

Traffic Impact Analysis

WILLIAMS INDUSTRIAL

Prepared for:
Williams Investment

September 2023

Prepared by:



12131 113th Avenue NE, Suite 203
Kirkland, WA 98034
Phone: 425-821-3665
Fax: 425-825-8434
www.transpogroup.com

1.21144.00

© 2023 Transpo Group

Table of Contents

Introduction	1
Project Description	1
Study Scope	1
Existing & Future Without-Project Conditions	5
Street System	5
Transit Service	7
Traffic Volumes	7
Traffic Operations	12
Traffic Safety	13
Project Impacts	15
Trip Generation	15
Trip Distribution & Assignment	15
Volume Impact Summary	20
Traffic Operations	21
Site Access	22
Mitigation and Recommendations	23
Transportation Mitigation Fees	23
Findings and Conclusions	24

Appendix

- Appendix A: Traffic Counts
- Appendix B: LOS Definitions
- Appendix C: LOS Worksheets
- Appendix D: Trip Generation Worksheets
- Appendix E: Snohomish County Key Intersections

List of Figures

Figure 1.	Site Vicinity & Study Intersections	3
Figure 2.	Preliminary Site Plan	4
Figure 3.	Existing Weekday PM Peak Hour Traffic Volumes	9
Figure 4.	Future (2026) Without-Project Weekday PM Peak Hour Traffic Volumes	10
Figure 5.	Future (2032) Without-Project Weekday PM Peak Hour Traffic Volumes	11
Figure 6.	Year of Opening (2026) Project Trip Distribution & Assignment	16
Figure 7.	Horizon Year (2032) Project Trip Distribution & Assignment	17
Figure 8.	Future (2026) With-Project Weekend PM Peak Hour Traffic Volumes	18
Figure 9.	Future (2032) With-Project Weekend PM Peak Hour Traffic Volumes	19

List of Tables

Table 1.	Study Area Existing Roadway Network Summary	5
Table 2.	Existing Weekday PM Peak Hour Intersection LOS Summary	13
Table 3.	Five-Year Collision Summary – 2018 to 2022	14
Table 4.	Estimated Weekday Vehicle Trip Generation	15
Table 5.	Future (2026) Weekday PM Peak Hour Traffic Volume Impacts at Study Intersections	20
Table 6.	Future (2032) Weekday PM Peak Hour Traffic Volume Impacts at Study Intersections	20
Table 7.	Year of Opening (2026) Weekday PM Peak Hour Intersection LOS Summary	21
Table 8.	Horizon Year (2032) Weekday PM Peak Hour Intersection LOS Summary	22

Introduction

The purpose of this traffic impact analysis (TIA) is to identify potential transportation-related impacts to the surrounding street network associated with the development of the proposed Industrial project in Marysville, WA.

Project Description

The proposed project would develop an industrial and business park across parcels in Arlington and Marysville. This study focused on the approximately 2,055,069 square feet of industrial park that would be developed in Marysville. The project proponent is also developing approximately 131,566 square feet of business park in Arlington. The development includes areas south of Arlington, west of 51st Avenue NE. The site vicinity is shown in Figure 1. Access to the development is proposed driveways along 51st Avenue NE, 47th Avenue NE, and 160th Street NE. Figure 2 illustrates the preliminary site plan. It is anticipated that the development would be constructed and occupied by 2026.

Study Scope

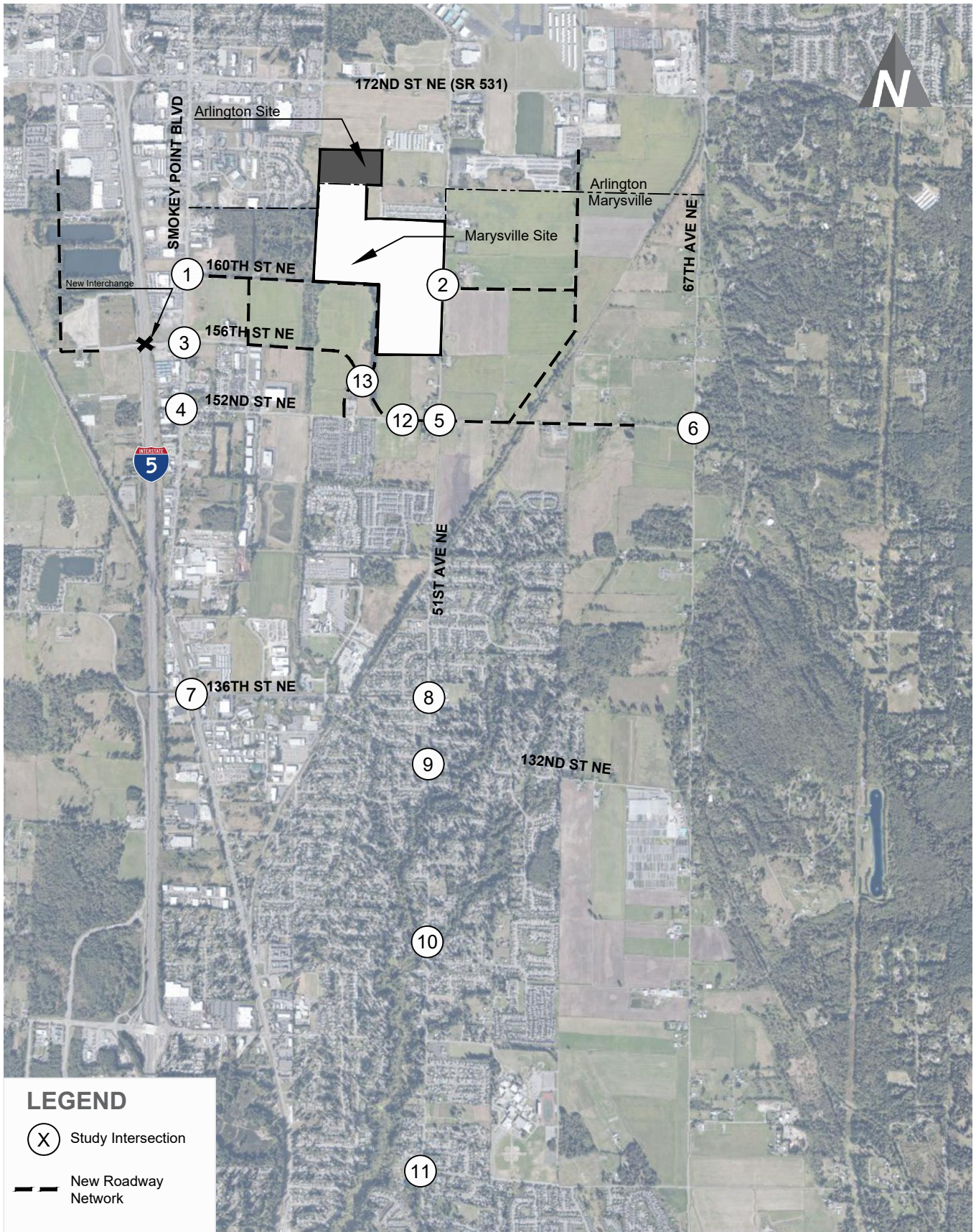
The scope of this analysis is based on anticipated impacts to WSDOT facilities as well as the 25-trip threshold in the City of Marysville. Given the interim and future buildout conditions of the network, additional intersections were added under future 2026 and 2032 conditions that don't exist under existing conditions. Based on anticipated travel patterns for project-generated vehicle traffic, the following intersections were selected for study during the designated years:

<u>Study Intersection</u>	<u>Evaluation Years</u>
• 1 Smokey Point Boulevard/160th Street NE	2032
• 2 51st Avenue NE/160th Street NE	2032
• 3 Smokey Point Boulevard/156th Street NE	Existing, 2026, 2032
• 4 Smokey Point Boulevard/152nd Street NE	Existing, 2026, 2032
• 5 51st Avenue NE/152nd Street NE	Existing, 2026, 2032
• 6 67th Avenue NE/152nd Street NE	Existing, 2026, 2032
• 7 Smokey Point Boulevard/136th Street NE	Existing, 2026, 2032
• 8 51st Avenue NE/136th Street NE	Existing, 2026, 2032
• 9 51st Avenue NE/132nd Street NE	Existing, 2026, 2032
• 10 51st Avenue NE/122nd Place NE	Existing, 2026, 2032
• 11 51st Avenue NE/108th Street NE/Shoultes Road	Existing, 2026, 2032
• 12 152nd Street NE/156th Street NE	2032
• 13 47th Avenue NE/156th Street NE	2032

The scope of the analysis included a review of the weekday PM peak hour conditions. The analysis includes a review of existing conditions in the vicinity of the project site, including the street network, non-motorized facilities, transit service, existing and future (2026 and 2032) without-project peak hour traffic volumes, traffic operations, and traffic safety. Future (2026 and 2032) with-project conditions are evaluated by adding site-generated traffic to future (2026 and 2032) without-project volumes and were then compared to future (2026 and 2032) without-project conditions to identify the relative impacts the proposed project has on the surrounding transportation system.

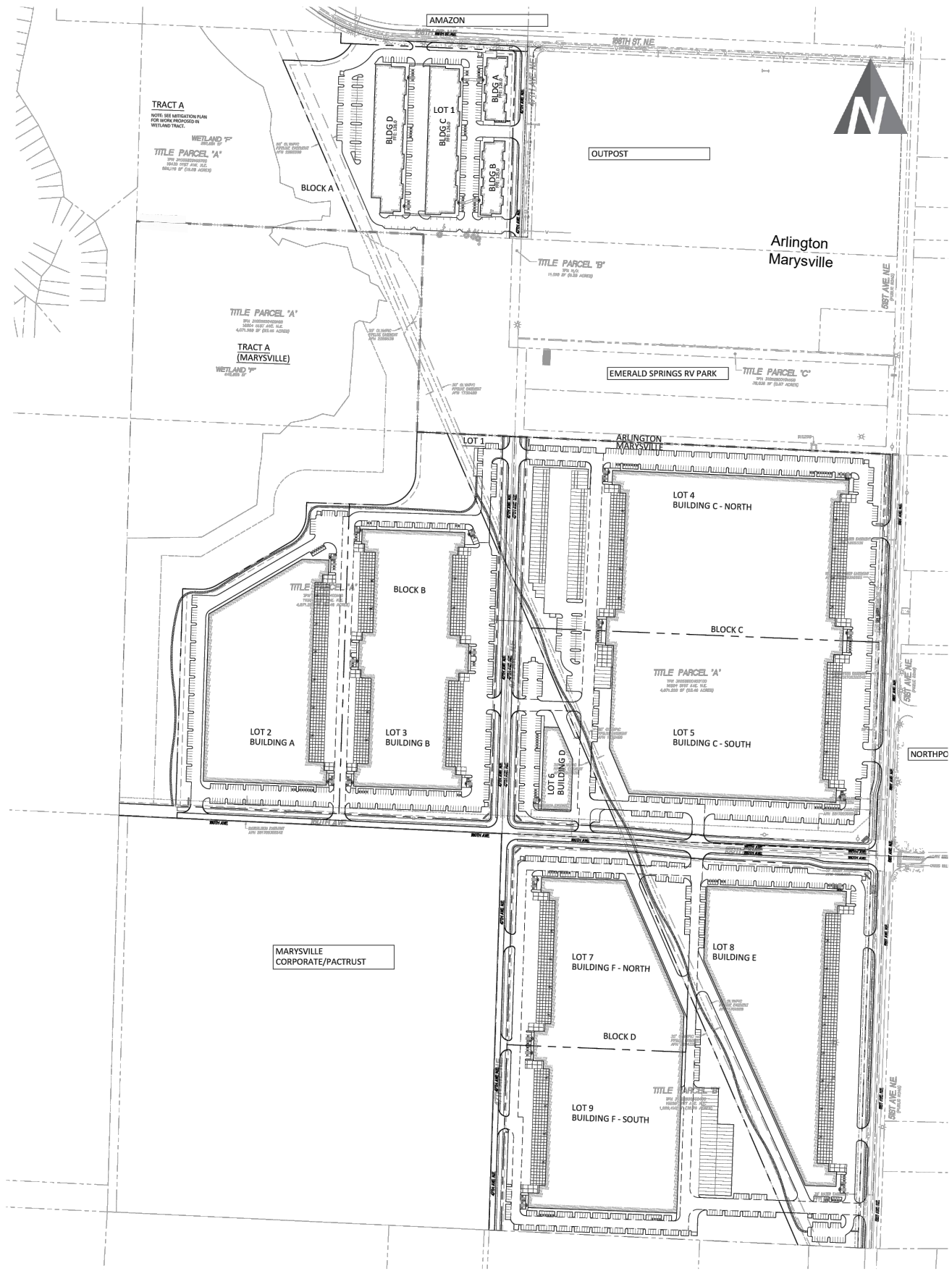
The City of Marysville utilizes two forecast analysis years, the year of opening and a horizon year. The horizon year is defined as six years after the year of opening. The year of opening is utilized for the State Environmental Policy Act (SEPA) disclosure and the horizon year analysis is utilized for the concurrency review.

Development of the parcels in the City of Arlington are included as a pipeline development in this analysis. As such the with-project forecasts and LOS analysis is representative of the cumulative conditions with the full development completed. The analysis of the Arlington parcels are summarized in a separate Traffic Impact Analysis submitted as part of the City of Arlington Land Use process.



Site Vicinity & Study Intersections

FIGURE



Preliminary Site Plan

Williams Industrial

FIGURE

2



Existing & Future Without-Project Conditions

This section describes existing and future (2026 and 2032) without-project conditions within the study area. Study area characteristics are provided for the existing street network, non-motorized facilities, transit service, existing and future without-project peak hour traffic volumes, traffic operations, and traffic safety.

Street System

The following describes the existing street network within the vicinity of the proposed project and any anticipated changes resulting from planned improvements.

Existing Inventory

Characteristics of the existing street system in the project vicinity are described in Table 1.

