysis Site Visit Pictures
3. USDA Soils Map & Description



LEGEND

- Subject Property
- 1/4 Mile Buffer
- Existing Storm
- Flow path
- # Image Marker

CORNERSTONE HOMES
TAYLOR LANE
DOWNSTREAM ANALYSIS MAP



Solid Ground Engineering
8105 166th Ave NE
Redmond, WA 98052

JOB NUMBER:	23-0009	DATE:	1-15-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

C:\Users\CooerDanby\Documents\CAD-GIS\Mapes\Taylor Marysville\Taylor Downstream Map.dwg

Downstream Analysis Photographs



Image 1: 49th St NE frontage, does not contribute upstream run-on



Image 2: 49th St NE and 83rd Ave NE intersection property corner



Image 3: 83rd Ave NE frontage, contributes some upstream run-on due to lack of existing swale/ditch

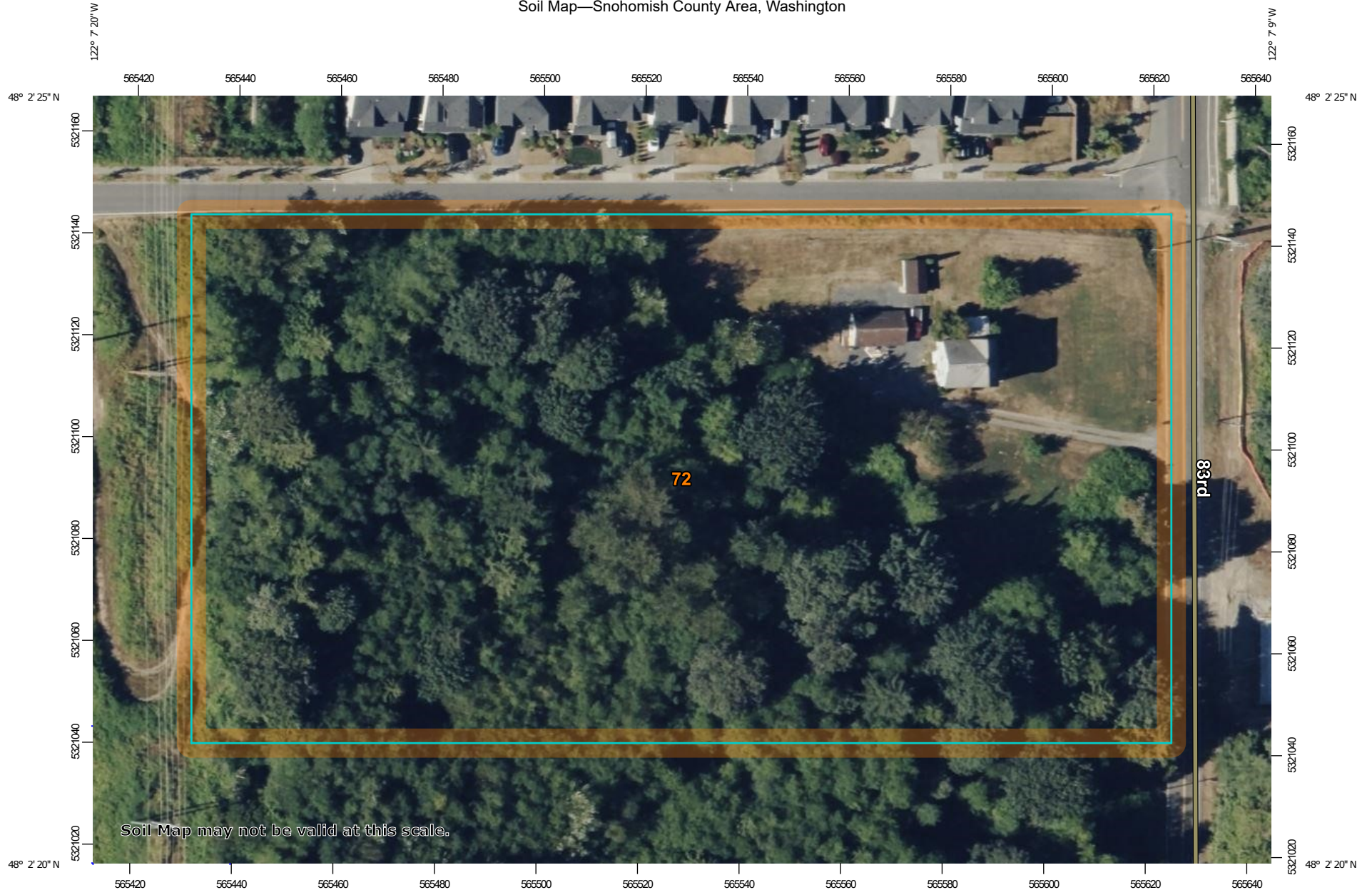


Image 4: North side of 44th St NE, prior to King Creek crossing



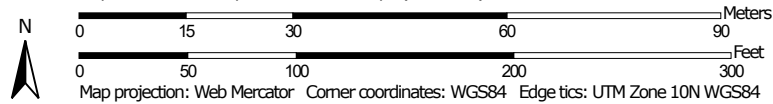
Image 5: South side of 44th St NE, King Creek continues beyond quarter mile boundary

Soil Map—Snohomish County Area, Washington



Soil Map may not be valid at this scale.

Map Scale: 1:1,060 if printed on A landscape (11" x 8.5") sheet.




MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Snohomish County Area, Washington

Survey Area Data: Version 25, Aug 29, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 14, 2022—Sep 1, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
72	Tokul gravelly medial loam, 0 to 8 percent slopes	5.0	100.0%
Totals for Area of Interest		5.0	100.0%

Snohomish County Area, Washington

72—Tokul gravelly medial loam, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2t61k

Elevation: 160 to 1,150 feet

Mean annual precipitation: 45 to 70 inches

Mean annual air temperature: 46 to 52 degrees F

Frost-free period: 140 to 200 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Tokul and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tokul

Setting

Landform: Hillslopes, till plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Side slope, tread

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Volcanic ash mixed with loess over glacial till

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material

Oa - 1 to 2 inches: highly decomposed plant material

A - 2 to 6 inches: gravelly medial loam

Bs1 - 6 to 9 inches: gravelly medial loam

Bs2 - 9 to 17 inches: gravelly medial loam

Bs3 - 17 to 24 inches: gravelly medial loam

BC - 24 to 33 inches: gravelly medial fine sandy loam

2Bsm - 33 to 62 inches: cemented material

Properties and qualities

Slope: 0 to 8 percent

Depth to restrictive feature: 20 to 39 inches to densic material; 20 to 39 inches to cemented horizon

Drainage class: Moderately well drained

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.06 in/hr)

Depth to water table: About 18 to 36 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: B
Ecological site: F002XA005WA - Puget Lowlands Moist Forest
Forage suitability group: Limited Depth Soils (G002XN302WA),
Limited Depth Soils (G002XF303WA)
Other vegetative classification: Limited Depth Soils
(G002XN302WA), Limited Depth Soils (G002XF303WA)
Hydric soil rating: No

Minor Components

Pastik

Percent of map unit: 5 percent
Landform: Terraces
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Barneston

Percent of map unit: 5 percent
Landform: Moraines, eskers, kames
Landform position (two-dimensional): Summit, shoulder
Landform position (three-dimensional): Interfluve, crest
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Norma

Percent of map unit: 3 percent
Landform: Drainageways, depressions
Landform position (three-dimensional): Dip
Down-slope shape: Linear, concave
Across-slope shape: Concave
Hydric soil rating: Yes

Mckenna

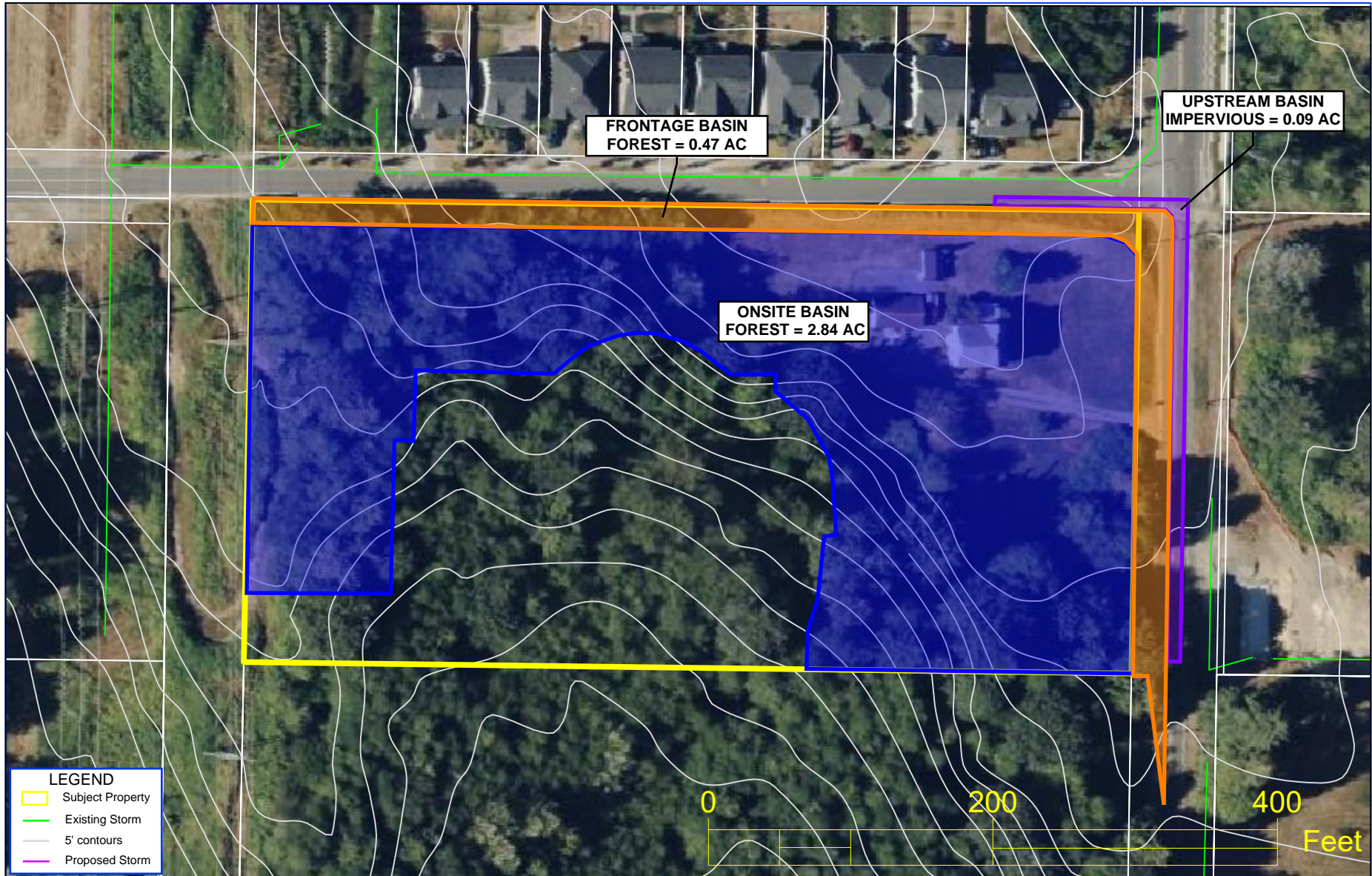
Percent of map unit: 2 percent
Landform: Drainageways, depressions
Landform position (three-dimensional): Dip
Down-slope shape: Linear, concave
Across-slope shape: Concave
Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Snohomish County Area, Washington
Survey Area Data: Version 25, Aug 29, 2023