Table 1. Study Area Existing Roadway Network Summary

Roadway	Roadway Classification	Posted Speed Limit	Number of Travel Lanes	Parking?	Sidewalks?	Bicycle Facilities?
Interstate 5 (I-5)	Interstate	60 to 70 mph ¹	6	No	No	No
132nd Street NE	Arterial	35 mph ³	2	No	No	No
136th Street NE	Arterial	35 mph	2 to 4	No	Intermittent	Intermittent
156th Street NE	Minor Arterial	25 mph	2 to 5	No	Yes	No
152nd Street NE	Major Collector	35 mph	2	No	Intermittent	No
Smokey Point Boulevard	Arterial	35 mph	5	No	Yes	No
43rd Avenue NE	Local	35 mph	2 to 3	No	Intermittent	No
51st Avenue NE	Arterial	35 mph	2 to 3	No	Intermittent	No
59th Avenue NE	Minor Arterial	No Limit Posted	2 to 3	No	Intermittent ⁴	No
67th Avenue NE	Minor Arterial	35 mph	2 to 3	No	Intermittent	No
122nd Place NE	Local	25 mph	2	No	No	No

Note: mph = miles per hour

1. 60 mph south of SR 531 and 70 mph north of SR 531.
2. Other principal arterial between I-5 SB Ramps and Smokey Point Boulevard. Minor arterial between Smokey Point Boulevard and SR 9.
3. 20 mph when in school zone
4. The Airport Trail is located west of 59th Avenue NE.

As shown in Table 1, availability of sidewalks in the area are intermittent. The Centennial Trail is also located in the area, providing recreational and commute possibilities. The Centennial Trail is a 30-mile paved multiuse trail that starts in Snohomish County and ends in Skagit County running through the Cities of Marysville and Arlington. The improvements described below as well as frontage improvements as part of planned developments in the area include construction of non-motorized facilities.

Planned Improvements

Based on a review of the Washington Department of Transportation (WSDOT) 2023-2026 Statewide Transportation Program (STIP), *Snohomish County 2023-2028 Transportation Improvement Plan*, and the *City of Marysville 2023-2028 Transportation Improvement Plan* there are a number of improvements in the area that would impact both capacity at study intersections and travel patterns in the area. The following summarizes the short (2026) and long-term (2032) projects planned in the area.

2026 Planned Improvements

The following improvements were assumed to be complete by the 2026 opening year:

- **156th Street NE Widening:** Widen 156th Street NE from 3 lanes to 5 lanes from Smokey Point Boulevard to west of the Hayho Creek.
- **169th Street NE Extension:** Extension of 169th Street NE from Smokey Point Boulevard to 51st Avenue NE is anticipated to happen over 3 phases. The roadway would include a three-lane cross section with one travel lane in each direction and a center two-way left-turn lane.
- **67th Avenue NE/152nd Street NE All-Way Stop:** Conversion of the side-street stop-controlled intersection to an all-way stop control. Project was completed by Snohomish County in 2022.
- **Shoultes Elementary Safe Routes to School:** Construction of pedestrian facilities including curbs, gutters, sidewalks and bicycle lanes along 51st Avenue NE between 132nd Street NE and 136th Street NE.

2032 Planned Improvements

The following improvements were assumed to be complete by the 2032 horizon year:

- **152nd Street NE Widening:** Widen 152nd Street NE between 51st Avenue NE and the Marysville City limits to three lanes including sidewalk and bike lanes. Widening would provide for one travel lane in each direction and a central two-way left-turn lane.
- **51st Avenue NE/152nd Street NE Signalization:** Construct a traffic signal at the intersection with additional intersection improvements. Intersection improvements would include construction of left-turn and right-turn lanes on all approaches. The resulting intersection would have a four-lane northbound/southbound cross section and a six-lane eastbound/westbound cross section.
- **156th Street NE Extension:** Extension of 156th Street NE as a five-lane section from approximately 47th Avenue NE to 152nd Street NE.
- **168th Street Extension:** Complete the three-lane segment between 47th Avenue NE and 59th Avenue NE.
- **47th Avenue Construction:** Construction of a three-lane facility with one travel lane in each direction and a center two-way left-turn lane between 168th Street NE and 156th Street NE.
- **59th Avenue NE Widening:** Widen 59th Avenue NE between 172nd Street NE (SR 531) and 152nd Street NE to a three-lane segment.
- **160th Street NE Construction:** Construction of a new three-lane roadway (160th Street NE) between Smokey Point Boulevard and 59th Avenue NE.
- **New I-5 Interchange:** Construction of a new single point urban interchange (SPUI) at 156th Street NE and I-5. The construction of the new interchange is anticipated to result in traffic shifts away from the 172nd Street NE (SR 531) corridor.
- **51st Avenue NE/132nd Street NE Widening:** Widening of the intersection to include a southbound left-turn lane.

Additional improvements identified on the County or City of Marysville TIPs but were not included in the analysis because funding of the projects have not been secured. Those improvements include:

- **67th Avenue NE/152nd Street:** Intersection improvements to include construction of a roundabout or a traffic signal at the intersection.
- **51st Avenue NE/132nd Street NE:** Construction of a traffic signal at the intersection.

Transit Service

Transit service in the study area is provided by Community Transit and is primarily provided along 172nd Street NE (SR 531), Smokey Point Boulevard, and 152nd Street NE. The nearest bus stop is located on 152nd Street NE at 47th Avenue NE and is served by route 202.

Route 202 provides service between the Smokey Point Transit Center in Arlington and the Lynwood Transit Center. Service is provided 7 days a week with AM and PM peak hour headways of approximately 15 minutes.

No planned transit improvements were identified. However, it is anticipated that transit service in the area may be adjusted to account for future growth.

Traffic Volumes

The following summarizes the traffic volumes for existing and future without-project conditions.

Existing Traffic Volumes

Existing traffic counts were obtained from the City of Marysville where available or collected at study intersections. All counts were collected between 2022 and 2023. Traffic volumes data collected in 2022 were grown at an average annual growth rate of 3 percent to establish existing 2023 conditions. The annual average growth rate of 3 percent is consistent with the City of Marysville Policy and previous work completed in the area. Detailed traffic counts are provided in Appendix A.

Future Without-Project Traffic Volumes

The following sections describe the methodologies to develop the forecast 2026 and 2032 traffic volumes which represent the year of opening and the horizon year, respectively.

Consistent with City requirements, future 2026 and 2032 without-project traffic volumes were forecasted by applying an annual growth rate of 2 percent to existing traffic volumes and adding traffic from “pipeline” development projects that would also contribute traffic to the study intersections. Three pipeline projects were identified in the vicinity of the project site. The three pipeline projects identified are Project Roxy, a portion of the Cascade Commerce Center, and the Marysville Industrial project. In addition to the three pipeline projects, the Arlington portion of the Williams Industrial project was included in the background.

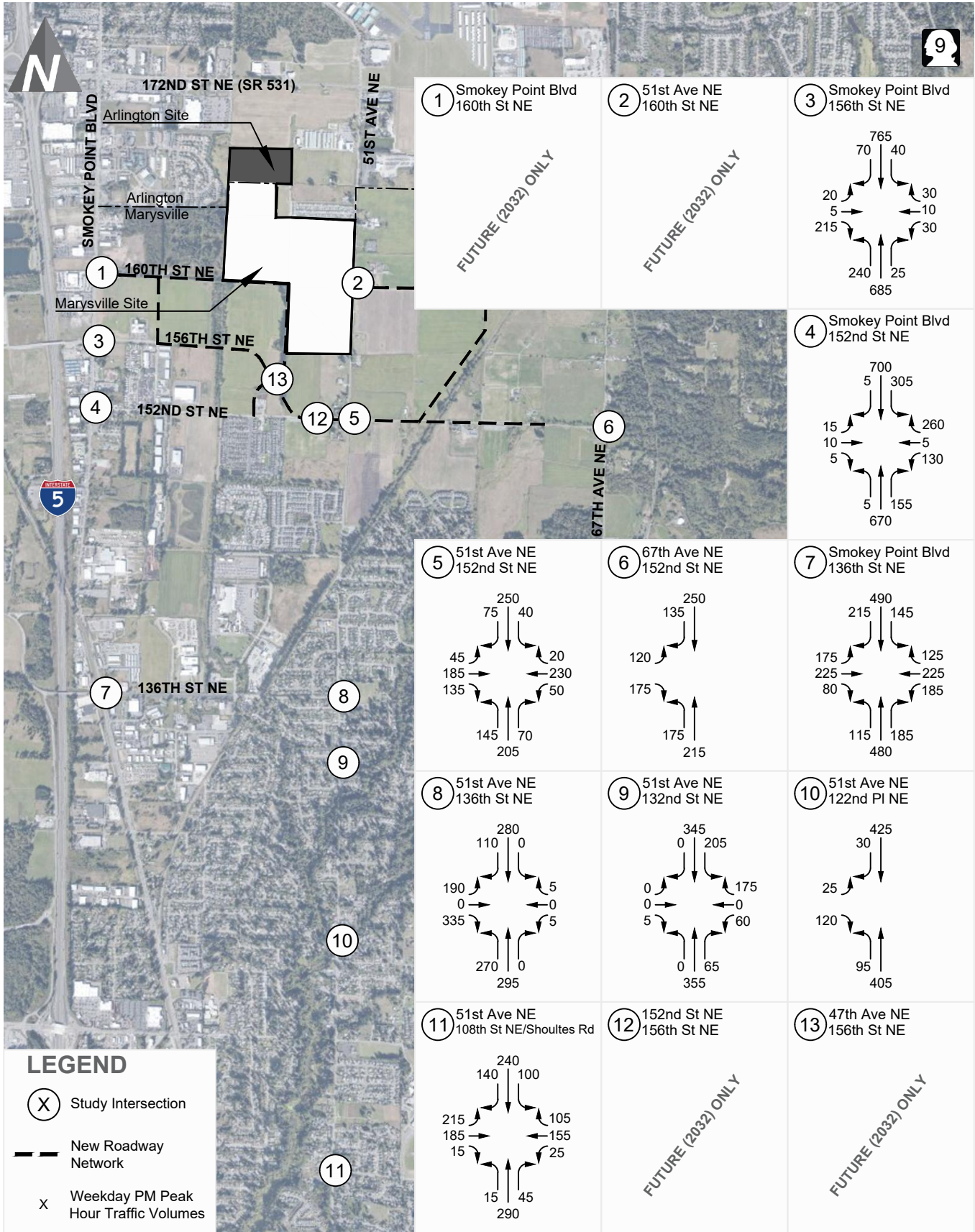
Project Roxy would develop an approximately 2.82 million square foot fulfillment center warehouse located between 172nd Street NE (SR 531) and 169th Street NE, and 43rd Avenue NE and 51st Avenue NE in the City of Arlington. Project Roxy is anticipated to be constructed and occupied by the end of 2022. Based on the timing of the counts used in this analysis and anticipation of the occupancy this project has been included in the background volume forecasting.

The Cascade Commerce Center (CCC) would develop approximately 4.15 million square feet of a mix of industrial uses anticipated to include a combination of industrial park, high-cube warehouse, and high-cube fulfillment center. The CCC is located in both the City of Arlington and the City of Marysville approximately south of 172nd Street NE (SR 531) and between 51st Avenue NE/59th Avenue NE and the railroad track to the east. The CCC is anticipated to be completed across 9 buildings which would be built and occupied separately. The full site is anticipated to be constructed by 2030; however, full buildout of the project has not been approved. Based on coordination with City staff only 250,000 square feet of warehouse have been approved. As such, a 250,000 square foot warehouse was assumed under future 2026 and 2032 without-project conditions.

The PacTrust Project would develop approximately 745,250 square feet of Industrial Park. The development includes areas north and south of 156th Street NE. Access to the development is provided by proposed driveways along 47th Avenue NE and 156th Street NE. It is anticipated that the development would be constructed and occupied by 2025.

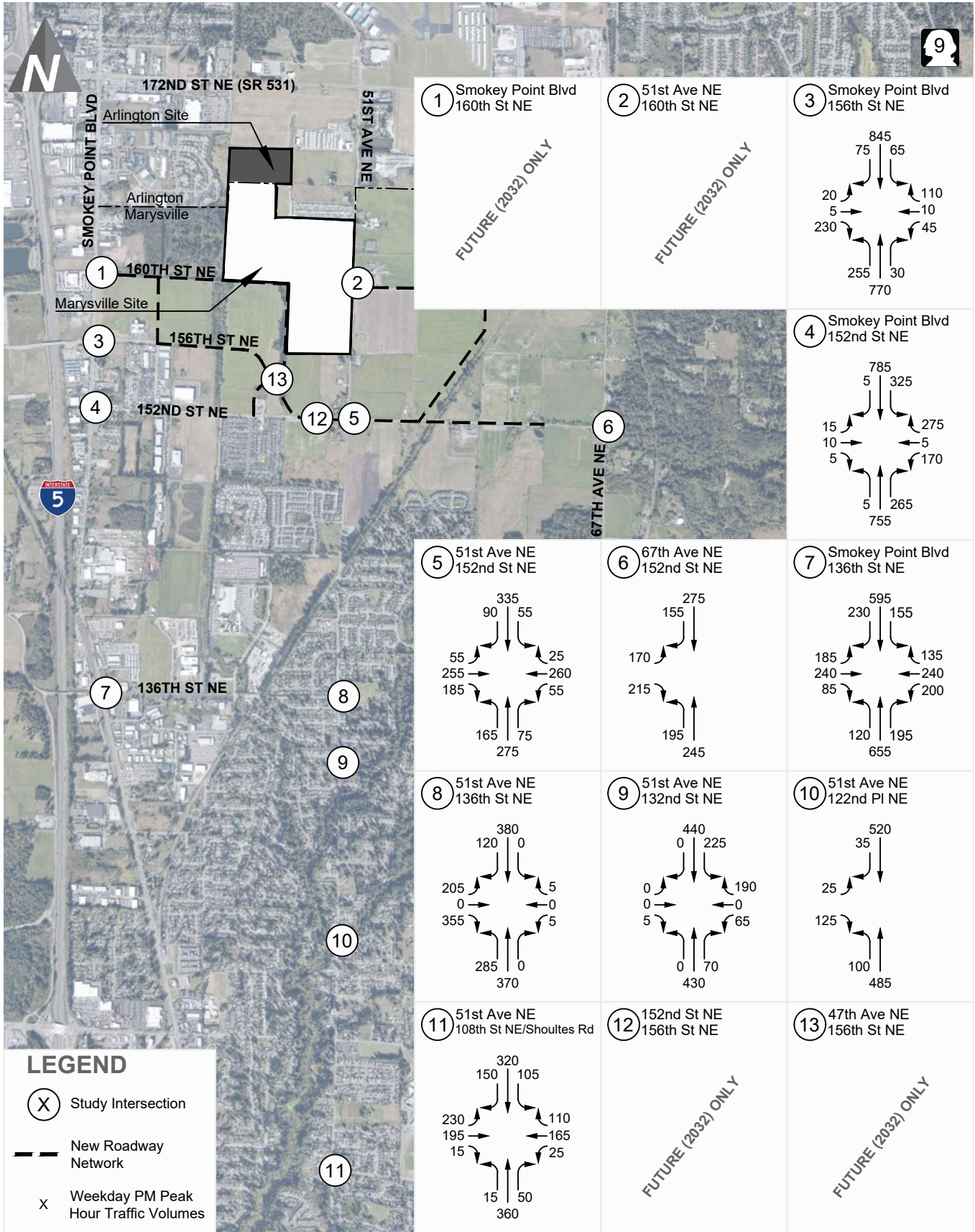
Williams Industrial (Marysville) development of up to 131,566 square feet of business park. IT is anticipated that the development would be constructed and occupied by 2026.