Appendix 4: Detention and Water Quality Design Analysis

1. Predeveloped Hydrology Map
2. Developed Hydrology Map
3. Perfilter Detail
4. WWHM2012 Output – Detention Vault

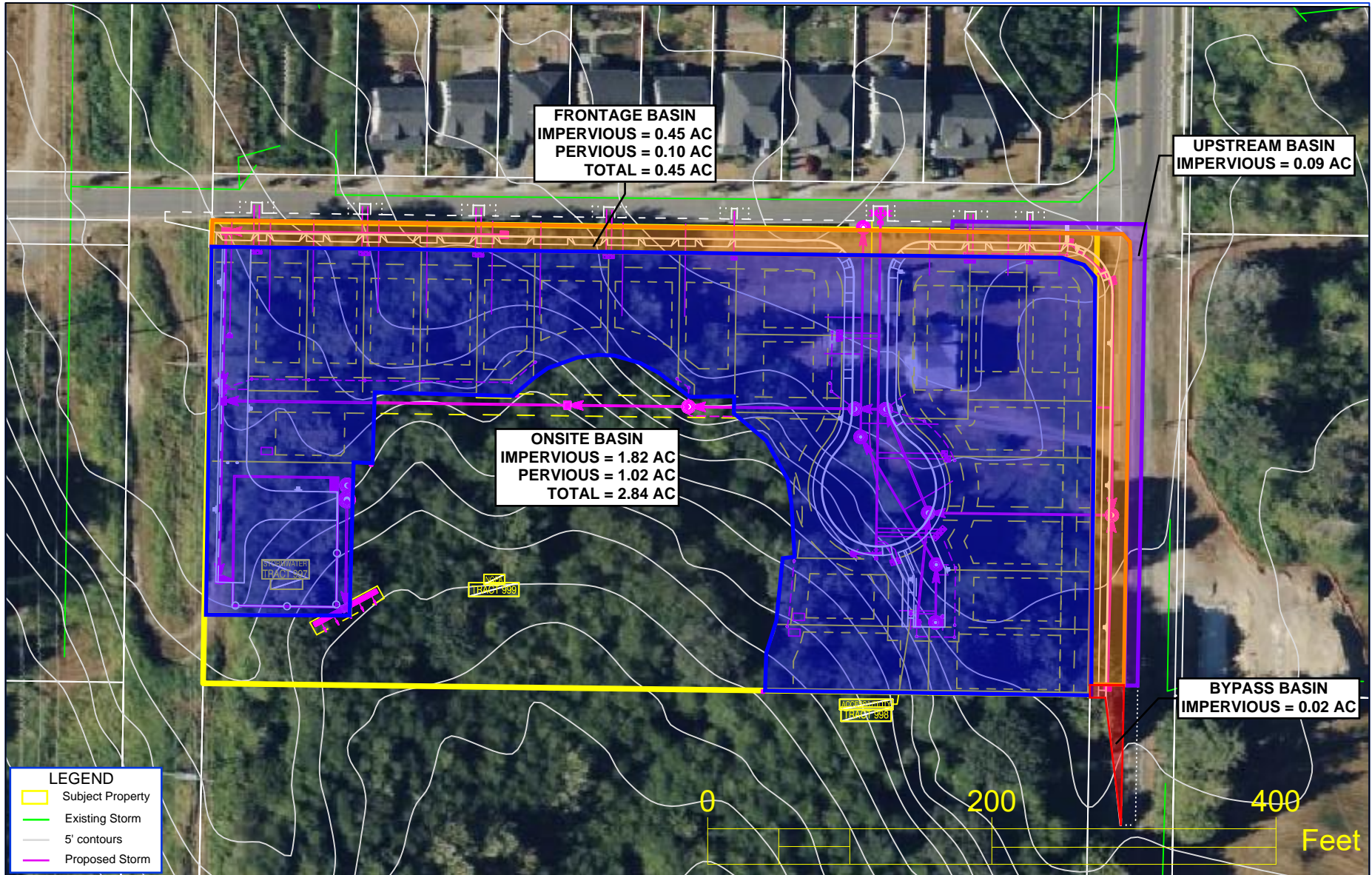



Solid Ground Engineering
8105 166th Ave NE
Redmond, WA 98052

CORNERSTONE HOMES
TAYLOR LANE
PREDEVELOPED HYDROLOGY MAP

JOB NUMBER:	23-0009	DATE:	1-15-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

4 OF 5



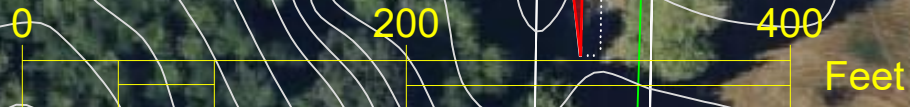
FRONTAGE BASIN
 IMPERVIOUS = 0.45 AC
 PERVIOUS = 0.10 AC
 TOTAL = 0.45 AC

UPSTREAM BASIN
 IMPERVIOUS = 0.09 AC

ONSITE BASIN
 IMPERVIOUS = 1.82 AC
 PERVIOUS = 1.02 AC
 TOTAL = 2.84 AC

BYPASS BASIN
 IMPERVIOUS = 0.02 AC

- LEGEND**
- Subject Property
 - Existing Storm
 - 5' contours
 - Proposed Storm



SGE
Solid Ground Engineering
 8105 166th Ave NE
 Redmond, WA 98052

CORNERSTONE HOMES
 TAYLOR LANE
 DEVELOPED HYDROLOGY MAP

JOB NUMBER:	23-0009	DATE:	1-15-24
JURISDICTION:	Marysville	DRAWN BY:	CJD

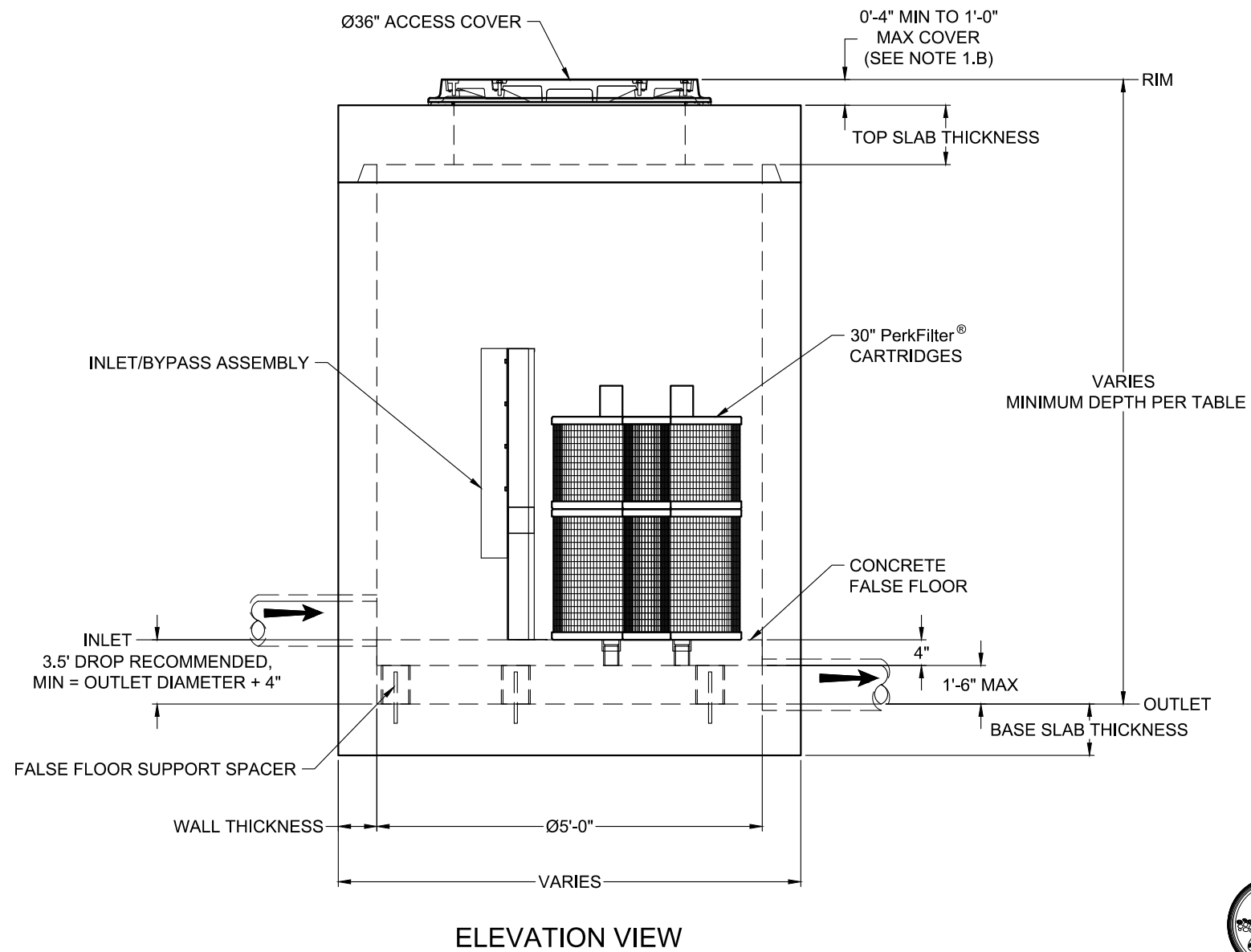
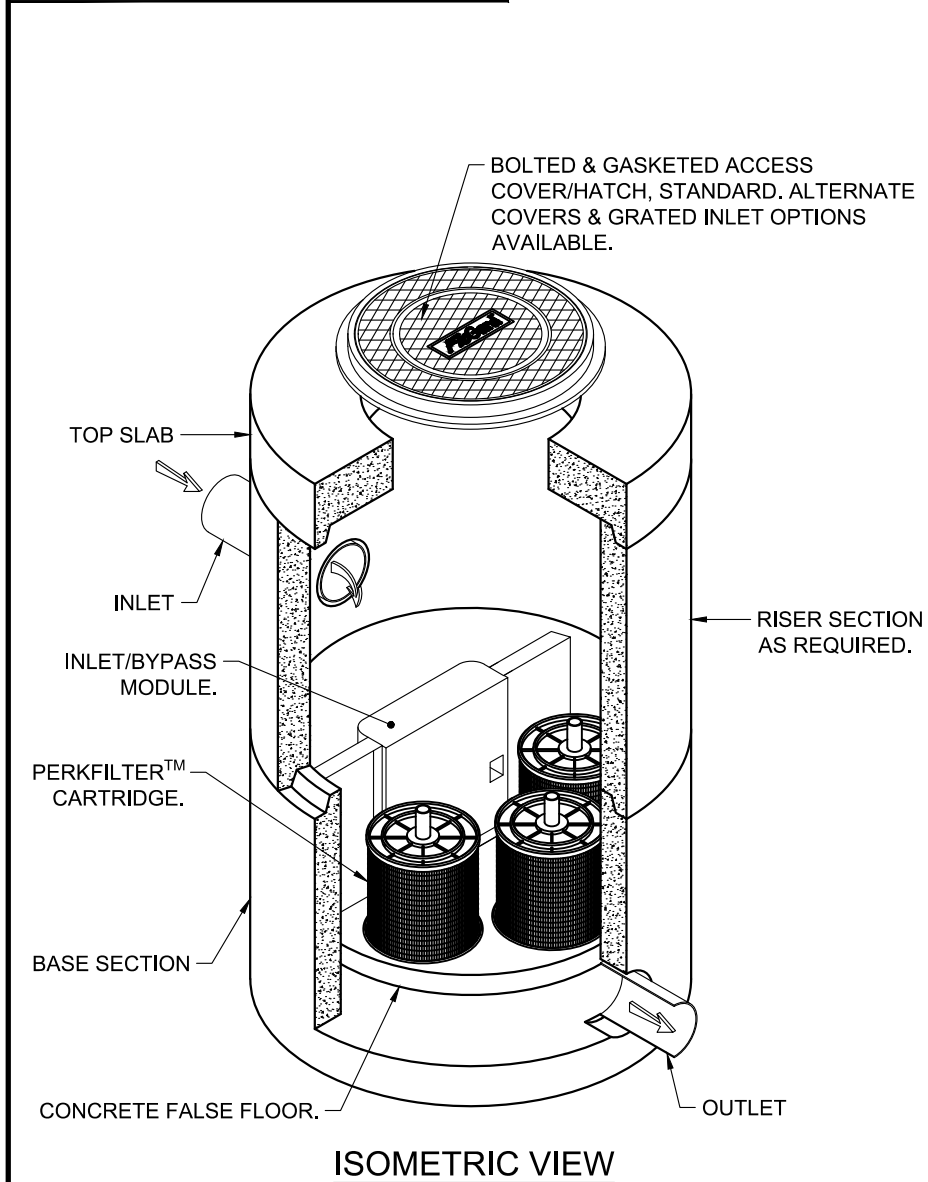
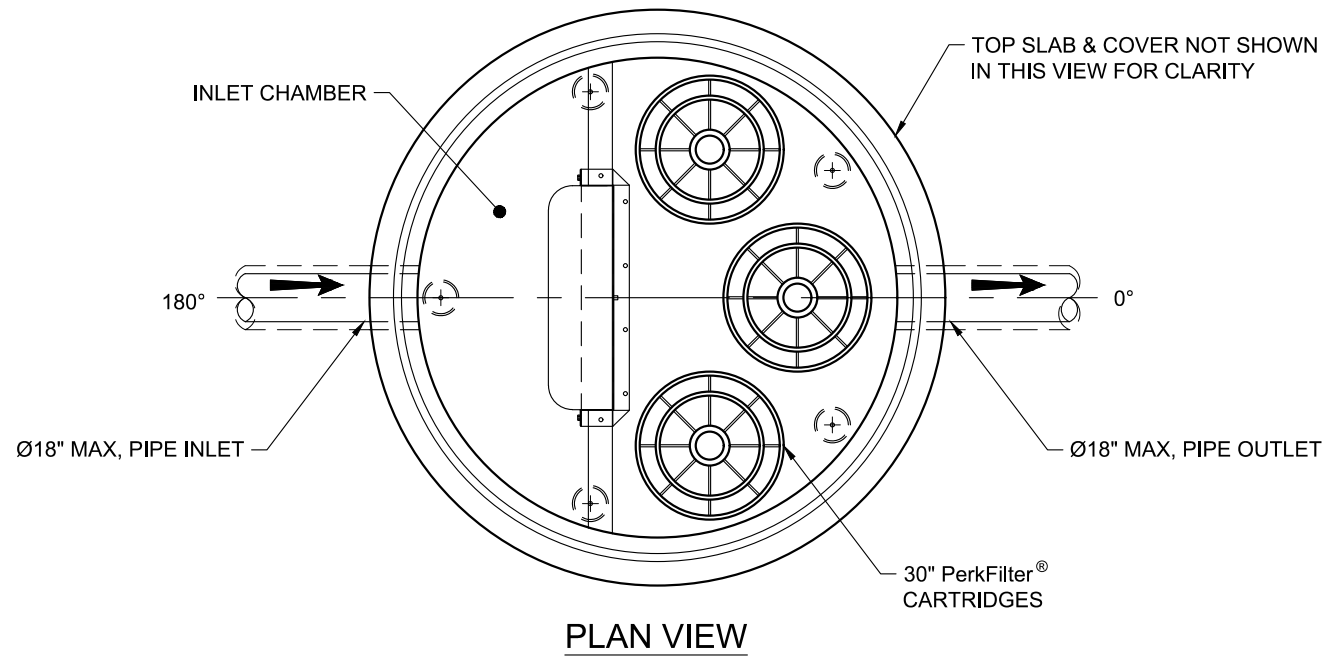
SITE SPECIFIC DATA					MINIMUM DEPTH	
Structure ID	-	Outlet Pipe Size	Minimum Rim to Outlet Depth			
Treatment Flow Rate (gpm/cfs)	-	Ø6"	6.08'			
Peak Flow Rate (cfs)	-	Ø8"	6.33'			
Cartridge Quantity	-	Ø10"	6.58'			
Rim Elevation	-	Ø12"	6.83'			
		Ø15"	7.08'			
		Ø18"	7.33'			

Pipe Data	Pipe Location	Pipe Size	Pipe Type	Invert Elevation
Inlet	-	-	-	-
Outlet	-	-	-	-

Notes:
-

PERFORMANCE SPECIFICATIONS	
Peak Treatment Capacities: ¹	
Max. Cartridge Quantity	3
NJDEP 80% Removal, 75 micron	102 gpm / 0.227 cfs
WA Ecology GULD - Basic & Phosphorus	51 gpm / 0.113 cfs
Max. Bypass Capacity	3.62 cfs

¹ Contact Oldcastle for alternative treatment and peak flow capacities.



NOTES:

- DESIGN LOADINGS:
 - AASHTO HS-20-44 (WITH IMPACT)
 - DESIGN SOIL COVER: 1'-0" MAXIMUM
 - ASSUMED WATER TABLE: BELOW INVERT.
 - LATERAL EARTH PRESSURE: 45 PCF (DRAINED)
 - LATERAL LIVE LOAD SURCHARGE: 80 PSF (APPLIED TO 8'-0" BELOW GRADE)
 - NO LATERAL SURCHARGE FROM ADJACENT BUILDINGS, WALLS, PIERS, OR FOUNDATIONS.
- CONCRETE 28-DAY MINIMUM COMPRESSIVE STRENGTH: 5,000 PSI MINIMUM.
- REINFORCING: REBAR, ASTM A615/A706, GRADE 60
- CEMENT: ASTM C150
- REQUIRED ALLOWABLE SOIL BEARING CAPACITY: 2,500 PSF
- REFERENCE STANDARD:
 - ASTM C 478
 - ASTM C 497
- THIS STRUCTURE IS DESIGNED TO THE PARAMETERS NOTED HEREIN. ENGINEER-OF-RECORD SHALL VERIFY THAT NOTED PARAMETERS MEET OR EXCEED PROJECT REQUIREMENTS. IF DESIGN PARAMETERS ARE INCORRECT, REVIEWING ENGINEER/AUTHORITY SHALL NOTIFY OLDCASTLE INFRASTRUCTURE UPON REVIEW OF THIS SUBMITTAL.
- OVERSIZED HOLES TO ACCOMMODATE SPECIFIC PIPE TYPE MUST BE CONCENTRIC TO PIPE ID. AFTER PIPES ARE INSTALLED, ALL ANNULAR SPACES SHALL BE FILLED WITH A MINIMUM OF 3,000 PSI CONCRETE FOR FULL THICKNESS OF PRECAST WALLS. PIPES ARE TO BE FLUSH WITH THE INSIDE SURFACE OF THE CONCRETE STRUCTURE.
- CONTRACTOR RESPONSIBLE TO VERIFY ALL SIZES, LOCATIONS, AND ELEVATIONS OF OPENINGS.
- CONTRACTOR RESPONSIBLE TO ENSURE ADEQUATE BEARING SURFACE IS PROVIDED (I.E. COMPACTED AND LEVEL PER PROJECT SPECIFICATIONS).
- SECTION HEIGHTS, SLAB/WALL THICKNESSES, AND KEYWAYS ARE SUBJECT TO CHANGE AS REQUIRED FOR SITE REQUIREMENTS AND/OR DUE TO PRODUCT AVAILABILITY AND PRODUCTION FACILITY CONSTRAINTS.
- FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT OLDCASTLE INFRASTRUCTURE.
- MAXIMUM PICK WEIGHTS:
 - TOP SLAB: XX,XXX LBS
 - RISER: XX,XXX LBS
 - BASE: XX,XXX LBS* (* COMBINED WEIGHT OF BASE INCLUDES FALSE FLOOR, AND PRODUCT INTERNALS.)
- INTERNALS SHALL CONSIST OF CARTRIDGES, INLET/BYPASS ASSEMBLIES, FALSE FLOOR AND FALSE FLOOR SUPPORT SPACERS.



Ph: 800.579.8819 | www.oldcastleinfrastructure.com/stormwater
 THIS DOCUMENT IS THE PROPERTY OF OLDCASTLE INFRASTRUCTURE, INC. IT IS CONFIDENTIAL, SUBMITTED FOR REFERENCE PURPOSES ONLY AND SHALL NOT BE USED IN ANY WAY INJURIOUS TO THE INTERESTS OF, OR WITHOUT THE WRITTEN PERMISSION OF OLDCASTLE INFRASTRUCTURE, INC. COPYRIGHT © 2023 OLDCASTLE INFRASTRUCTURE, INC. ALL RIGHTS RESERVED.