Forecast future 2026 and 2032 without-project traffic volumes for the future horizon year are shown in Figure 4 and Figure 5.

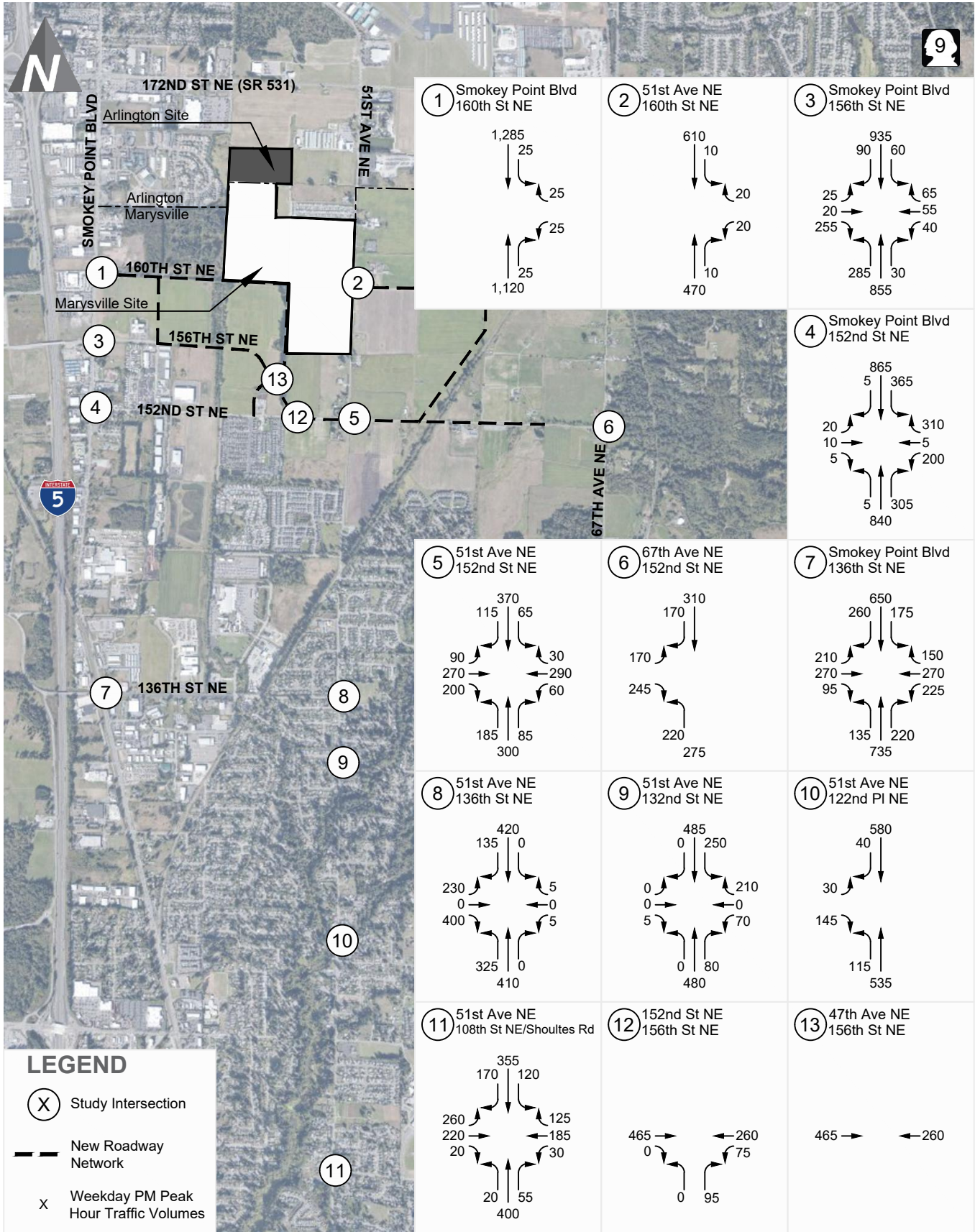


Existing Weekday PM Peak Hour Traffic Volumes

FIGURE



Future (2026) Without-Project Weekday PM Peak Hour Traffic Volumes FIGURE



Future (2032) Without-Project Weekday PM Peak Hour Traffic Volumes FIGURE

Traffic Operations

The following sections summarize traffic operations for existing and future conditions within the study area.

The operational characteristics of an intersection are determined by calculating the intersection level of service (LOS). At signalized, all-way stop, and roundabout intersections, LOS is measured in average control delay per vehicle and is typically reported using the intersection delay. At unsignalized side-street, stop-controlled intersections, LOS is measured by the average delay on the worst-movement of the intersection. Traffic operations and average vehicle delay for an intersection can be described qualitatively with a range of levels of service (LOS A through LOS F), with LOS A indicating free-flowing traffic and LOS F indicating extreme congestion and long vehicle delays. Appendix B contains a detailed explanation of LOS criteria and definitions.

For the operations analysis of existing conditions at the signalized study intersections, signal timing and phasing information was obtained from the WSDOT or the City of Marysville. As described previously, there are a number of improvements planned by 2026 and 2032 that would impact intersection capacity and travel patterns. Those improvements were accounted for in the future 2026 and 2032 without-project analysis.

Weekday PM peak hour traffic operations for existing and future without-project conditions were evaluated at the study intersections based on the procedures identified in the *Highway Capacity Manual* 6th Edition, unless otherwise noted for signal timing constraints, and were evaluated using *Synchro 11*. *Synchro 11* is a software program that uses *HCM* methodology to evaluate intersection LOS and average vehicle delays. Roundabout controlled intersections were evaluated utilizing *Sidra 9* and the WSDOT methodology for analyzing roundabouts. Results for the existing and future without-project operations analyses are summarized in Table 2. Detailed LOS worksheets for each intersection analysis are included in Appendix C.

The City of Marysville has an LOS E mitigated standard for arterial-arterial or arterial-collector intersections along the Smokey Point Boulevard corridor between the north and south City limits. The remaining intersections carry an LOS D standard. The intersection of 67th Avenue NE/152nd Street NE is under Snohomish County jurisdiction. Snohomish County evaluates concurrency on a corridor basis and as such does not have an individual intersection standard to be applied to this intersection.

Table 2. Existing Weekday PM Peak Hour Intersection LOS Summary

Intersection	Traffic Control	Existing			Future 2026 Without-Project			Future 2032 Without-Project		
		LOS ¹	Delay ²	v/c ³	LOS	Delay	v/c	LOS	Delay	v/c
1. Smokey Point Blvd/160th St NE	Signal	2032 Only			2032 Only			A	6.6	-
2. 51st Ave NE/160th St NE	AWSC	2032 Only			2032 Only			A	6.3	-
3. Smokey Point Blvd/156th St NE	Signal	B	15.0	-	C	22.1	-	C	27.4	-
4. Smokey Point Blvd/152nd St NE	Signal	F	117.3	-	F	143.1	-	D	53.8	-
5. 51st Ave NE/152nd St NE	AWSC/Signal ⁴	F	58.3	-	F	171.8	-	B	18.2	-
6. 67th Ave NE/152nd St NE	AWSC	C	15.7	-	C	24.4	-	E	38.5	-
7. Smokey Point Blvd/136th St NE	Signal	D	38.4	-	D	41.5	-	D	40.0	-
8. 51st Ave NE/136th St NE ⁵	Signal	B	14.5	-	B	16.8	-	B	18.7	-
9. 51st Ave NE/132nd St NE	AWSC	D	27.8	-	F	67.2	-	E	45.1	-
10. 51st Ave NE/122nd PI NE	Signal	A	9.4	-	A	8.9	-	A	9.8	-
11. 51st Ave NE/Shoultes Rd/108th St NE	RAB	A	7.7	0.42	A	8.3	0.51	A	8.5	0.55
12. 156th St NE/152nd St NE ⁵	Signal	2032 Only			2032 Only			B	10.5	-
13. 47th Ave NE/156th NE	Signal	2032 Only			2032 Only			B	18.9	-

Note: AWSC = All-way stop-control, RAB = roundabout.

1. Level of Service (A – F) as defined by the *Highway Capacity Manual* (HCM) 6th Edition (TRB)

2. Average delay per vehicle in seconds.

3. Volume to capacity ratio reported for roundabouts.

4. Signal under future 2032 conditions.

5. Intersections evaluated utilizing HCM 2000 methodology due to signal timing constraints not allowed under HCM 6th Edition.

As shown in Table 2, under existing conditions, two intersections operate at LOS F and do not meet LOS standards. Under future (2026) without-project conditions, with inclusion of area planned improvements and increases in background traffic, the number of intersections forecast to not meet the applicable LOS standard is three intersections. The intersections forecast to not meet the LOS standard in 2026 include:

- Smokey Point Boulevard/152nd Street NE
- 51st Avenue NE/152nd Street NE
- 51st Avenue NE/132nd Street NE

Under future (2032) without-project conditions, one intersections are forecast not to meet the City of Marysville LOS standards. The 51st Avenue NE/132nd Street NE would improve from LOS F to LOS E with the implementation of the southbound left-turn lane but would not meet LOS standards.

Traffic Safety

Recent collision records were reviewed within the study area to identify existing traffic safety issues at the study intersections. The most recent complete five-year summary of accident data from the WSDOT is for the period between January 1, 2018 and December 31, 2022. This information is summarized in Table 3.

Table 3. Five-Year Collision Summary – 2018 to 2022

Location	Number of Collisions					Total	Annual Average
	2018	2019	2020	2021	2022		
3. Smokey Point Blvd/156th St NE	5	1	3	5	4	18	3.60
4. Smokey Point Blvd/152nd St NE	5	7	5	3	4	24	4.80
5. 51st Ave NE/152nd St NE	3	4	3	6	1	17	3.40
6. 67th Ave NE/152nd St NE	3	5	1	3	1	13	2.60
7. Smokey Point Blvd/136th St NE	2	6	3	5	6	22	4.40
8. 51st Ave NE/136th St NE ⁵	0	0	3	1	1	5	1.00
9. 51st Ave NE/132nd St NE	1	2	0	3	0	6	1.20
10. 51st Ave NE/122nd St NE	0	1	0	1	0	2	0.40
11. 51st Ave NE/Shoultes Rd/108th St NE	9	5	4	5	2	25	5.00

Source: WSDOT, 2023

Under 23 U.S. Code § 409 and 23 U.S. Code § 148, safety data, reports, surveys, schedules, lists compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subject to discovery or admitted into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

As shown in Table 3, the total number of collisions at the study area intersections ranged between 2 and 25 over the five years resulting in an annual average between 0.40 and 5.00 collisions per year. In the study area there were a total of 3 reported collisions that involved either a pedestrian or a cyclist. There was one reported fatality in the study area over the five-year period. The fatality was reported at the 67th Avenue NE/152nd Street NE intersection and involved an impaired driver exceeding safe speeds.

The highest number of reported collisions reported was at the 51st Avenue NE/Shoultes Road/108th Street NE intersection. Of the 25 reported collisions the majority were property damage only (88 percent). No pattern or safety issue are identified at the intersection.

Project Impacts

This section of the report documents the proposed project’s impacts on the surrounding street network and study intersections. First, estimated traffic volumes generated by the proposed project are distributed and assigned to adjacent streets and intersections within the study area for the weekday PM peak hour study period. Next, project trips are added to future without-project traffic volumes and any potential impact to traffic operations. Site specific items are also discussed such as the operation of the site’s access driveway.

Trip Generation

The proposed project is constructing approximately 131,566 square feet of Business Park in Arlington and 2,055,069 square feet of Industrial Park in Marysville. Trip generation estimates have been prepared for the development based on trip rates identified using the Institute of Transportation Engineers (ITE) Trip Generation Manual, 11th Edition (2021) based on Business Park (LU #770) and Industrial Park (LU #130). The site is currently undeveloped.

Table 4 provides a summary of the trip generation for the proposed land use in Arlington and Marysville. A detailed summary of the trip generation calculations for these uses has been provided in Appendix D.

Table 4. Estimated Weekday Vehicle Trip Generation

Land Use	Size	Daily Trips ¹	AM Peak-Hour Trips			PM Peak-Hour Trips		
			In	Out	Total	In	Out	Total
<i>Proposed</i>								
Marysville – Industrial Park (LU #130)	2,055,069 sf	6,926	566	133	699	154	545	699

Notes: sf = square-feet
1. Vehicle trips were estimated based on vehicle trip calculations and localized mode split information.

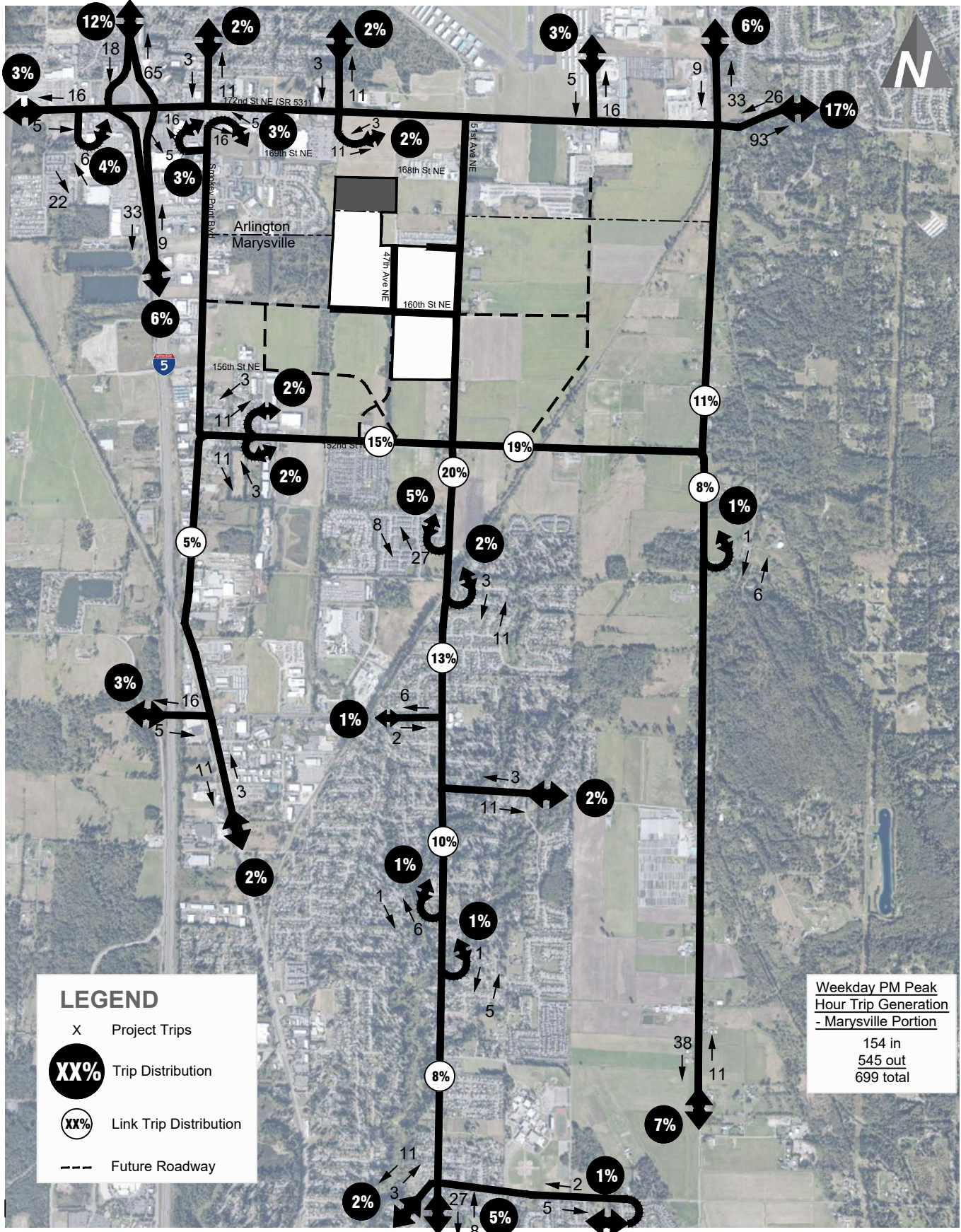
As shown in Table 4, within the Marysville parcels, the project is anticipated to generate 6,926 weekday daily trips with 699 during the AM peak hour and 699 during the PM peak hour.

Trip Distribution & Assignment

Trip distribution patterns developed for the project were based on information provided by the City of Marysville which are reflective of the anticipated travel patterns for the industrial center. The 2026 year of opening trip distribution utilized the existing network, and the 2032 horizon year utilized the anticipated build out network with the exception of the 152nd Street NE extension to SR 9. The project is currently unfunded and not expected to be completed by 2032.

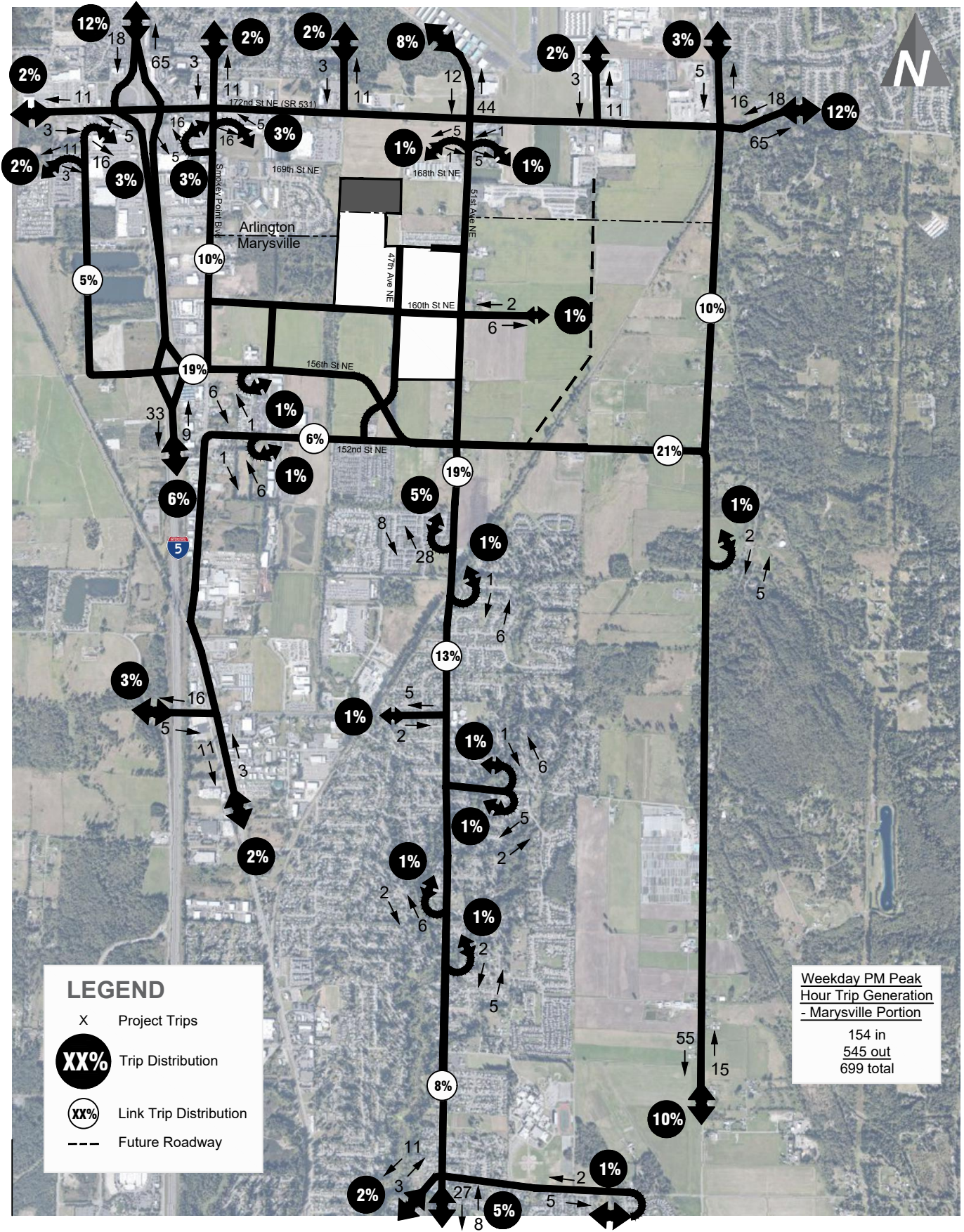
Figure 6 illustrates the year of opening (2026) vehicle trip distribution and assignment for the proposed project. Figure 7 illustrates the horizon year (2032) vehicle trip distribution and assignment for the proposed project. The resulting 2026 weekday PM peak hour traffic volumes are shown on Figure 8 and the 2032 PM peak hour traffic volumes are shown on Figure 9.

Additionally, consistent with Snohomish County requirements, project trips have been shown at key intersections impacted by three or more directional trips on an approach or departure. The project trips are shown graphically and in tabular form in Appendix E.



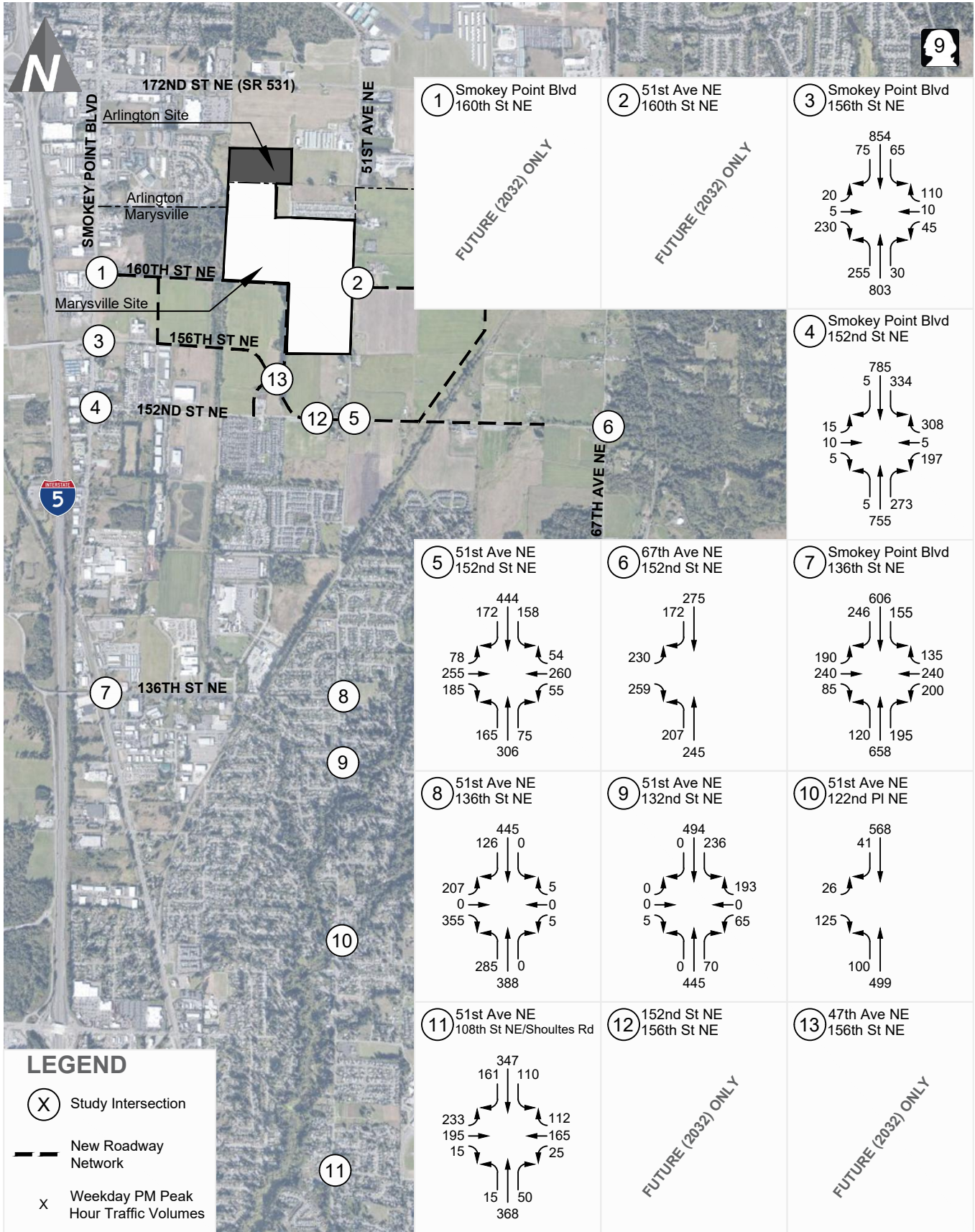
Year of Opening (2026) Project Trip Distribution & Assignment

FIGURE



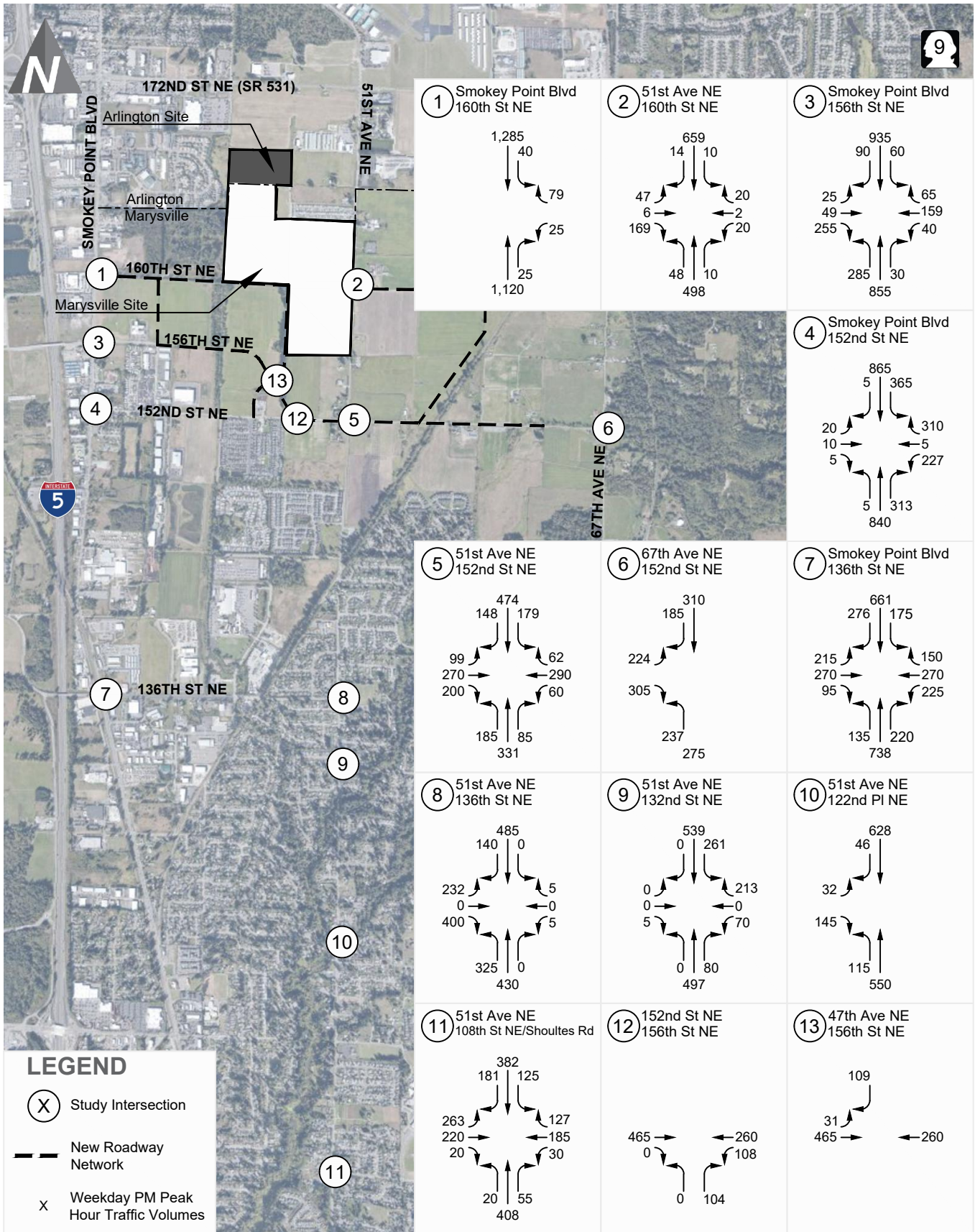
Horizon Year (2032) Project Trip Distribution & Assignment

FIGURE 7



Future (2026) With-Project Weekday PM Peak Hour Traffic Volumes

FIGURE



Future (2032) With-Project Weekday PM Peak Hour Traffic Volumes

FIGURE

Volume Impact Summary

The assigned project generated traffic was added to the future without-project weekday PM peak hour traffic volumes at the study intersections under forecast 2026 and 2032 conditions. Table 5 summarizes the project share of traffic volumes at the study intersections during the weekday PM peak hour under forecast 2026 year of opening conditions. Table 6 summarizes the forecast 2032 horizon year weekday PM peak hour project share.