PerkFilter® Manhole (STANDARD)
 Ø60" with 30" Cartridges

CUSTOMER	-	REVISION	-	SHEET	1 OF 1
PROJECT NAME	-	REV DATE	-		
SHEET NAME	Specifier Drawing				
	PFMH-60-30				



WWHM2012
PROJECT REPORT

General Model Information

WWHM2012 Project Name: Prelim Vault Sizing

Site Name:

Site Address:

City:

Report Date: 11/30/2023

Gage: Everett

Data Start: 1948/10/01

Data End: 2009/09/30

Timestep: 15 Minute

Precip Scale: 1.200

Version Date: 2023/01/27

Version: 4.2.19

POC Thresholds

Low Flow Threshold for POC1: 50 Percent of the 2 Year

High Flow Threshold for POC1: 50 Year

Landuse Basin Data

Predeveloped Land Use

onsite

Bypass:	No
GroundWater:	No
Pervious Land Use C, Forest, Steep	acre 2.84
Pervious Total	2.84
Impervious Land Use	acre
Impervious Total	0
Basin Total	2.84

frontage

Bypass:	No
GroundWater:	No
Pervious Land Use C, Forest, Flat	acre 0.47
Pervious Total	0.47
Impervious Land Use	acre
Impervious Total	0
Basin Total	0.47

upstream

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
Pervious Total	0
Impervious Land Use	acre
ROADS FLAT	0.09
Impervious Total	0.09
Basin Total	0.09

Mitigated Land Use

onsite

Bypass: No

GroundWater: No

Pervious Land Use acre

C, Pasture, Flat 0.75

C, Pasture, Mod 0.27

Pervious Total 1.02

Impervious Land Use acre

ROADS FLAT 0.36

ROOF TOPS FLAT 1.04

DRIVEWAYS FLAT 0.35

SIDEWALKS FLAT 0.07

Impervious Total 1.82

Basin Total 2.84

frontage

Bypass:	No
GroundWater:	No
Pervious Land Use C, Pasture, Flat	acre 0.1
Pervious Total	0.1
Impervious Land Use ROADS FLAT SIDEWALKS FLAT	acre 0.25 0.1
Impervious Total	0.35
Basin Total	0.45

frontage bypass

Bypass:	Yes
GroundWater:	No
Pervious Land Use	acre
Pervious Total	0
Impervious Land Use	acre
ROADS FLAT	0.02
Impervious Total	0.02
Basin Total	0.02

upstream

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
Pervious Total	0
Impervious Land Use	acre
ROADS FLAT	0.09
Impervious Total	0.09
Basin Total	0.09

Routing Elements
Predeveloped Routing

Mitigated Routing

Vault 1

Width: 90 ft.
 Length: 70 ft.
 Depth: 7 ft.
 Discharge Structure
 Riser Height: 6 ft.
 Riser Diameter: 12 in.
 Orifice 1 Diameter: 1.563 in. Elevation:0 ft.
 Orifice 2 Diameter: 2.375 in. Elevation:4.2 ft.
 Orifice 3 Diameter: 1.531 in. Elevation:5.3 ft.
 Element Flows To:
 Outlet 1 Outlet 2

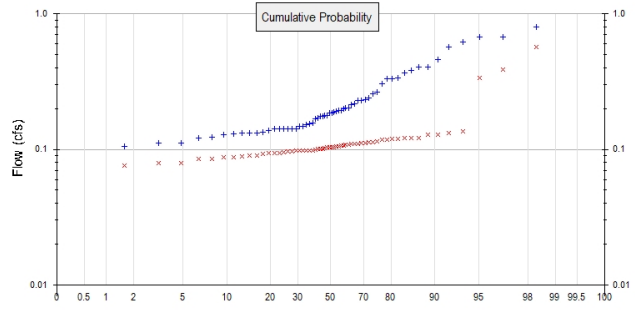
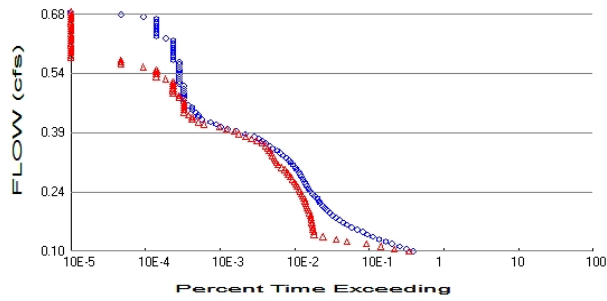
Vault Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
0.0000	0.144	0.000	0.000	0.000
0.0778	0.144	0.011	0.018	0.000
0.1556	0.144	0.022	0.026	0.000
0.2333	0.144	0.033	0.032	0.000
0.3111	0.144	0.045	0.037	0.000
0.3889	0.144	0.056	0.041	0.000
0.4667	0.144	0.067	0.045	0.000
0.5444	0.144	0.078	0.048	0.000
0.6222	0.144	0.090	0.052	0.000
0.7000	0.144	0.101	0.055	0.000
0.7778	0.144	0.112	0.058	0.000
0.8556	0.144	0.123	0.061	0.000
0.9333	0.144	0.135	0.064	0.000
1.0111	0.144	0.146	0.066	0.000
1.0889	0.144	0.157	0.069	0.000
1.1667	0.144	0.168	0.071	0.000
1.2444	0.144	0.180	0.073	0.000
1.3222	0.144	0.191	0.076	0.000
1.4000	0.144	0.202	0.078	0.000
1.4778	0.144	0.213	0.080	0.000
1.5556	0.144	0.225	0.082	0.000
1.6333	0.144	0.236	0.084	0.000
1.7111	0.144	0.247	0.086	0.000
1.7889	0.144	0.258	0.088	0.000
1.8667	0.144	0.270	0.090	0.000
1.9444	0.144	0.281	0.092	0.000
2.0222	0.144	0.292	0.094	0.000
2.1000	0.144	0.303	0.096	0.000
2.1778	0.144	0.315	0.097	0.000
2.2556	0.144	0.326	0.099	0.000
2.3333	0.144	0.337	0.101	0.000
2.4111	0.144	0.348	0.102	0.000
2.4889	0.144	0.360	0.104	0.000
2.5667	0.144	0.371	0.106	0.000
2.6444	0.144	0.382	0.107	0.000
2.7222	0.144	0.393	0.109	0.000
2.8000	0.144	0.405	0.110	0.000
2.8778	0.144	0.416	0.112	0.000

2.9556	0.144	0.427	0.113	0.000
3.0333	0.144	0.438	0.115	0.000
3.1111	0.144	0.450	0.116	0.000
3.1889	0.144	0.461	0.118	0.000
3.2667	0.144	0.472	0.119	0.000
3.3444	0.144	0.483	0.121	0.000
3.4222	0.144	0.494	0.122	0.000
3.5000	0.144	0.506	0.123	0.000
3.5778	0.144	0.517	0.125	0.000
3.6556	0.144	0.528	0.126	0.000
3.7333	0.144	0.539	0.128	0.000
3.8111	0.144	0.551	0.129	0.000
3.8889	0.144	0.562	0.130	0.000
3.9667	0.144	0.573	0.132	0.000
4.0444	0.144	0.584	0.133	0.000
4.1222	0.144	0.596	0.134	0.000
4.2000	0.144	0.607	0.135	0.000
4.2778	0.144	0.618	0.179	0.000
4.3556	0.144	0.629	0.198	0.000
4.4333	0.144	0.641	0.213	0.000
4.5111	0.144	0.652	0.226	0.000
4.5889	0.144	0.663	0.237	0.000
4.6667	0.144	0.674	0.247	0.000
4.7444	0.144	0.686	0.257	0.000
4.8222	0.144	0.697	0.266	0.000
4.9000	0.144	0.708	0.274	0.000
4.9778	0.144	0.719	0.282	0.000
5.0556	0.144	0.731	0.290	0.000
5.1333	0.144	0.742	0.298	0.000
5.2111	0.144	0.753	0.305	0.000
5.2889	0.144	0.764	0.312	0.000
5.3667	0.144	0.776	0.335	0.000
5.4444	0.144	0.787	0.349	0.000
5.5222	0.144	0.798	0.361	0.000
5.6000	0.144	0.809	0.372	0.000
5.6778	0.144	0.821	0.383	0.000
5.7556	0.144	0.832	0.392	0.000
5.8333	0.144	0.843	0.402	0.000
5.9111	0.144	0.854	0.411	0.000
5.9889	0.144	0.866	0.419	0.000
6.0667	0.144	0.877	0.610	0.000
6.1444	0.144	0.888	1.008	0.000
6.2222	0.144	0.899	1.490	0.000
6.3000	0.144	0.911	1.961	0.000
6.3778	0.144	0.922	2.338	0.000
6.4556	0.144	0.933	2.580	0.000
6.5333	0.144	0.944	2.774	0.000
6.6111	0.144	0.956	2.943	0.000
6.6889	0.144	0.967	3.102	0.000
6.7667	0.144	0.978	3.252	0.000
6.8444	0.144	0.989	3.395	0.000
6.9222	0.144	1.001	3.532	0.000
7.0000	0.144	1.012	3.664	0.000
7.0778	0.144	1.023	3.790	0.000
7.1556	0.000	0.000	3.912	0.000

Analysis Results

POC 1



+ Predeveloped x Mitigated

Predeveloped Landuse Totals for POC #1

Total Pervious Area: 3.31
 Total Impervious Area: 0.09

Mitigated Landuse Totals for POC #1

Total Pervious Area: 1.12
 Total Impervious Area: 2.28

Flow Frequency Method: Log Pearson Type III 17B

Flow Frequency Return Periods for Predeveloped. POC #1

Return Period	Flow(cfs)
2 year	0.195214
5 year	0.311313
10 year	0.407627
25 year	0.554341
50 year	0.68372
100 year	0.832006

Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs)
2 year	0.10527
5 year	0.143273
10 year	0.172345
25 year	0.213816
50 year	0.248357
100 year	0.28621

Annual Peaks

Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1949	0.228	0.098
1950	0.256	0.101
1951	0.171	0.094
1952	0.174	0.079
1953	0.176	0.085
1954	0.677	0.104
1955	0.239	0.110
1956	0.192	0.118
1957	0.265	0.121
1958	0.673	0.098

1959	0.169	0.107
1960	0.201	0.113
1961	0.799	0.106
1962	0.201	0.102
1963	0.331	0.093
1964	0.212	0.093
1965	0.131	0.103
1966	0.105	0.088
1967	0.190	0.097
1968	0.232	0.121
1969	0.620	0.094
1970	0.128	0.090
1971	0.231	0.110
1972	0.177	0.108
1973	0.141	0.098
1974	0.367	0.097
1975	0.194	0.089
1976	0.135	0.108
1977	0.123	0.095
1978	0.142	0.079
1979	0.382	0.113
1980	0.185	0.088
1981	0.138	0.096
1982	0.177	0.128
1983	0.305	0.098
1984	0.152	0.118
1985	0.218	0.115
1986	0.459	0.337
1987	0.200	0.135
1988	0.142	0.109
1989	0.185	0.071
1990	0.141	0.111
1991	0.156	0.111
1992	0.155	0.100
1993	0.132	0.090
1994	0.111	0.104
1995	0.149	0.119
1996	0.333	0.119
1997	0.571	0.573
1998	0.148	0.085
1999	0.130	0.104
2000	0.121	0.121
2001	0.059	0.076
2002	0.141	0.106
2003	0.111	0.098
2004	0.189	0.133
2005	0.141	0.104
2006	0.404	0.128
2007	0.335	0.102
2008	0.405	0.386
2009	0.132	0.099

Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	Mitigated
1	0.7991	0.5725
2	0.6774	0.3858
3	0.6727	0.3371

4	0.6201	0.1352
5	0.5705	0.1327
6	0.4588	0.1284
7	0.4054	0.1278
8	0.4040	0.1213
9	0.3819	0.1210
10	0.3665	0.1205
11	0.3354	0.1194
12	0.3326	0.1192
13	0.3306	0.1181
14	0.3052	0.1175
15	0.2648	0.1146
16	0.2555	0.1129
17	0.2386	0.1126
18	0.2324	0.1112
19	0.2308	0.1110
20	0.2283	0.1100
21	0.2179	0.1096
22	0.2124	0.1094
23	0.2013	0.1079
24	0.2008	0.1076
25	0.2004	0.1072
26	0.1939	0.1065
27	0.1922	0.1055
28	0.1905	0.1045
29	0.1892	0.1044
30	0.1851	0.1042
31	0.1851	0.1040
32	0.1774	0.1031
33	0.1772	0.1017
34	0.1761	0.1015
35	0.1741	0.1012
36	0.1707	0.1004
37	0.1687	0.0991
38	0.1564	0.0984
39	0.1553	0.0978
40	0.1517	0.0978
41	0.1486	0.0976
42	0.1480	0.0976
43	0.1420	0.0974
44	0.1417	0.0968
45	0.1415	0.0959
46	0.1412	0.0950
47	0.1411	0.0942
48	0.1407	0.0942
49	0.1381	0.0934
50	0.1348	0.0931
51	0.1324	0.0899
52	0.1319	0.0896
53	0.1312	0.0888
54	0.1302	0.0877
55	0.1275	0.0877
56	0.1234	0.0853
57	0.1209	0.0846
58	0.1108	0.0791
59	0.1108	0.0787
60	0.1052	0.0758
61	0.0585	0.0707

Duration Flows

The Facility PASSED

Flow(cfs)	Predev	Mit	Percentage	Pass/Fail
0.0976	8339	7330	87	Pass
0.1035	6688	4648	69	Pass
0.1094	5407	3041	56	Pass
0.1154	4336	1946	44	Pass
0.1213	3531	1075	30	Pass
0.1272	2890	769	26	Pass
0.1331	2398	512	21	Pass
0.1390	2021	388	19	Pass
0.1450	1754	383	21	Pass
0.1509	1472	376	25	Pass
0.1568	1239	372	30	Pass
0.1627	1070	368	34	Pass
0.1687	955	362	37	Pass
0.1746	848	357	42	Pass
0.1805	755	353	46	Pass
0.1864	685	347	50	Pass
0.1923	630	339	53	Pass
0.1983	578	329	56	Pass
0.2042	538	316	58	Pass
0.2101	506	308	60	Pass
0.2160	471	299	63	Pass
0.2219	452	294	65	Pass
0.2279	431	284	65	Pass
0.2338	398	271	68	Pass
0.2397	366	263	71	Pass
0.2456	347	251	72	Pass
0.2515	328	243	74	Pass
0.2575	314	233	74	Pass
0.2634	302	221	73	Pass
0.2693	289	211	73	Pass
0.2752	274	198	72	Pass
0.2811	264	188	71	Pass
0.2871	254	176	69	Pass
0.2930	243	169	69	Pass
0.2989	230	159	69	Pass
0.3048	220	143	65	Pass
0.3107	202	128	63	Pass
0.3167	189	121	64	Pass
0.3226	178	117	65	Pass
0.3285	165	114	69	Pass
0.3344	150	109	72	Pass
0.3403	135	103	76	Pass
0.3463	125	97	77	Pass
0.3522	109	92	84	Pass
0.3581	100	87	87	Pass
0.3640	86	78	90	Pass
0.3699	78	66	84	Pass
0.3759	70	56	80	Pass
0.3818	62	47	75	Pass
0.3877	47	38	80	Pass
0.3936	35	33	94	Pass
0.3995	27	26	96	Pass
0.4055	22	21	95	Pass

0.4114	19	13	68	Pass
0.4173	16	11	68	Pass
0.4232	12	9	75	Pass
0.4291	11	9	81	Pass
0.4351	11	8	72	Pass
0.4410	10	7	70	Pass
0.4469	9	7	77	Pass
0.4528	9	7	77	Pass
0.4587	8	7	87	Pass
0.4647	7	7	100	Pass
0.4706	7	7	100	Pass
0.4765	7	6	85	Pass
0.4824	7	6	85	Pass
0.4883	7	5	71	Pass
0.4943	7	5	71	Pass
0.5002	7	5	71	Pass
0.5061	7	5	71	Pass
0.5120	6	5	83	Pass
0.5180	6	5	83	Pass
0.5239	6	4	66	Pass
0.5298	6	3	50	Pass
0.5357	6	3	50	Pass
0.5416	6	3	50	Pass
0.5476	6	3	50	Pass
0.5535	6	2	33	Pass
0.5594	6	1	16	Pass
0.5653	6	1	16	Pass
0.5712	5	1	20	Pass
0.5772	5	0	0	Pass
0.5831	5	0	0	Pass
0.5890	5	0	0	Pass
0.5949	5	0	0	Pass
0.6008	5	0	0	Pass
0.6068	5	0	0	Pass
0.6127	5	0	0	Pass
0.6186	5	0	0	Pass
0.6245	4	0	0	Pass
0.6304	3	0	0	Pass
0.6364	3	0	0	Pass
0.6423	3	0	0	Pass
0.6482	3	0	0	Pass
0.6541	3	0	0	Pass
0.6600	3	0	0	Pass
0.6660	3	0	0	Pass
0.6719	3	0	0	Pass
0.6778	2	0	0	Pass
0.6837	1	0	0	Pass

Water Quality

Water Quality BMP Flow and Volume for POC #1

On-line facility volume: 0.1562 acre-feet

On-line facility target flow: 0.0818 cfs.

Adjusted for 15 min: 0.0818 cfs.

Off-line facility target flow: 0.0523 cfs.

Adjusted for 15 min: 0.0523 cfs.

LID Report

LID Technique	Used for Treatment ?	Total Volume Needs Treatment (ac-ft)	Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Vault 1 POC	<input type="checkbox"/>	475.34			<input type="checkbox"/>	0.00			
Total Volume Infiltrated		475.34	0.00	0.00		0.00	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

Model Default Modifications

Total of 0 changes have been made.

PERLND Changes

No PERLND changes have been made.

IMPLND Changes

No IMPLND changes have been made.

Appendix
Predeveloped Schematic



Mitigated Schematic



Predeveloped UCI File

RUN

GLOBAL

```
WVHM4 model simulation
START      1948 10 01      END      2009 09 30
RUN INTERP OUTPUT LEVEL   3      0
RESUME     0 RUN         1
UNIT SYSTEM 1
```

END GLOBAL

FILES

```
<File> <Un#> <-----File Name----->***
<-ID->                                     ***
WDM      26    Prelim Vault Sizing.wdm
MESSU    25    PrePrelim Vault Sizing.MES
          27    PrePrelim Vault Sizing.L61
          28    PrePrelim Vault Sizing.L62
          30    POCPrelim Vault Sizing1.dat
```

END FILES

OPN SEQUENCE

INGRP INDELT 00:15

```
PERLND 12
PERLND 10
IMPLND 1
COPY    501
DISPLY 1
```

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INFO1

```
# - #<-----Title----->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND
1 onsite MAX 1 2 30 9
```

END DISPLY-INFO1

END DISPLY

COPY

TIMESERIES

```
# - # NPT NMN ***
1 1 1 1
501 1 1
```

END TIMESERIES

END COPY

GENER

OPCODE

```
# # OPCD ***
```

END OPCODE

PARM

```
# # K ***
```

END PARM

END GENER

PERLND

GEN-INFO

```
<PLS ><-----Name----->NBLKS Unit-systems Printer ***
# - # User t-series Engl Metr ***
in out ***
```

```
12 C, Forest, Steep 1 1 1 1 27 0
10 C, Forest, Flat 1 1 1 1 27 0
```

END GEN-INFO

*** Section PWATER***

ACTIVITY

```
<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
12 0 0 1 0 0 0 0 0 0 0 0 0
10 0 0 1 0 0 0 0 0 0 0 0 0
```

END ACTIVITY

PRINT-INFO

```
<PLS > ***** Print-flags ***** PIVL PYR
```

```

# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
12      0      0      4      0      0      0      0      0      0      0      0      0      0      1      9
10      0      0      4      0      0      0      0      0      0      0      0      0      0      1      9
END PRINT-INFO

```

```

PWAT-PARM1
<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
12      0      0      0      0      0      0      0      0      0      0      0
10      0      0      0      0      0      0      0      0      0      0      0
END PWAT-PARM1

```

```

PWAT-PARM2
<PLS > PWATER input info: Part 2 ***
# - # ***FOREST LZSN INFILF LSUR SLSUR KVARY AGWRC
12      0      4.5      0.08      400      0.15      0.5      0.996
10      0      4.5      0.08      400      0.05      0.5      0.996
END PWAT-PARM2

```

```

PWAT-PARM3
<PLS > PWATER input info: Part 3 ***
# - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
12      0      0      2      2      0      0      0
10      0      0      2      2      0      0      0
END PWAT-PARM3

```

```

PWAT-PARM4
<PLS > PWATER input info: Part 4 ***
# - # CEPSC UZSN NSUR INTFW IRC LZETP ***
12      0.2      0.3      0.35      6      0.3      0.7
10      0.2      0.5      0.35      6      0.5      0.7
END PWAT-PARM4

```

```

PWAT-STATE1
<PLS > *** Initial conditions at start of simulation
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
# - # *** CEPS SURS UZS IFWS LZS AGWS GWVS
12      0      0      0      0      2.5      1      0
10      0      0      0      0      2.5      1      0
END PWAT-STATE1