Table 5. Future (2026) Weekday PM Peak Hour Traffic Volume Impacts at Study Intersections

Intersection	PM Peak Hour Total Entering Vehicles			Project Share
	2026 Without-Project	Project Trips	2026 With-Project	
3. Smokey Point Blvd/156th St NE	2,460	42	2,502	1.7%
4. Smokey Point Blvd/152nd St NE	2,620	77	2,697	2.9%
5. 51st Ave NE/152nd St NE	1,830	377	2,207	17.1%
6. 67th Ave NE/152nd St NE	1,255	133	1,388	9.6%
7. Smokey Point Blvd/136th St NE	3,035	35	3,070	1.1%
8. 51st Ave NE/136th St NE	1,725	91	1,816	5.0%
9. 51st Ave NE/132nd St NE	1,425	84	1,509	5.6%
10. 51st Ave NE/122nd PI NE	1,295	71	1,366	5.2%
11. 51st Ave NE/Shoultes Rd/108th St NE	1,740	56	1,796	3.1%

As shown in Table 5, the proposed project is estimated to account for approximately 5 percent of the total weekday PM peak hour traffic at the majority of study intersections. Traffic volumes fluctuate day-to-day and the anticipated traffic increase at study intersections are within the range of typical daily traffic fluctuations which can be up to 5 percent. There are two intersections where the project is forecast to represent more than 5 percent of the total volumes, which are intersections closer to the site, and include:

- 51st Avenue NE/152nd Street NE
- 67th Avenue NE/152nd Street NE

Table 6. Future (2032) Weekday PM Peak Hour Traffic Volume Impacts at Study Intersections

Intersection	PM Peak Hour Total Entering Vehicles			Project Share
	2032 Without-Project	Project Trips	2032 With-Project	
1. Smokey Point Blvd/160th St NE	2,505	69	2,574	2.7%
2. 51st Ave NE/160th St NE	1,140	363	1,503	24.4%
3. Smokey Point Blvd/156th St NE	2,715	133	2,848	4.7%
4. Smokey Point Blvd/152nd St NE	2,935	35	2,970	1.2%
5. 51st Ave NE/152nd St NE	2,060	321	2,381	13.5%
6. 67th Ave NE/152nd St NE	1,390	146	1,536	9.5%
7. Smokey Point Blvd/136th St NE	3,395	35	3,430	1.0%
8. 51st Ave NE/136th St NE	1,930	91	2,021	4.5%
9. 51st Ave NE/132nd St NE	1,580	84	1,664	5.0%
10. 51st Ave NE/122nd PI NE	1,445	71	1,516	4.7%
11. 51st Ave NE/Shoultes Rd/108th St NE	1,960	56	2,016	2.8%
12. 156th St NE/152nd St NE	895	42	937	4.5%
13. 47th Ave NE/156th NE	725	140	865	16.2%

Under the horizon year conditions, with buildout of the network, the project is anticipated to represent between 1 and approximately 24 percent of the traffic volumes at the study intersections with the higher percentages closer to the site.

Traffic Operations

The following section summarizes the future with-project LOS at the study intersections relative to the without-project conditions to identify project-related impacts.

Intersection parameters such as channelization and intersection control applied to the future with-project analyses were consistent with those used in the evaluation of future without-project conditions. Signal timing splits were optimized under future 2032 without project conditions. Those signal timings were then utilized for the future (2032) with-project conditions. A comparison of the future 2026 year of opening without-project and with-project weekday PM peak hour traffic operations are summarized in Table 7. A comparison of the future 2032 horizon year without-project and with-project weekday PM peak hour traffic operations are summarized in Table 8. Detailed LOS worksheets are provided in Appendix C.

Table 7. Year of Opening (2026) Weekday PM Peak Hour Intersection LOS Summary

Intersection	Traffic Control	Future 2026 Without-Project			Future 2026 With-Project		
		LOS ¹	Delay ²	v/c ⁴	LOS	Delay	v/c
3. Smokey Point Blvd/156th St NE	Signal	C	22.1	-	C	21.5	-
4. Smokey Point Blvd/152nd St NE	Signal	F	143.1	-	F	156.5	-
5. 51st Ave NE/152nd St NE	AWSC	F	171.8	-	F	322.9	-
6. 67th Ave NE/152nd St NE	AWSC	C	24.4	-	E	40.2	-
7. Smokey Point Blvd/136th St NE	Signal	D	41.5	-	D	41.7	-
8. 51st Ave NE/136th St NE ⁴	Signal	B	16.8	-	B	19.6	-
9. 51st Ave NE/132nd St NE	AWSC	F	67.2	-	F	93.2	-
10. 51st Ave NE/122nd St NE	Signal	A	8.9	-	A	9.3	-
11. 51st Ave NE/Shoultes Rd/108th St NE	RAB	A	8.3	0.51	A	8.5	0.55

Note: AWSC = All-way stop-control, RAB = roundabout.

1. Level of Service (A – F) as defined by the *Highway Capacity Manual* (HCM) 6th Edition (TRB)

2. Average delay per vehicle in seconds.

3. Volume to capacity ratio reported for roundabouts.

4. Intersections run utilizing HCM 2000 methodology due to signal timing constraints not allowed under HCM 6th Edition.

As shown in Table 7, all study intersections remain at the same LOS under with-project conditions with the exception of one intersection. The 67th Avenue NE/152nd Street NE intersection is forecast to degrade from LOS C to LOS E under with-project conditions. Under without or with-project conditions three intersections are anticipated to operate at LOS F and would not meet LOS standards.

This is an interim condition and as described in the following section, longer term improvements are being funded through the City’s impact fee program at the intersections that would include widening and signalization or construction of roundabouts. Additionally, the County has identified a project at the 67th Avenue NE/152nd Street NE intersection that would include implementation of a signal or roundabout and widening at the intersection. The timing of this project hasn’t been identified and therefore was not assumed in the horizon year described below.

Table 8. Horizon Year (2032) Weekday PM Peak Hour Intersection LOS Summary

Intersection	Traffic Control	Future 2032 Without-Project			Future 2032 With-Project		
		LOS ¹	Delay ²	v/c ³	LOS	Delay	v/c
1. Smokey Point Blvd/160th St NE	Signal	A	6.6	-	A	8.2	-
2. 51st Ave NE/160th St NE	AWSC	A	6.3	-	C	22.1	-
3. Smokey Point Blvd/156th St NE	Signal	C	27.4	-	C	27.5	-
4. Smokey Point Blvd/152nd St NE	Signal	D	53.8	-	D	54.6	-
5. 51st Ave NE/152nd St NE	Signal	B	18.2	-	C	20.8	-
6. 67th Ave NE/152nd St NE	AWSC	E	38.5	-	F	67.0	-
7. Smokey Point Blvd/136th St NE	Signal	D	40.0	-	D	40.5	-
8. 51st Ave NE/136th St NE ⁵	Signal	B	18.7	-	C	21.2	-
9. 51st Ave NE/132nd St NE	AWSC	E	45.1	-	F	56.5	-
10. 51st Ave NE/122nd St NE	Signal	A	9.8	-	B	10.2	-
11. 51st Ave NE/Shoultes Rd/108th St NE	RAB	A	8.5	0.55	B	10.3	0.63
12. 156th St NE/152nd St NE ⁵	Signal	B	10.5	-	B	11.0	-
13. 47th Ave NE/156th St NE	Signal	B	18.9	-	B	19.1	-

Note: AWSC = All-way stop-control, RAB = roundabout.

1. Level of Service (A – F) as defined by the *Highway Capacity Manual* (HCM) 6th Edition (TRB)

2. Average delay per vehicle in seconds.

3. Volume to capacity ratio reported for roundabouts.

4. Signal under future 2032 conditions.

5. Intersections run utilizing HCM 2000 methodology due to signal timing constraints not allowed under HCM 6th Edition.

As shown in Table 8, with completion of the roadway network and construction of the new interchange at 156th Street NE, two of the study intersections are not forecast to meet the LOS standards. Under forecast 2032 conditions the 67th Avenue NE/152nd Street NE and 51st Avenue NE/132nd Street NE intersections are forecast to degrade from LOS E to LOS F with between without and with-project conditions.

As noted previously, the City of Marysville has an LOS E mitigated standard for arterial-arterial or arterial-collector intersections along the Smokey Point Boulevard corridor between the north and south City limits. The remaining intersections carry an LOS D standard.

Snohomish County doesn't have an individual intersection LOS standard. As such, no mitigation is proposed at the 67th Avenue NE/152nd Street NE intersection. The County has identified the need for improvements at the intersection and the project proponent will contribute impact fees to the County, funding future improvements.

The 51st Avenue NE/132nd Street NE intersections don't meet concurrency standards for the City of Marysville and would require mitigation. The City, as part of the 2023-2028 TIP has identified the need for a signal at the intersection; however, the project is unfunded. As part of mitigation for the proposed project the project proponent will construct the traffic signal at the intersection.

Site Access

Site access is proposed via 13 driveways with access to 47th Avenue NE, 51st Avenue NE, and 160th Street NE. As shown in Table 8, the 51st Avenue NE/160th Street NE intersection, which is a proximate to most site driveways is shown to operate at LOS C under future (2032) with-project conditions, as such this intersection would be representative of operations of the site driveways. As such, no operational issues are anticipated with site driveways given the number and locations of the driveways. Based on the geography of the site, sight distance issues are not anticipated; however, object heights less than 3.5 feet would be maintained at sight driveways.

Mitigation and Recommendations

The proposed project would provide half-street construction of portions of 160th Street NE and 47th Avenue NE as well as frontage improvements along 51st Avenue NE. The project impacts to the surrounding transportation system would be mitigated through the City of Marysville, Snohomish County, and WSDOT impact fee programs.

Transportation Mitigation Fees

To mitigate impacts of the proposal on the surrounding transportation system, the developer would be required to pay impact fees to three jurisdictions: the City of Marysville, Snohomish County, and WSDOT based on current interlocal agreements that have been established between these entities. The following provides an estimate only, the final fees will be calculated at time of permit issuance.

City of Marysville

The City of Marysville traffic mitigation fees are currently \$2,220 per PM peak hour trip. Based on the anticipated trip generation of 699 trips the resulting City of Marysville impact fee would be **\$1,551,780** ($\$2,220/\text{trip} \times 699 \text{ trips}$). These fees will be reduced for TIF eligible improvements constructed by the applicant. The fee rate is subject to annual increases and will be based on the adopted rates at the time of building permit issuance. As noted previously, the project proponent will construct a traffic signal at the 51st Avenue NE/132nd Street NE intersection as mitigation for project related impacts. The project proponent will seek a credit in impact fees assessed for the project as the intersection is not meeting standards without the proposed project and the City has identified this improvement on the adopted TIP.

Snohomish County

Snohomish County has an interlocal agreement with the City of Marysville. Per the Snohomish County Traffic Mitigation Worksheet for City Developments Impacting County Streets, the percentage of trips impacting County Streets was determined to be 20 percent. Per SCC 30.66B.330 the fee for commercial uses within the urban growth area of TSA is \$157 per average daily trip (ADT). The resulting fee was estimated to be approximately **\$217,476.40** ($20\% \times 6,926 \text{ ADT} \times \157 per ADT). The fee rate is subject to annual increases and will be based on the adopted rates at the time of building permit issuance.

WSDOT

Per the interlocal agreement with WSDOT, project-related impacts can be mitigated through the payment of a flat fee of \$36 per ADT or a proportional share based on the WSDOT projects currently planned. Based on the project distribution, the project would impact the WSDOT projects on 172nd Street NE (SR 531) and the I-5 Interchange at 156th Street NE; however, those projects have been identified as funded. As such no impact fees would be paid to WSDOT.

Findings and Conclusions

This transportation impact analysis summarizes the transportation impacts associated with the proposed industrial project in Marysville, WA.

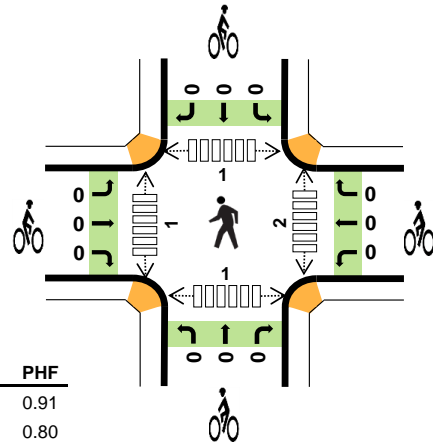
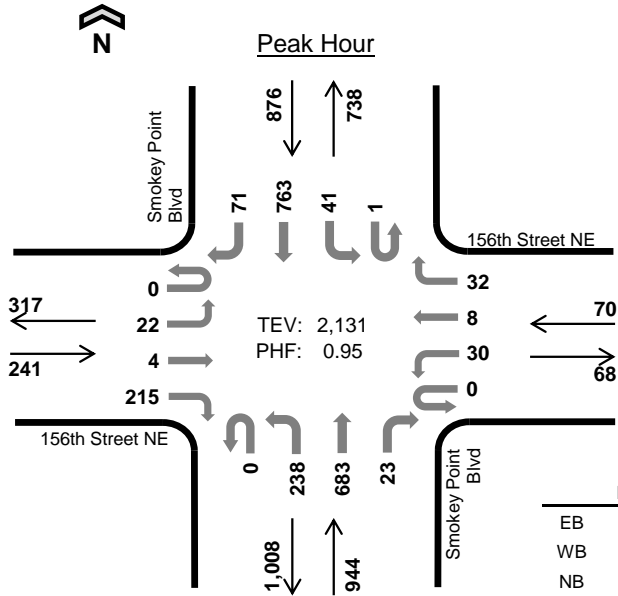
- The proposed project would construct approximately 2,055,069 square feet of industrial park.
- The development is anticipated to generate 6,926 weekday daily trips, with 699 trips occurring during the weekday AM peak hour and 699 trips during the PM peak hour.
- Under 2026 year of opening conditions all study intersections remain at the same LOS under with-project conditions with the exception of one intersection. The 67th Avenue NE/152nd Street intersection is forecast to degrade from LOS C to LOS E under with-project conditions.
- Under 2032 horizon year conditions one study intersection is anticipated to be below the LOS standard of LOS D with or without the proposed project. The 51st Avenue NE/132nd Street NE intersections are forecast to degrade from LOS E to LOS F and would not meet the LOS D standard. Improvements at the 51st Avenue NE/132nd Street NE intersection has been identified by the City of Marysville which includes signalization of the intersection. The project proponent is proposing to construct a signal at the 51st Avenue NE/132nd Street NE intersection as mitigation for project impacts. The 67th Avenue NE/152nd Street NE intersection doesn't carry an intersection LOS standard; therefore, no improvements have been identified at the intersection. However, the project proponent will contribute impact fees to the County.
- Access to the project is proposed driveways along 51st Avenue NE, 47th Avenue NE, and 160th Street NE.
- The developer would be required to pay transportation mitigation fees. The mitigation fee is estimated to be a total of \$1,769,256.40 based on the project's trip generation. Fees will be adjusted to reflect improvements constructed by the project proponent.

Appendix A: Traffic Counts

Smokey Point Blvd 156th Street NE



Date: 05/31/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	2.5%	0.91
WB	4.3%	0.80
NB	2.8%	0.90
SB	3.7%	0.87
TOTAL	3.1%	0.95

Two-Hour Count Summaries

Interval Start	156th Street NE Eastbound				156th Street NE Westbound				Smokey Point Blvd Northbound				Smokey Point Blvd Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	5	0	47	0	9	2	16	0	55	188	4	0	9	153	12	500	0	
4:15 PM	0	5	2	48	0	10	3	9	0	56	177	4	1	8	186	12	521	0	
4:30 PM	0	6	0	57	0	10	2	8	0	57	193	11	0	17	177	25	563	0	
4:45 PM	0	9	1	56	0	5	2	12	0	64	144	6	0	8	172	19	498	2,082	
5:00 PM	0	2	1	54	0	5	1	3	0	61	169	2	0	8	228	15	549	2,131	
5:15 PM	0	9	1	63	0	6	3	10	0	52	138	3	0	8	177	12	482	2,092	
5:30 PM	0	7	1	60	0	5	0	7	0	48	128	3	0	3	185	12	459	1,988	
5:45 PM	0	3	1	41	0	2	2	5	0	43	141	1	0	8	162	8	417	1,907	
Count Total	0	46	7	426	0	52	15	70	0	436	1,278	34	1	69	1,440	115	3,989	0	
Peak Hour	All	0	22	4	215	0	30	8	32	0	238	683	23	1	41	763	71	2,131	0
	HV	0	0	1	5	0	0	1	2	0	4	15	7	0	8	22	2	67	0
	HV%	-	0%	25%	2%	-	0%	13%	6%	-	2%	2%	30%	0%	20%	3%	3%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	5	2	8	15	0	0	0	1	1	0	1	0	0	1
4:15 PM	1	1	4	6	12	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	4	15	20	0	0	0	0	0	1	0	1	0	2
4:45 PM	2	0	11	2	15	0	0	0	0	0	1	0	0	1	2
5:00 PM	3	1	7	9	20	0	0	0	0	0	0	1	0	0	1
5:15 PM	0	5	3	6	14	0	0	0	0	0	1	0	0	0	1
5:30 PM	1	2	3	10	16	0	0	0	0	0	0	0	0	0	0
5:45 PM	1	0	5	9	15	0	0	0	0	0	0	1	0	0	1
Count Total	8	15	39	65	127	0	0	0	1	1	3	3	1	1	8
Peak Hour	6	3	26	32	67	0	0	0	0	0	2	1	1	1	5

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	156th Street NE				156th Street NE				Smokey Point Blvd				Smokey Point Blvd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	0	0	3	0	2	0	0	2	0	0	1	7	0	15	0
4:15 PM	0	0	0	1	0	0	0	1	0	0	3	1	0	0	6	0	12	0
4:30 PM	0	0	0	0	0	0	1	0	0	0	3	1	0	5	10	0	20	0
4:45 PM	0	0	1	1	0	0	0	0	0	2	5	4	0	1	1	0	15	62
5:00 PM	0	0	0	3	0	0	0	1	0	2	4	1	0	2	5	2	20	67
5:15 PM	0	0	0	0	0	3	0	2	0	0	3	0	0	0	6	0	14	69
5:30 PM	0	0	0	1	0	0	0	2	0	0	3	0	0	1	9	0	16	65
5:45 PM	0	0	0	1	0	0	0	0	0	2	3	0	0	0	9	0	15	65
Count Total	0	0	1	7	0	6	1	8	0	6	26	7	0	10	53	2	127	0
Peak Hour	0	0	1	5	0	0	1	2	0	4	15	7	0	8	22	2	67	0

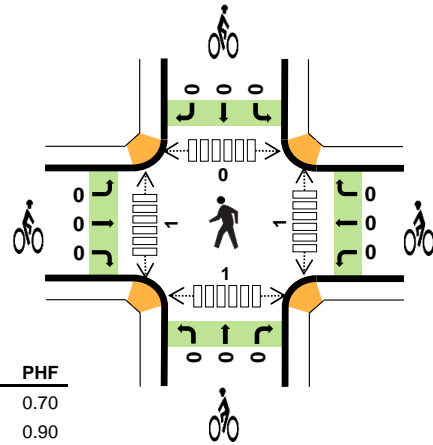
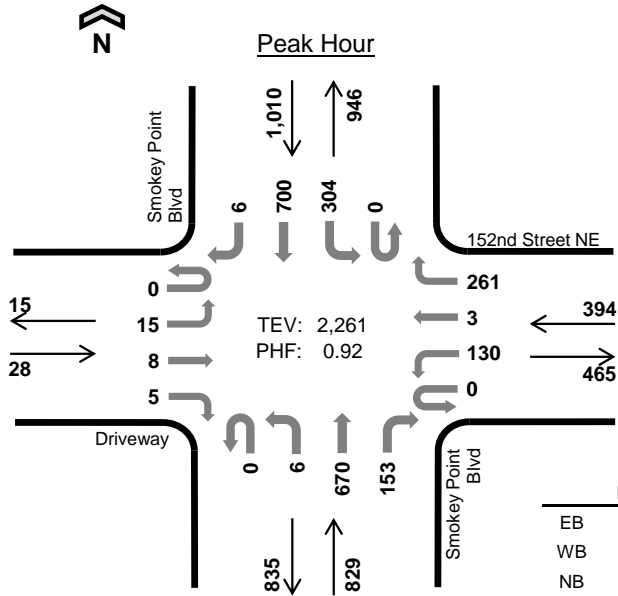
Two-Hour Count Summaries - Bikes																	
Interval Start	156th Street NE			156th Street NE			Smokey Point Blvd			Smokey Point Blvd			15-min Total	Rolling One Hour			
	Eastbound			Westbound			Northbound			Southbound							
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT					
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Smokey Point Blvd 152nd Street NE



Date: 05/31/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	3.6%	0.70
WB	3.6%	0.90
NB	3.6%	0.91
SB	2.6%	0.90
TOTAL	3.1%	0.92

Two-Hour Count Summaries

Interval Start	Driveway				152nd Street NE				Smokey Point Blvd				Smokey Point Blvd				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Northbound		Southbound		Northbound		Southbound								
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	2	1	1	0	31	3	62	0	4	184	34	0	58	141	3	524	0	
4:15 PM	0	3	5	2	0	29	1	79	0	0	167	35	0	69	157	1	548	0	
4:30 PM	0	5	1	2	0	32	0	71	0	2	183	44	0	83	190	1	614	0	
4:45 PM	0	4	0	0	0	39	0	51	0	1	162	44	0	66	162	1	530	2,216	
5:00 PM	0	3	2	1	0	30	2	60	0	3	158	30	0	86	191	3	569	2,261	
5:15 PM	0	0	1	4	0	32	1	51	0	3	144	37	1	78	177	5	534	2,247	
5:30 PM	0	10	0	3	0	23	0	49	0	2	131	42	0	84	152	2	498	2,131	
5:45 PM	0	5	1	3	0	18	2	44	0	0	123	30	0	66	135	0	427	2,028	
Count Total	0	32	11	16	0	234	9	467	0	15	1,252	296	1	590	1,305	16	4,244	0	
Peak Hour	All	0	15	8	5	0	130	3	261	0	6	670	153	0	304	700	6	2,261	0
	HV	0	1	0	0	0	12	0	2	0	0	20	10	0	8	18	0	71	0
	HV%	-	7%	0%	0%	-	9%	0%	1%	-	0%	3%	7%	-	3%	3%	0%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	3	9	7	19	0	0	0	0	0	0	1	0	0	1
4:15 PM	0	1	6	6	13	0	0	0	0	0	0	0	0	0	0
4:30 PM	1	4	5	8	18	0	0	0	0	0	1	0	0	0	1
4:45 PM	0	6	13	6	25	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	3	6	6	15	0	0	0	0	0	0	1	0	1	2
5:15 PM	0	2	2	6	10	0	0	0	0	0	1	0	1	0	2
5:30 PM	0	0	3	7	10	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	3	2	4	9	0	0	0	0	0	0	1	0	0	1
Count Total	1	22	46	50	119	0	0	0	0	0	2	3	1	1	7
Peak Hour	1	14	30	26	71	0	0	0	0	0	1	1	0	1	3

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Driveway				152nd Street NE				Smokey Point Blvd				Smokey Point Blvd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	0	0	2	0	1	0	1	1	7	0	2	5	0	19	0
4:15 PM	0	0	0	0	0	1	0	0	0	0	3	3	0	2	4	0	13	0
4:30 PM	0	1	0	0	0	3	0	1	0	0	4	1	0	1	7	0	18	0
4:45 PM	0	0	0	0	0	5	0	1	0	0	11	2	0	3	3	0	25	75
5:00 PM	0	0	0	0	0	3	0	0	0	0	2	4	0	2	4	0	15	71
5:15 PM	0	0	0	0	0	1	0	1	0	1	1	0	0	0	6	0	10	68
5:30 PM	0	0	0	0	0	0	0	0	0	0	1	2	0	2	5	0	10	60
5:45 PM	0	0	0	0	0	1	0	2	0	0	1	1	0	1	3	0	9	44
Count Total	0	1	0	0	0	16	0	6	0	2	24	20	0	13	37	0	119	0
Peak Hour	0	1	0	0	0	12	0	2	0	0	20	10	0	8	18	0	71	0

Two-Hour Count Summaries - Bikes																		
Interval Start	Driveway			152nd Street NE			Smokey Point Blvd			Smokey Point Blvd			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

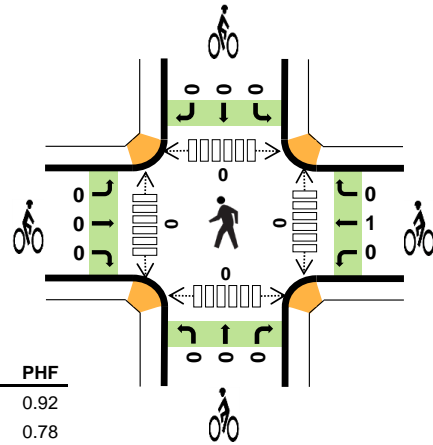
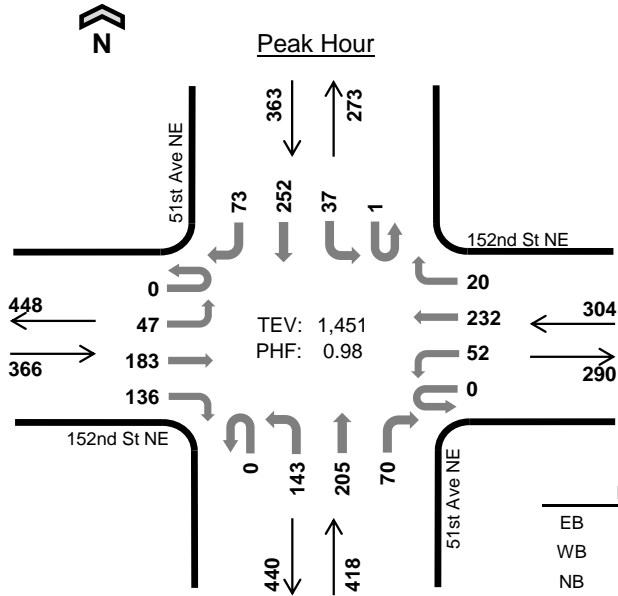


51st Ave NE 152nd St NE

Date: 06/01/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	3.3%	0.92
WB	3.6%	0.78
NB	1.4%	0.96
SB	2.2%	0.84
TOTAL	2.5%	0.98

Two-Hour Count Summaries

Interval Start	152nd St NE Eastbound				152nd St NE Westbound				51st Ave NE Northbound				51st Ave NE Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	7	56	35	0	16	37	6	0	27	45	16	0	8	42	8	303	0	
4:15 PM	0	11	43	35	0	13	49	5	0	28	51	20	1	10	74	23	363	0	
4:30 PM	0	15	51	31	0	7	69	6	0	41	53	15	0	7	56	20	371	0	
4:45 PM	0	9	40	31	0	17	76	5	0	37	47	19	0	11	63	17	372	1,409	
5:00 PM	0	12	49	39	0	15	38	4	0	37	54	16	0	9	59	13	345	1,451	
5:15 PM	0	7	48	32	0	12	46	1	0	26	36	27	0	5	58	9	307	1,395	
5:30 PM	0	7	45	27	0	13	60	3	0	41	46	23	0	5	44	12	326	1,350	
5:45 PM	0	11	43	50	0	12	62	9	0	50	48	15	0	8	39	15	362	1,340	
Count Total	0	79	375	280	0	105	437	39	0	287	380	151	1	63	435	117	2,749	0	
Peak Hour	All	0	47	183	136	0	52	232	20	0	143	205	70	1	37	252	73	1,451	0
	HV	0	5	2	5	0	1	10	0	0	3	3	0	0	2	3	3	37	0
	HV%	-	11%	1%	4%	-	2%	4%	0%	-	2%	1%	0%	0%	5%	1%	4%	3%	0