```

END PERLND

```

IMPLND
GEN-INFO
<PLS ><-----Name-----> Unit-systems Printer ***
# - # User t-series Engl Metr ***
in out ***
1 ROADS/FLAT 1 1 1 27 0
END GEN-INFO
*** Section IWATER***

```

```

ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
1 0 0 1 0 0 0
END ACTIVITY

```

```

PRINT-INFO
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
1 0 0 4 0 0 4 1 9
END PRINT-INFO

```

```

IWAT-PARM1
<PLS > IWATER variable monthly parameter value flags ***
# - # CSNO RTOP VRS VNN RTLI ***
1 0 0 0 0 0
END IWAT-PARM1

```

IWAT-PARM2


```

HYDR-PARM2
# - # FTABNO LEN DELTH STCOR KS DB50 ***
<-----><-----><-----><-----><-----><-----><-----> ***
END HYDR-PARM2
HYDR-INIT
RCHRES Initial conditions for each HYDR section ***
# - # *** VOL Initial value of COLIND Initial value of OUTDGT
*** ac-ft for each possible exit for each possible exit
<-----><-----> <-----><-----><-----><-----> *** <-----><-----><-----><-----><----->
END HYDR-INIT
END RCHRES

SPEC-ACTIONS
END SPEC-ACTIONS
FTABLES
END FTABLES

EXT SOURCES
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # ***
WDM 2 PREC ENGL 1.2 PERLND 1 999 EXTNL PREC
WDM 2 PREC ENGL 1.2 IMPLND 1 999 EXTNL PREC
WDM 1 EVAP ENGL 0.76 PERLND 1 999 EXTNL PETINP
WDM 1 EVAP ENGL 0.76 IMPLND 1 999 EXTNL PETINP

END EXT SOURCES

EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
COPY 501 OUTPUT MEAN 1 1 48.4 WDM 501 FLOW ENGL REPL
END EXT TARGETS

MASS-LINK
<Volume> <-Grp> <-Member-><--Mult--> <Target> <-Grp> <-Member->***
<Name> <Name> # #<-factor-> <Name> <Name> # #***
MASS-LINK 12
PERLND PWATER SURO 0.083333 COPY INPUT MEAN
END MASS-LINK 12

MASS-LINK 13
PERLND PWATER IFWO 0.083333 COPY INPUT MEAN
END MASS-LINK 13

MASS-LINK 15
IMPLND IWATER SURO 0.083333 COPY INPUT MEAN
END MASS-LINK 15

END MASS-LINK

END RUN

```

Mitigated UCI File

RUN

GLOBAL

WVHM4 model simulation
START 1948 10 01 END 2009 09 30
RUN INTERP OUTPUT LEVEL 3 0
RESUME 0 RUN 1 UNIT SYSTEM 1
END GLOBAL

FILES

<File>	<Un#>	<-----File Name----->	***
<-ID->			***
WDM	26	Prelim Vault Sizing.wdm	
MESSU	25	MitPrelim Vault Sizing.MES	
	27	MitPrelim Vault Sizing.L61	
	28	MitPrelim Vault Sizing.L62	
	30	POCPrelim Vault Sizing1.dat	

END FILES

OPN SEQUENCE

INGRP INDELT 00:15
PERLND 13
PERLND 14
IMPLND 1
IMPLND 4
IMPLND 5
IMPLND 8
RCHRES 1
COPY 1
COPY 501
COPY 601
DISPLY 1

END INGRP

END OPN SEQUENCE

DISPLY

DISPLY-INF01

#	-	#	<-----Title----->	***	TRAN	PIVL	DIG1	FIL1	PYR	DIG2	FIL2	YRND
1			Vault 1		MAX				1	2	30	9

END DISPLY-INF01

END DISPLY

COPY

TIMESERIES

#	-	#	NPT	NMN	***
1			1	1	
501			1	1	
601			1	1	

END TIMESERIES

END COPY

GENER

OPCODE

#	#	OPCD	***

END OPCODE

PARM

#	#	K	***

END PARM

END GENER

PERLND

GEN-INFO

<PLS >	<-----Name----->	NBLKS	Unit-systems	Printer	***	
#	-	#	User	t-series	Engl Metr	***
			in	out		***
13	C, Pasture, Flat	1	1	1	27	0
14	C, Pasture, Mod	1	1	1	27	0

END GEN-INFO

*** Section PWATER***

ACTIVITY

<PLS > ***** Active Sections *****

```

# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
13 0 0 1 0 0 0 0 0 0 0 0 0 0
14 0 0 1 0 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
13 0 0 4 0 0 0 0 0 0 0 0 0 1 9
14 0 0 4 0 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

PWAT-PARM1

```

<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
13 0 0 0 0 0 0 0 0 0 0 0
14 0 0 0 0 0 0 0 0 0 0 0
END PWAT-PARM1

```

PWAT-PARM2

```

<PLS > PWATER input info: Part 2 ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
13 0 4.5 0.06 400 0.05 0.5 0.996
14 0 4.5 0.06 400 0.1 0.5 0.996
END PWAT-PARM2

```

PWAT-PARM3

```

<PLS > PWATER input info: Part 3 ***
# - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
13 0 0 2 2 0 0 0
14 0 0 2 2 0 0 0
END PWAT-PARM3

```

PWAT-PARM4

```

<PLS > PWATER input info: Part 4 ***
# - # CEPSC UZSN NSUR INTFW IRC LZETP ***
13 0.15 0.4 0.3 6 0.5 0.4
14 0.15 0.4 0.3 6 0.5 0.4
END PWAT-PARM4

```

PWAT-STATE1

```

<PLS > *** Initial conditions at start of simulation
ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
# - # *** CEPS SURS UZS IFWS LZS AGWS GWVS
13 0 0 0 0 2.5 1 0
14 0 0 0 0 2.5 1 0
END PWAT-STATE1

```

END PERLND

IMPLND

GEN-INFO

```

<PLS ><-----Name-----> Unit-systems Printer ***
# - # User t-series Engr Metr ***
in out ***
1 ROADS/FLAT 1 1 1 27 0
4 ROOF TOPS/FLAT 1 1 1 27 0
5 DRIVEWAYS/FLAT 1 1 1 27 0
8 SIDEWALKS/FLAT 1 1 1 27 0
END GEN-INFO
*** Section IWATER***

```

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
1 0 0 1 0 0 0
4 0 0 1 0 0 0
5 0 0 1 0 0 0
8 0 0 1 0 0 0
END ACTIVITY

```

PRINT-INFO

```

<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL  *****
1   0   0   4   0   0   4   1   9
4   0   0   4   0   0   0   1   9
5   0   0   4   0   0   0   1   9
8   0   0   4   0   0   0   1   9

```

END PRINT-INFO

IWAT-PARM1

```

<PLS >  IWATER variable monthly parameter value flags  ***
# - # CSNO RTOP  VRS  VNM RTLI  ***
1   0   0   0   0   0
4   0   0   0   0   0
5   0   0   0   0   0
8   0   0   0   0   0

```

END IWAT-PARM1

IWAT-PARM2

```

<PLS >      IWATER input info: Part 2      ***
# - # ***  LSUR      SLSUR      NSUR      RETSC
1   400      0.01      0.1      0.1
4   400      0.01      0.1      0.1
5   400      0.01      0.1      0.1
8   400      0.01      0.1      0.1

```

END IWAT-PARM2

IWAT-PARM3

```

<PLS >      IWATER input info: Part 3      ***
# - # ***PETMAX  PETMIN
1   0          0
4   0          0
5   0          0
8   0          0

```

END IWAT-PARM3

IWAT-STATE1

```

<PLS > *** Initial conditions at start of simulation
# - # ***  RETS      SURS
1   0          0
4   0          0
5   0          0
8   0          0

```

END IWAT-STATE1

END IMPLND

SCHEMATIC

<-Source-> <Name> #	<--Area--> <-factor-->	<-Target-> <Name> #	MBLK Tbl#	*** ***
onsite***				
PERLND 13	0.75	RCHRES 1	2	
PERLND 13	0.75	RCHRES 1	3	
PERLND 14	0.27	RCHRES 1	2	
PERLND 14	0.27	RCHRES 1	3	
IMPLND 1	0.36	RCHRES 1	5	
IMPLND 4	1.04	RCHRES 1	5	
IMPLND 5	0.35	RCHRES 1	5	
IMPLND 8	0.07	RCHRES 1	5	
frontage***				
PERLND 13	0.1	RCHRES 1	2	
PERLND 13	0.1	RCHRES 1	3	
IMPLND 1	0.25	RCHRES 1	5	
IMPLND 8	0.1	RCHRES 1	5	
upstream***				
IMPLND 1	0.09	RCHRES 1	5	
frontage bypass***				
IMPLND 1	0.02	COPY 501	15	
IMPLND 1	0.02	COPY 601	15	

*****Routing*****

```

PERLND 13          0.75    COPY    1    12
PERLND 14          0.27    COPY    1    12
IMPLND 1           0.36    COPY    1    15
IMPLND 4           1.04    COPY    1    15
IMPLND 5           0.35    COPY    1    15
IMPLND 8           0.07    COPY    1    15
PERLND 13          0.75    COPY    1    13
PERLND 14          0.27    COPY    1    13
PERLND 13          0.1     COPY    1    12
IMPLND 1           0.25    COPY    1    15
IMPLND 8           0.1     COPY    1    15
PERLND 13          0.1     COPY    1    13
IMPLND 1           0.09    COPY    1    15
RCHRES 1           1       COPY    501  16
END SCHEMATIC

```

NETWORK

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1

```

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
END NETWORK

```

RCHRES

```

GEN-INFO
RCHRES      Name      Nexits  Unit Systems  Printer      ***
# - #<-----><----> User T-series  Engl Metr LKFG      ***
              in out
1 Vault 1          1  1  1  1  28  0  1
END GEN-INFO
*** Section RCHRES***

```

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUGF PKFG PHFG ***
1 1 0 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB PIVL PYR *****
1 4 0 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

HYDR-PARM1

```

RCHRES      Flags for each HYDR Section      ***
# - # VC A1 A2 A3 ODFVFG for each *** ODGTFG for each FUNCT for each
      FG FG FG FG possible exit *** possible exit possible exit
      * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
1 0 1 0 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
END HYDR-PARM1

```

HYDR-PARM2

```

# - # FTABNO      LEN      DELTH      STCOR      KS      DB50      ***
<---><-----><-----><-----><-----><-----><----->
1 1 0.01 0.0 0.0 0.5 0.0
END HYDR-PARM2

```

HYDR-INIT

```

RCHRES      Initial conditions for each HYDR section      ***
# - # *** VOL      Initial value of COLIND      Initial value of OUTDGT
      *** ac-ft      for each possible exit      for each possible exit
<-----><-----> <-----><-----><-----> *** <-----><-----><-----><----->
1 0 4.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
END HYDR-INIT