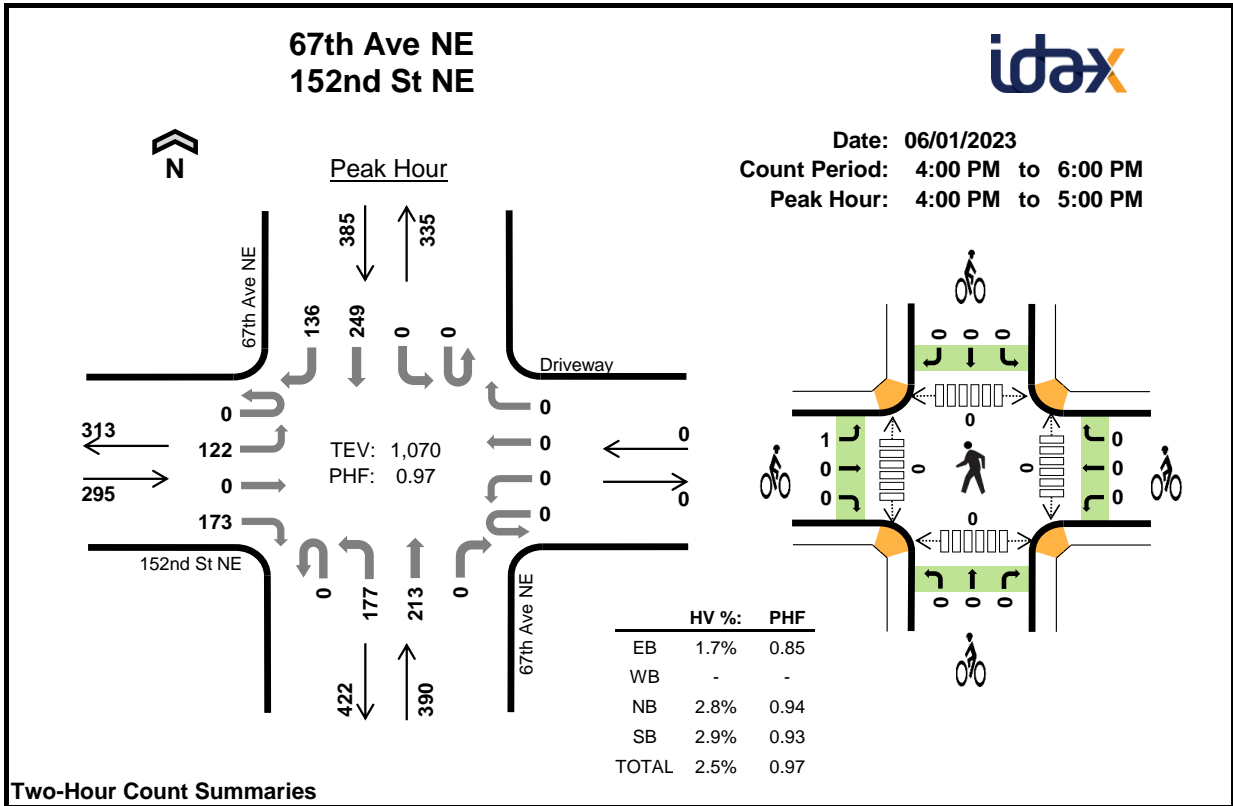
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	1	1	1	11	14	0	0	0	0	0	0	0	0	0	0
4:15 PM	7	4	1	3	15	0	0	0	0	0	0	0	0	0	0
4:30 PM	1	1	5	2	9	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	5	0	2	7	0	0	0	0	0	0	0	0	0	0
5:00 PM	4	1	0	1	6	0	1	0	0	1	0	0	0	0	0
5:15 PM	1	2	1	1	5	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	3	2	5	0	0	0	0	0	0	0	0	0	0
5:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0
Count Total	15	14	11	22	62	0	1	0	0	1	0	0	0	0	0
Peak Hour	12	11	6	8	37	0	1	0	0	1	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	152nd St NE				152nd St NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	1	0	0	1	0	0	0	1	0	0	2	7	2	14	0
4:15 PM	0	3	1	3	0	0	4	0	0	0	1	0	0	0	2	1	15	0
4:30 PM	0	1	0	0	0	0	1	0	0	3	2	0	0	1	0	1	9	0
4:45 PM	0	0	0	0	0	1	4	0	0	0	0	0	0	1	1	0	7	45
5:00 PM	0	1	1	2	0	0	1	0	0	0	0	0	0	0	0	1	6	37
5:15 PM	0	0	1	0	0	0	2	0	0	1	0	0	0	0	0	1	5	27
5:30 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	1	1	5	23
5:45 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	17
Count Total	0	5	3	7	0	1	13	0	0	6	5	0	0	4	11	7	62	0
Peak Hour	0	5	2	5	0	1	10	0	0	3	3	0	0	2	3	3	37	0

Two-Hour Count Summaries - Bikes																	
Interval Start	152nd St NE			152nd St NE			51st Ave NE			51st Ave NE			15-min Total	Rolling One Hour			
	Eastbound			Westbound			Northbound			Southbound							
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT					
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Count Total	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0
Peak Hour	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.



Two-Hour Count Summaries

Interval Start	152nd St NE				Driveway				67th Ave NE				67th Ave NE				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Westbound		Northbound		Northbound		Southbound		Southbound						
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	34	0	53	0	0	0	0	0	32	56	0	0	0	68	25	268	0	
4:15 PM	0	22	0	42	0	0	0	0	0	38	61	0	0	0	72	31	266	0	
4:30 PM	0	37	0	42	0	0	0	0	0	54	45	0	0	0	62	37	277	0	
4:45 PM	0	29	0	36	0	0	0	0	0	53	51	0	0	0	47	43	259	1,070	
5:00 PM	0	39	0	39	0	0	0	0	0	22	47	0	0	0	58	26	231	1,033	
5:15 PM	0	34	0	40	0	0	0	0	0	29	42	0	0	0	47	28	220	987	
5:30 PM	0	37	0	32	0	0	0	0	0	46	43	0	0	0	44	30	232	942	
5:45 PM	0	27	0	37	0	0	0	0	0	60	50	0	0	0	29	29	232	915	
Count Total	0	259	0	321	0	0	0	0	0	334	395	0	0	0	427	249	1,985	0	
Peak Hour	All	0	122	0	173	0	0	0	0	0	177	213	0	0	0	249	136	1,070	0
	HV	0	1	0	4	0	0	0	0	0	6	5	0	0	0	8	3	27	0
	HV%	-	1%	-	2%	-	-	-	-	-	3%	2%	-	-	-	3%	2%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	3	0	1	4	8	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	4	2	6	0	0	0	0	0	0	0	0	0	0
4:30 PM	2	0	3	1	6	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	3	4	7	1	0	0	0	1	0	0	0	0	0
5:00 PM	2	0	1	1	4	0	0	0	1	1	0	0	0	0	0
5:15 PM	1	0	2	1	4	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0
Count Total	8	0	16	13	37	1	0	0	1	2	0	0	0	0	0
Peak Hour	5	0	11	11	27	1	0	0	0	1	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																			
Interval Start	152nd St NE				Driveway				67th Ave NE				67th Ave NE				15-min Total	Rolling One Hour	
	Eastbound				Westbound				Northbound				Southbound						
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	1	0	2	0	0	0	0	0	1	0	0	0	0	0	4	0	8	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	1	1	6	0
4:30 PM	0	0	0	2	0	0	0	0	0	0	1	2	0	0	0	0	1	6	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	1	7	27
5:00 PM	0	0	0	2	0	0	0	0	0	0	0	1	0	0	0	0	1	4	23
5:15 PM	0	1	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1	4	21
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	16
5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	10
Count Total	0	2	0	6	0	0	0	0	0	0	8	8	0	0	0	8	5	37	0
Peak Hour	0	1	0	4	0	0	0	0	0	0	6	5	0	0	0	8	3	27	0

Two-Hour Count Summaries - Bikes																			
Interval Start	152nd St NE			Driveway			67th Ave NE			67th Ave NE			15-min Total	Rolling One Hour					
	Eastbound			Westbound			Northbound			Southbound									
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT							
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Count Total	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2	2	0
Peak Hour	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2	0

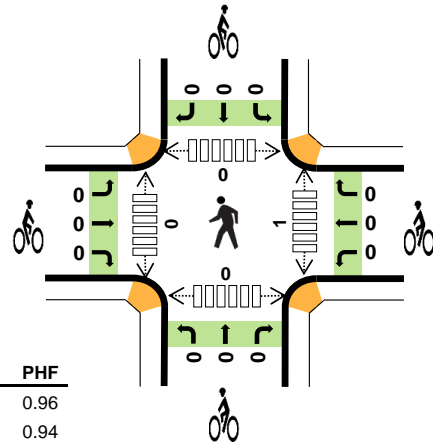
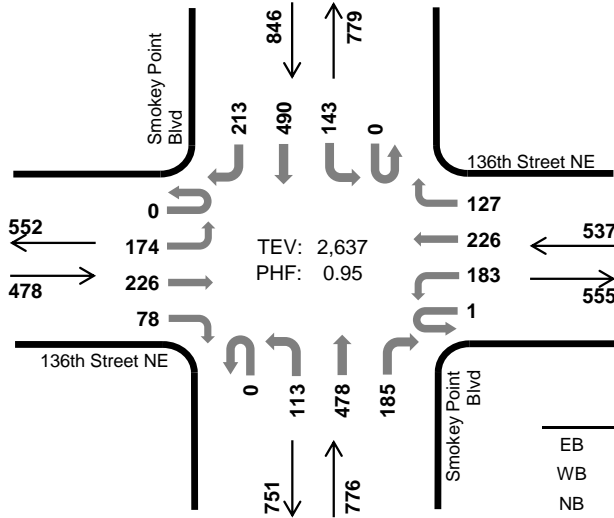
Note: U-Turn volumes for bikes are included in Left-Turn, if any.

Smokey Point Blvd 136th Street NE



Peak Hour

Date: 05/31/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	6.7%	0.96
WB	2.2%	0.94
NB	2.7%	0.96
SB	2.4%	0.89
TOTAL	3.2%	0.95

Two-Hour Count Summaries

Interval Start	136th Street NE				136th Street NE				Smokey Point Blvd				Smokey Point Blvd				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Northbound		Southbound		Northbound		Southbound								
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	24	55	13	0	47	70	34	0	28	128	49	0	20	106	50	624	0	
4:15 PM	0	46	59	17	0	40	55	29	0	21	119	40	0	31	110	42	609	0	
4:30 PM	0	46	57	21	0	50	57	35	0	30	133	36	0	46	126	54	691	0	
4:45 PM	0	45	51	18	1	44	46	37	0	31	112	59	0	26	114	60	644	2,568	
5:00 PM	0	37	59	22	0	49	68	26	0	31	114	50	0	40	140	57	693	2,637	
5:15 PM	0	34	54	15	0	29	46	17	0	30	120	57	0	28	129	50	609	2,637	
5:30 PM	0	50	44	27	0	25	16	10	0	20	122	44	0	35	99	49	541	2,487	
5:45 PM	0	28	51	14	0	35	25	21	0	31	83	40	0	29	86	49	492	2,335	
Count Total	0	310	430	147	1	319	383	209	0	222	931	375	0	255	910	411	4,903	0	
Peak Hour	All	0	174	226	78	1	183	226	127	0	113	478	185	0	143	490	213	2,637	0
	HV	0	7	24	1	0	3	9	0	0	1	18	2	0	1	12	7	85	0
	HV%	-	4%	11%	1%	0%	2%	4%	0%	-	1%	4%	1%	-	1%	2%	3%	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	5	3	4	5	17	0	0	0	0	0	0	0	0	0	0
4:15 PM	8	5	6	2	21	0	0	0	0	0	0	0	0	0	0
4:30 PM	8	3	6	8	25	0	0	0	0	0	1	0	0	0	1
4:45 PM	11	1	5	5	22	0	0	0	0	0	0	0	0	0	0
5:00 PM	5	3	4	5	17	0	0	0	0	0	0	0	0	0	0
5:15 PM	3	2	2	5	12	0	0	0	0	0	0	0	0	0	0
5:30 PM	2	1	2	7	12	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	4	2	5	11	0	0	0	0	0	0	0	0	0	0
Count Total	42	22	31	42	137	0	0	0	0	0	1	0	0	0	1
Peak Hour	32	12	21	20	85	0	0	0	0	0	1	0	0	0	1

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	136th Street NE				136th Street NE				Smokey Point Blvd				Smokey Point Blvd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	1	4	0	0	0	3	0	0	0	4	0	0	1	4	0	17	0
4:15 PM	0	0	8	0	0	1	4	0	0	1	5	0	0	0	1	1	21	0
4:30 PM	0	1	7	0	0	0	3	0	0	0	4	2	0	1	4	3	25	0
4:45 PM	0	5	5	1	0	0	1	0	0	0	5	0	0	0	3	2	22	85
5:00 PM	0	1	4	0	0	2	1	0	0	0	4	0	0	0	4	1	17	85
5:15 PM	0	1	2	0	0	1	0	1	0	0	1	1	0	1	2	2	12	76
5:30 PM	0	2	0	0	0	1	0	0	0	0	1	1	0	4	3	0	12	63
5:45 PM	0	0	0	0	0	1	0	3	0	0	2	0	0	0	1	4	11	52
Count Total	0	11	30	1	0	6	12	4	0	1	26	4	0	7	22	13	137	0
Peak Hour	0	7	24	1	0	3	9	0	0	1	18	2	0	1	12	7	85	0

Two-Hour Count Summaries - Bikes																		
Interval Start	136th Street NE			136th Street NE			Smokey Point Blvd			Smokey Point Blvd			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