```

END RCHRES

SPEC-ACTIONS
 END SPEC-ACTIONS
 FTABLES

FTABLE 1
 92 4

Depth (ft)	Area (acres)	Volume (acre-ft)	Outflow1 (cfs)	Velocity (ft/sec)	Travel Time*** (Minutes)***
0.000000	0.144628	0.000000	0.000000		
0.077778	0.144628	0.011249	0.018477		
0.155556	0.144628	0.022498	0.026130		
0.233333	0.144628	0.033747	0.032003		
0.311111	0.144628	0.044995	0.036954		
0.388889	0.144628	0.056244	0.041315		
0.466667	0.144628	0.067493	0.045259		
0.544444	0.144628	0.078742	0.048885		
0.622222	0.144628	0.089991	0.052260		
0.700000	0.144628	0.101240	0.055430		
0.777778	0.144628	0.112489	0.058429		
0.855556	0.144628	0.123737	0.061281		
0.933333	0.144628	0.134986	0.064005		
1.011111	0.144628	0.146235	0.066619		
1.088889	0.144628	0.157484	0.069134		
1.166667	0.144628	0.168733	0.071560		
1.244444	0.144628	0.179982	0.073907		
1.322222	0.144628	0.191230	0.076182		
1.400000	0.144628	0.202479	0.078390		
1.477778	0.144628	0.213728	0.080538		
1.555556	0.144628	0.224977	0.082631		
1.633333	0.144628	0.236226	0.084671		
1.711111	0.144628	0.247475	0.086664		
1.788889	0.144628	0.258724	0.088612		
1.866667	0.144628	0.269972	0.090517		
1.944444	0.144628	0.281221	0.092384		
2.022222	0.144628	0.292470	0.094214		
2.100000	0.144628	0.303719	0.096008		
2.177778	0.144628	0.314968	0.097770		
2.255556	0.144628	0.326217	0.099501		
2.333333	0.144628	0.337466	0.101202		
2.411111	0.144628	0.348714	0.102874		
2.488889	0.144628	0.359963	0.104521		
2.566667	0.144628	0.371212	0.106141		
2.644444	0.144628	0.382461	0.107737		
2.722222	0.144628	0.393710	0.109310		
2.800000	0.144628	0.404959	0.110861		
2.877778	0.144628	0.416208	0.112390		
2.955556	0.144628	0.427456	0.113899		
3.033333	0.144628	0.438705	0.115388		
3.111111	0.144628	0.449954	0.116857		
3.188889	0.144628	0.461203	0.118309		
3.266667	0.144628	0.472452	0.119743		
3.344444	0.144628	0.483701	0.121160		
3.422222	0.144628	0.494949	0.122561		
3.500000	0.144628	0.506198	0.123946		
3.577778	0.144628	0.517447	0.125316		
3.655556	0.144628	0.528696	0.126670		
3.733333	0.144628	0.539945	0.128011		
3.811111	0.144628	0.551194	0.129338		
3.888889	0.144628	0.562443	0.130651		
3.966667	0.144628	0.573691	0.131951		
4.044444	0.144628	0.584940	0.133238		
4.122222	0.144628	0.596189	0.134513		
4.200000	0.144628	0.607438	0.135776		
4.277778	0.144628	0.618687	0.179716		
4.355556	0.144628	0.629936	0.198639		
4.433333	0.144628	0.641185	0.213436		
4.511111	0.144628	0.652433	0.226093		
4.588889	0.144628	0.663682	0.237378		
4.666667	0.144628	0.674931	0.247686		
4.744444	0.144628	0.686180	0.257252		

```

4.822222 0.144628 0.697429 0.266228
4.900000 0.144628 0.708678 0.274721
4.977778 0.144628 0.719927 0.282808
5.055556 0.144628 0.731175 0.290547
5.133333 0.144628 0.742424 0.297984
5.211111 0.144628 0.753673 0.305156
5.288889 0.144628 0.764922 0.312090
5.366667 0.144628 0.776171 0.335241
5.444444 0.144628 0.787420 0.349526
5.522222 0.144628 0.798669 0.361693
5.600000 0.144628 0.809917 0.372745
5.677778 0.144628 0.821166 0.383050
5.755556 0.144628 0.832415 0.392799
5.833333 0.144628 0.843664 0.402106
5.911111 0.144628 0.854913 0.411046
5.988889 0.144628 0.866162 0.419673
6.066667 0.144628 0.877410 0.610261
6.144444 0.144628 0.888659 1.008783
6.222222 0.144628 0.899908 1.490066
6.300000 0.144628 0.911157 1.961409
6.377778 0.144628 0.922406 2.338529
6.455556 0.144628 0.933655 2.580843
6.533333 0.144628 0.944904 2.773986
6.611111 0.144628 0.956152 2.943065
6.688889 0.144628 0.967401 3.101990
6.766667 0.144628 0.978650 3.252427
6.844444 0.144628 0.989899 3.395629
6.922222 0.144628 1.001148 3.532567
7.000000 0.144628 1.012397 3.664010
7.077778 0.144628 1.023646 3.790583

```

END FTABLE 1

END FTABLES

EXT SOURCES

```

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor-->strg <Name> # # <Name> # # ***
WDM 2 PREC ENGL 1.2 PERLND 1 999 EXTNL PREC
WDM 2 PREC ENGL 1.2 IMPLND 1 999 EXTNL PREC
WDM 1 EVAP ENGL 0.76 PERLND 1 999 EXTNL PETINP
WDM 1 EVAP ENGL 0.76 IMPLND 1 999 EXTNL PETINP

```

END EXT SOURCES

EXT TARGETS

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor-->strg <Name> # <Name> tem strg strg***
RCHRES 1 HYDR RO 1 1 1 WDM 1000 FLOW ENGL REPL
RCHRES 1 HYDR STAGE 1 1 1 WDM 1001 STAG ENGL REPL
COPY 1 OUTPUT MEAN 1 1 48.4 WDM 701 FLOW ENGL REPL
COPY 501 OUTPUT MEAN 1 1 48.4 WDM 801 FLOW ENGL REPL
COPY 601 OUTPUT MEAN 1 1 48.4 WDM 901 FLOW ENGL REPL

```

END EXT TARGETS

MASS-LINK

```

<Volume> <-Grp> <-Member-><--Mult--> <Target> <-Grp> <-Member->***
<Name> <Name> # #<-factor--> <Name> <Name> # #***
MASS-LINK 2
PERLND PWATER SURO 0.083333 RCHRES INFLOW IVOL
END MASS-LINK 2

MASS-LINK 3
PERLND PWATER IFWO 0.083333 RCHRES INFLOW IVOL
END MASS-LINK 3

MASS-LINK 5
IMPLND IWATER SURO 0.083333 RCHRES INFLOW IVOL
END MASS-LINK 5

MASS-LINK 12
PERLND PWATER SURO 0.083333 COPY INPUT MEAN

```



```

END MASS-LINK 12

MASS-LINK 13
PERLND PWATER IFWO 0.083333 COPY INPUT MEAN
END MASS-LINK 13

MASS-LINK 15
IMPLND IWATER SURO 0.083333 COPY INPUT MEAN
END MASS-LINK 15

MASS-LINK 16
RCHRES ROFLOW COPY INPUT MEAN
END MASS-LINK 16

END MASS-LINK

END RUN

```

Predeveloped HSPF Message File

Mitigated HSPF Message File

Disclaimer

Legal Notice

This program and accompanying documentation are provided 'as-is' without warranty of any kind. The entire risk regarding the performance and results of this program is assumed by End User. Clear Creek Solutions Inc. and the governmental licensee or sublicensees disclaim all warranties, either expressed or implied, including but not limited to implied warranties of program and accompanying documentation. In no event shall Clear Creek Solutions Inc. be liable for any damages whatsoever (including without limitation to damages for loss of business profits, loss of business information, business interruption, and the like) arising out of the use of, or inability to use this program even if Clear Creek Solutions Inc. or their authorized representatives have been advised of the possibility of such damages. Software Copyright © by : Clear Creek Solutions, Inc. 2005-2023; All Rights Reserved.

Clear Creek Solutions, Inc.
6200 Capitol Blvd. Ste F
Olympia, WA. 98501
Toll Free 1(866)943-0304
Local (360)943-0304

www.clearcreeksolutions.com

Appendix 5: Conveyance Analysis

Nothing is necessary for this appendix at this time

Appendix 6: Operations and Maintenance Manual

1. Operations and Maintenance Manual

Table V-A.2: Maintenance Standards - Infiltration (continued)

Maintenance Component	Defect	Conditions When Maintenance Is Needed	Results Expected When Maintenance Is Performed
		(A percolation test pit or test of facility indicates facility is only working at 90% of its designed capabilities. Test every 2 to 5 years. If two inches or more sediment is present, remove).	
Filter Bags (if applicable)	Filled with Sediment and Debris	Sediment and debris fill bag more than 1/2 full.	Filter bag is replaced or system is redesigned.
Rock Filters	Sediment and Debris	By visual inspection, little or no water flows through filter during heavy rain storms.	Gravel in rock filter is replaced.
Side Slopes of Pond	Erosion	See Table V-A.1: Maintenance Standards - Detention Ponds	See Table V-A.1: Maintenance Standards - Detention Ponds
Emergency Overflow Spillway and Berms over 4 feet in height.	Tree Growth	See Table V-A.1: Maintenance Standards - Detention Ponds	See Table V-A.1: Maintenance Standards - Detention Ponds
	Piping	See Table V-A.1: Maintenance Standards - Detention Ponds	See Table V-A.1: Maintenance Standards - Detention Ponds
Emergency Overflow Spillway	Rock Missing	See Table V-A.1: Maintenance Standards - Detention Ponds	See Table V-A.1: Maintenance Standards - Detention Ponds
	Erosion	See Table V-A.1: Maintenance Standards - Detention Ponds	See Table V-A.1: Maintenance Standards - Detention Ponds
Pre-settling Ponds and Vaults	Facility or sump filled with Sediment and/or debris	6" or designed sediment trap depth of sediment.	Sediment is removed.

Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Storage Area	Plugged Air Vents	One-half of the cross section of a vent is blocked at any point or the vent is damaged.	Vents open and functioning.
	Debris and Sediment	Accumulated sediment depth exceeds 10% of the diameter of the storage area for 1/2 length of storage vault or any point depth exceeds 15% of diameter. (Example: 72-inch storage tank would require cleaning when sediment reaches depth of 7 inches for more than 1/2 length of tank.)	All sediment and debris removed from storage area.
	Joints Between Tank/Pipe Section	Any openings or voids allowing material to be transported into facility. (Will require engineering analysis to determine structural stability).	All joint between tank/pipe sections are sealed.
	Tank Pipe Bent Out of Shape	Any part of tank/pipe is bent out of shape more than 10% of its design shape. (Review required by engineer to determine structural stability).	Tank/pipe repaired or replaced to design.
	Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab	Cracks wider than 1/2-inch and any evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determines that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or any evidence of soil particles entering the vault through the walls.	Vault replaced or repaired to design specifications and is structurally sound. No cracks more than 1/4-inch wide at the joint of the inlet/outlet pipe.

Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults) (continued)

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Manhole	Cover Not in Place	Cover is missing or only partially in place. Any open manhole requires maintenance.	Manhole is closed.
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread (may not apply to self-locking lids).	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. Intent is to keep cover from sealing off access to maintenance.	Cover can be removed and reinstalled by one maintenance person.
	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, misalignment, not securely attached to structure wall, rust, or cracks.	Ladder meets design standards. Allows maintenance person safe access.
Catch Basins	See Table V-A.5: Maintenance Standards - Catch Basins	See Table V-A.5: Maintenance Standards - Catch Basins	See Table V-A.5: Maintenance Standards - Catch Basins

Table V-A.4: Maintenance Standards - Control Structure/Flow Restrictor

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
General	Trash and Debris (Includes Sediment)	Material exceeds 25% of sump depth or 1 foot below orifice plate.	Control structure orifice is not blocked. All trash and debris removed.
	Structural Damage	Structure is not securely attached to manhole wall. Structure is not in upright position (allow up to 10% from plumb). Connections to outlet pipe are not watertight and show signs of rust. Any holes - other than designed holes - in the structure.	Structure securely attached to wall and outlet pipe. Structure in correct position. Connections to outlet pipe are water tight; structure repaired or replaced and works as designed. Structure has no holes other than designed holes.
Cleanout Gate	Damaged or Missing	Cleanout gate is not watertight or is missing. Gate cannot be moved up and down by one maintenance person. Chain/rod leading to gate is missing or damaged. Gate is rusted over 50% of its surface area.	Gate is watertight and works as designed. Gate moves up and down easily and is watertight. Chain is in place and works as designed. Gate is repaired or replaced to meet design standards.
Orifice Plate	Damaged or Missing	Control device is not working properly due to missing, out of place, or bent orifice plate.	Plate is in place and works as designed.
	Obstructions	Any trash, debris, sediment, or vegetation blocking the plate.	Plate is free of all obstructions and works as designed.
Overflow Pipe	Obstructions	Any trash or debris blocking (or having the potential of blocking) the overflow pipe.	Pipe is free of all obstructions and works as designed.
Manhole	See Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)	See Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)	See Table V-A.3: Maintenance Standards - Closed Detention Systems (Tanks/Vaults)
Catch Basin	See Table V-A.5: Maintenance Standards - Catch Basins	See Table V-A.5: Maintenance Standards - Catch Basins	See Table V-A.5: Maintenance Standards - Catch Basins

Table V-A.5: Maintenance Standards - Catch Basins

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is performed
General	Trash & Debris	Trash or debris which is located immediately in front of the catch basin opening or is blocking inletting capacity of the basin by more than 10%. Trash or debris (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of six inches clearance from the debris surface to the invert of the lowest pipe. Trash or debris in any inlet or outlet pipe blocking more than 1/3 of its height. Dead animals or vegetation that could generate odors that could cause complaints or dangerous gases (e.g., methane).	No Trash or debris located immediately in front of catch basin or on grate opening. No trash or debris in the catch basin. Inlet and outlet pipes free of trash or debris. No dead animals or vegetation present within the catch basin.
	Sediment	Sediment (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of 6 inches clearance from the sediment surface to the invert of the lowest pipe.	No sediment in the catch basin
	Structure Damage to Frame and/or Top Slab	Top slab has holes larger than 2 square inches or cracks wider than 1/4 inch. (Intent is to make sure no material is running into basin). Frame not sitting flush on top slab, i.e., separation of more than 3/4 inch of the frame from the top slab. Frame not securely attached	Top slab is free of holes and cracks. Frame is sitting flush on the riser rings or top slab and firmly attached.
	Fractures or Cracks in Basin Walls/ Bottom	Maintenance person judges that structure is unsound. Grout fillet has separated or cracked wider than 1/2 inch and longer than 1 foot at the joint of any inlet/outlet pipe or any evidence of soil particles entering catch basin through cracks.	Basin replaced or repaired to design standards. Pipe is regouted and secure at basin wall.
	Settlement/ Mis-alignment	If failure of basin has created a safety, function, or design problem.	Basin replaced or repaired to design standards.
	Vegetation	Vegetation growing across and blocking more than 10% of the basin opening. Vegetation growing in inlet/outlet pipe joints that is more than six inches tall and less than six inches apart.	No vegetation blocking opening to basin. No vegetation or root growth present.
	Contamination and Pollution	See Table V-A.1: Maintenance Standards - Detention Ponds	No pollution present.
Catch Basin Cover	Cover Not in Place	Cover is missing or only partially in place. Any open catch basin requires maintenance.	Cover/grate is in place, meets design standards, and is secured
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread.	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. (Intent is keep cover from sealing off access to maintenance.)	Cover can be removed by one maintenance person.
Ladder	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, not securely attached to basin wall, misalignment, rust, cracks, or sharp edges.	Ladder meets design standards and allows maintenance person safe access.
Metal Grates (If Applicable)	Grate opening Unsafe	Grate with opening wider than 7/8 inch.	Grate opening meets design standards.
	Trash and Debris	Trash and debris that is blocking more than 20% of grate surface inletting capacity.	Grate free of trash and debris.
	Damaged or Missing.	Grate missing or broken member(s) of the grate.	Grate is in place, meets the design standards, and is installed and aligned with the flow path.

Table V-A.13: Maintenance Standards - Sand Filters (Above Ground/Open) (continued)

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
	Flow Spreader	Flow spreader uneven or clogged so that flows are not uniformly distributed across sand filter.	Spreader leveled and cleaned so that flows are spread evenly over sand filter.
	Damaged Pipes	Any part of the piping that is crushed or deformed more than 20% or any other failure to the piping.	Pipe repaired or replaced.

Table V-A.14: Maintenance Standards - Sand Filters (Below Ground/Enclosed)

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Below Ground Vault.	Sediment Accumulation on Sand Media Section	Sediment depth exceeds 1/2-inch.	No sediment deposits on sand filter section that which would impede permeability of the filter section.
	Sediment Accumulation in Pre-Settling Portion of Vault	Sediment accumulation in vault bottom exceeds the depth of the sediment zone plus 6-inches.	No sediment deposits in first chamber of vault.
	Trash/Debris Accumulation	Trash and debris accumulated in vault, or pipe inlet/outlet, floatables and non-floatables.	Trash and debris removed from vault and inlet/outlet piping.
	Sediment in Drain Pipes/Cleanouts	When drain pipes, cleanouts become full with sediment and/or debris.	Sediment and debris removed.
	Short Circuiting	When seepage/flow occurs along the vault walls and corners. Sand eroding near inflow area.	Sand filter media section re-laid and compacted along perimeter of vault to form a semi-seal. Erosion protection added to dissipate force of incoming flow and curtail erosion.
	Damaged Pipes	Inlet or outlet piping damaged or broken and in need of repair.	Pipe repaired and/or replaced.
	Access Cover Damaged/Not Working	Cover cannot be opened, corrosion/deformation of cover. Maintenance person cannot remove cover using normal lifting pressure.	Cover repaired to proper working specifications or replaced.
	Ventilation	Ventilation area blocked or plugged	Blocking material removed or cleared from ventilation area. A specified % of the vault surface area must provide ventilation to the vault interior (see design specifications).
	Vault Structure Damaged; Includes Cracks in Walls, Bottom, Damage to Frame and/or Top Slab.	Cracks wider than 1/2-inch or evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determine that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or evidence of soil particles entering through the cracks.	Vault replaced or repairs made so that vault meets design specifications and is structurally sound. Vault repaired so that no cracks exist wider than 1/4-inch at the joint of the inlet/outlet pipe.
	Baffles/Internal walls	Baffles or walls corroding, cracking, warping and/or showing signs of failure as determined by maintenance/inspection person.	Baffles repaired or replaced to specifications.
Access Ladder Damaged	Ladder is corroded or deteriorated, not functioning properly, not securely attached to structure wall, missing rungs, cracks, and misaligned.	Ladder replaced or repaired to specifications, and is safe to use as determined by inspection personnel.	

Table V-A.15: Maintenance Standards - Manufactured Media Filters

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Below Ground	Sediment Accumulation on Media.	Sediment depth exceeds 0.25-inches.	No sediment deposits which would impede permeability of the

Table V-A.15: Maintenance Standards - Manufactured Media Filters (continued)

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
Vault			compost media.
	Sediment Accumulation in Vault	Sediment depth exceeds 6-inches in first chamber.	No sediment deposits in vault bottom of first chamber.
	Trash/Debris Accumulation	Trash and debris accumulated on compost filter bed.	Trash and debris removed from the compost filter bed.
	Sediment in Drain Pipes/Clean-Outs	When drain pipes, clean-outs, become full with sediment and/or debris.	Sediment and debris removed.
	Damaged Pipes	Any part of the pipes that are crushed or damaged due to corrosion and/or settlement.	Pipe repaired and/or replaced.
	Access Cover Damaged/Not Working	Cover cannot be opened; one person cannot open the cover using normal lifting pressure, corrosion/deformation of cover.	Cover repaired to proper working specifications or replaced.
	Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab	Cracks wider than 1/2-inch or evidence of soil particles entering the structure through the cracks, or maintenance/inspection personnel determine that the vault is not structurally sound. Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or evidence of soil particles entering through the cracks.	Vault replaced or repairs made so that vault meets design specifications and is structurally sound. Vault repaired so that no cracks exist wider than 1/4-inch at the joint of the inlet/outlet pipe.
	Baffles	Baffles corroding, cracking warping, and/or showing signs of failure as determined by maintenance/inspection person.	Baffles repaired or replaced to specifications.
Access Ladder Damaged	Ladder is corroded or deteriorated, not functioning properly, not securely attached to structure wall, missing rungs, cracks, and misaligned.	Ladder replaced or repaired and meets specifications, and is safe to use as determined by inspection personnel.	
Below Ground Cartridge Type	Media	Drawdown of water through the media takes longer than 1 hour, and/or overflow occurs frequently.	Media cartridges replaced.
	Short Circuiting	Flows do not properly enter filter cartridges.	Filter cartridges replaced.

Appendix 7: Special Reports and Studies

1. Critical Areas Report, Wetland Resources, Inc., dated January 25th, 2024
2. Geotechnical Engineering Design Study, Earth Solutions NW, dated January 25th, 2024