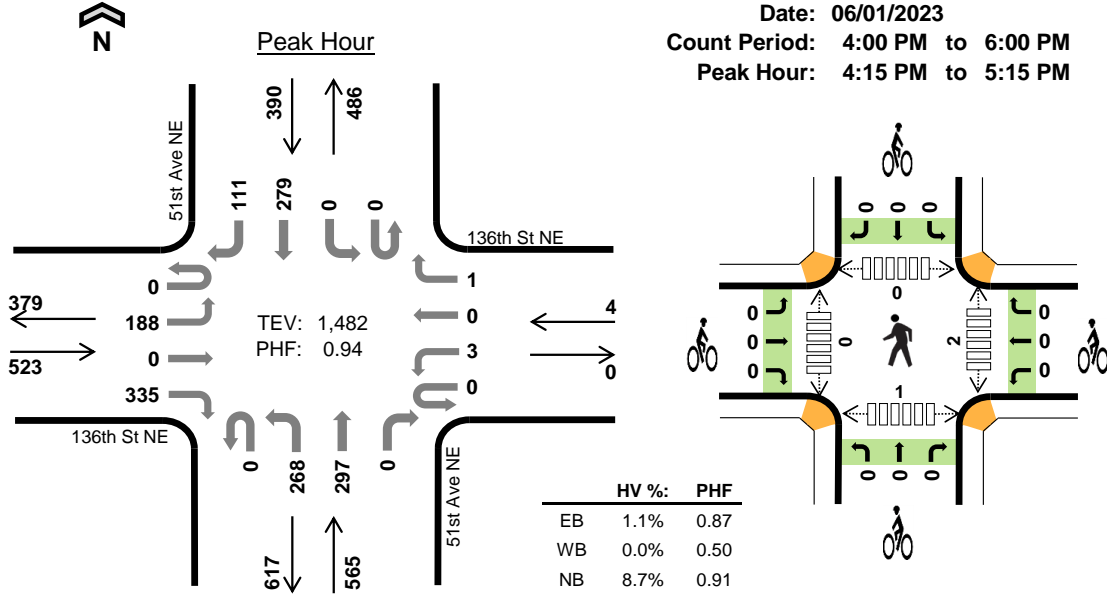


51st Ave NE 136th St NE

Date: 06/01/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:15 PM to 5:15 PM



Two-Hour Count Summaries

Interval Start	136th St NE Eastbound				136th St NE Westbound				51st Ave NE Northbound				51st Ave NE Southbound				15-min Total	Rolling One Hour	
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	40	0	61	0	3	3	2	0	60	62	0	0	0	50	38	319	0	
4:15 PM	0	41	0	76	0	1	0	1	0	69	68	0	0	0	66	31	353	0	
4:30 PM	0	64	0	87	0	0	0	0	0	75	80	0	0	0	66	22	394	0	
4:45 PM	0	44	0	92	0	0	0	0	0	66	68	0	0	0	77	29	376	1,442	
5:00 PM	0	39	0	80	0	2	0	0	0	58	81	0	0	0	70	29	359	1,482	
5:15 PM	0	46	0	78	0	0	0	1	0	50	65	0	0	0	61	24	325	1,454	
5:30 PM	0	48	0	51	0	2	0	0	0	47	83	0	0	0	47	30	308	1,368	
5:45 PM	0	35	0	59	0	0	0	0	0	40	86	0	0	0	69	24	313	1,305	
Count Total	0	357	0	584	0	8	3	4	0	465	593	0	0	0	506	227	2,747	0	
Peak Hour	All	0	188	0	335	0	3	0	1	0	268	297	0	0	0	279	111	1,482	0
	HV	0	1	0	5	0	0	0	0	0	43	6	0	0	0	6	3	64	0
	HV%	-	1%	-	1%	-	0%	-	0%	-	16%	2%	-	-	-	2%	3%	4%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	3	0	3	6	12	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	8	6	14	0	0	0	0	0	2	0	0	0	2
4:30 PM	2	0	22	0	24	0	0	0	0	0	0	0	0	1	1
4:45 PM	2	0	15	2	19	0	0	0	0	0	0	0	0	0	0
5:00 PM	2	0	4	1	7	0	0	0	0	0	0	0	0	0	0
5:15 PM	1	0	5	0	6	0	0	0	0	0	3	0	0	0	3
5:30 PM	1	0	6	1	8	0	0	0	0	0	2	0	0	1	3
5:45 PM	0	0	3	1	4	0	0	0	0	0	0	0	0	0	0
Count Total	11	0	66	17	94	0	0	0	0	0	7	0	0	2	9
Peak Hour	6	0	49	9	64	0	0	0	0	0	2	0	0	1	3

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	136th St NE				136th St NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	3	0	0	0	0	0	2	1	0	0	0	2	4	12	0
4:15 PM	0	0	0	0	0	0	0	0	0	7	1	0	0	0	4	2	14	0
4:30 PM	0	0	0	2	0	0	0	0	0	17	5	0	0	0	0	0	24	0
4:45 PM	0	1	0	1	0	0	0	0	0	15	0	0	0	0	1	1	19	69
5:00 PM	0	0	0	2	0	0	0	0	0	4	0	0	0	0	1	0	7	64
5:15 PM	0	0	0	1	0	0	0	0	0	3	2	0	0	0	0	0	6	56
5:30 PM	0	0	0	1	0	0	0	0	0	5	1	0	0	0	1	0	8	40
5:45 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	1	0	4	25
Count Total	0	1	0	10	0	0	0	0	0	55	11	0	0	0	10	7	94	0
Peak Hour	0	1	0	5	0	0	0	0	0	43	6	0	0	0	6	3	64	0

Two-Hour Count Summaries - Bikes																	
Interval Start	136th St NE			136th St NE			51st Ave NE			51st Ave NE			15-min Total	Rolling One Hour			
	Eastbound			Westbound			Northbound			Southbound							
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT					
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

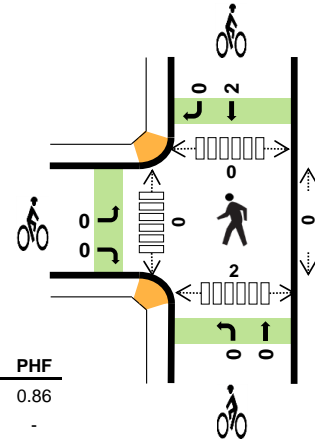
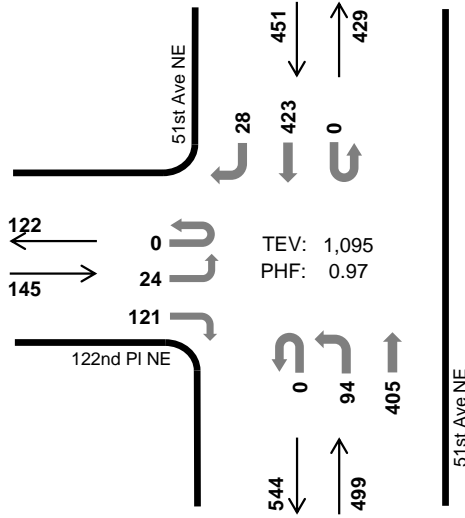
Note: U-Turn volumes for bikes are included in Left-Turn, if any.

51st Ave NE 122nd PI NE



Peak Hour

Date: 06/01/2023
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:00 PM to 5:00 PM



	HV %:	PHF
EB	2.8%	0.86
WB	-	-
NB	7.4%	0.86
SB	2.9%	0.85
TOTAL	4.9%	0.97

Two-Hour Count Summaries

Interval Start	122nd PI NE				0				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	5	0	29	0	0	0	0	0	20	104	0	0	0	98	4		
4:15 PM	0	6	0	36	0	0	0	0	0	25	92	0	0	0	113	3		
4:30 PM	0	5	0	32	0	0	0	0	0	25	120	0	0	0	93	7		
4:45 PM	0	8	0	24	0	0	0	0	0	24	89	0	0	0	119	14		
5:00 PM	0	8	0	17	0	0	0	0	0	12	98	0	0	0	104	8		
5:15 PM	0	7	0	31	0	0	0	0	0	17	81	0	0	0	86	7		
5:30 PM	0	9	0	27	0	0	0	0	0	20	85	0	0	0	77	2		
5:45 PM	0	9	0	24	0	0	0	0	0	24	94	0	0	0	88	7		
Count Total	0	57	0	220	0	0	0	0	0	167	763	0	0	0	778	52		
Peak Hour	All	0	24	0	121	0	0	0	0	0	94	405	0	0	0	423	28	
	HV	0	0	0	4	0	0	0	0	0	1	36	0	0	0	13	0	
	HV%	-	0%	-	3%	-	-	-	-	-	1%	9%	-	-	-	3%	0%	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	2	0	3	4	9	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	8	6	14	0	0	0	2	2	0	0	0	2	2
4:30 PM	1	0	13	0	14	0	0	0	0	0	0	0	0	0	0
4:45 PM	1	0	13	3	17	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	3	2	5	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	3	0	3	0	0	1	0	1	0	0	0	0	0
5:30 PM	1	0	2	1	4	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	1	1	2	0	0	0	0	0	0	0	0	1	1
Count Total	5	0	46	17	68	0	0	1	2	3	0	0	0	3	3
Peak Hr	4	0	37	13	54	0	0	0	2	2	0	0	0	2	2

Two-Hour Count Summaries - Heavy Vehicles														15-min Total	Rolling One Hour			
Interval Start	122nd PI NE				0				51st Ave NE				51st Ave NE					
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	2	0	0	0	0	0	0	3	0	0	0	4	0	9	0
4:15 PM	0	0	0	0	0	0	0	0	0	1	7	0	0	0	6	0	14	0
4:30 PM	0	0	0	1	0	0	0	0	0	0	13	0	0	0	0	0	14	0
4:45 PM	0	0	0	1	0	0	0	0	0	0	13	0	0	0	3	0	17	54
5:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	2	0	5	50
5:15 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	39
5:30 PM	0	0	0	1	0	0	0	0	0	0	2	0	0	0	1	0	4	29
5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	2	14
Count Total	0	0	0	5	0	0	0	0	0	3	43	0	0	0	17	0	68	0
Peak Hour	0	0	0	4	0	0	0	0	0	1	36	0	0	0	13	0	54	0

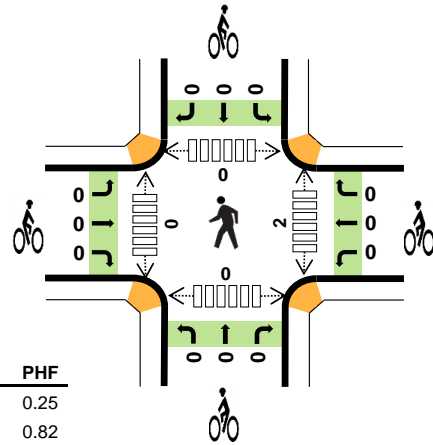
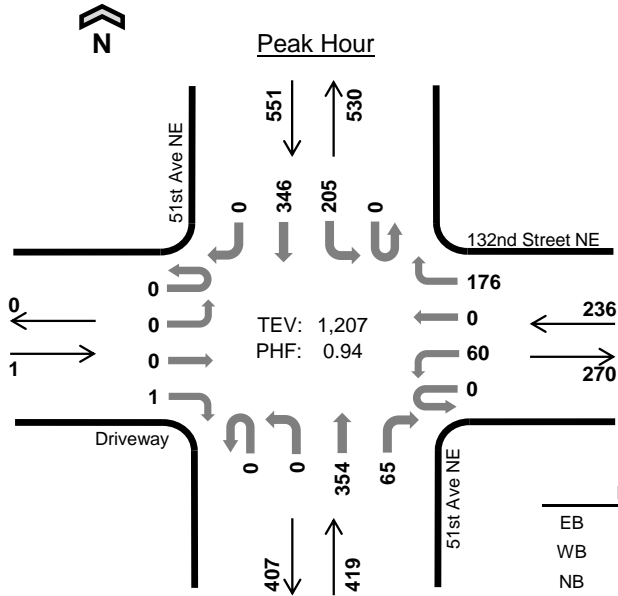
Two-Hour Count Summaries - Bikes														15-min Total	Rolling One Hour			
Interval Start	122nd PI NE			0			51st Ave NE			51st Ave NE								
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	2	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	1	1
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Count Total	0	0	0	0	0	0	0	1	0	0	2	0	0	2	0	3	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	2	0	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

51st Ave NE 132nd Street NE



Date: 05/31/2023
 Count Period: 4:00 PM to 6:00 PM
 Peak Hour: 4:15 PM to 5:15 PM



	HV %:	PHF
EB	0.0%	0.25
WB	5.9%	0.82
NB	9.3%	0.94
SB	1.6%	0.85
TOTAL	5.1%	0.94

Two-Hour Count Summaries

Interval Start	Driveway				132nd Street NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Northbound		Southbound		UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	0	0	0	0	18	0	50	0	0	87	14	0	63	80	1	313	0	
4:15 PM	0	0	0	1	0	21	0	51	0	0	79	14	0	51	75	0	292	0	
4:30 PM	0	0	0	0	0	13	0	43	0	0	91	19	0	51	77	0	294	0	
4:45 PM	0	0	0	0	0	17	0	37	0	0	98	14	0	46	89	0	301	1,200	
5:00 PM	0	0	0	0	0	9	0	45	0	0	86	18	0	57	105	0	320	1,207	
5:15 PM	0	0	0	0	0	12	0	47	0	1	91	19	0	35	85	0	290	1,205	
5:30 PM	0	0	0	0	0	8	0	29	0	0	79	17	0	59	72	0	264	1,175	
5:45 PM	0	0	0	0	0	12	0	33	0	0	64	19	0	42	81	0	251	1,125	
Count Total	0	0	0	1	0	110	0	335	0	1	675	134	0	404	664	1	2,325	0	
Peak Hour	All	0	0	0	1	0	60	0	176	0	0	354	65	0	205	346	0	1,207	0
	HV	0	0	0	0	0	1	0	13	0	0	37	2	0	4	5	0	62	0
	HV%	-	-	-	0%	-	2%	-	7%	-	-	10%	3%	-	2%	1%	-	5%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	6	4	2	12	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	5	11	3	19	0	0	0	0	0	1	0	0	0	1
4:30 PM	0	3	11	3	17	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	4	10	1	15	0	0	0	0	0	1	0	0	0	1
5:00 PM	0	2	7	2	11	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	2	4	3	9	0	0	0	0	0	2	2	0	0	4
5:30 PM	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	2	2	4	8	0	0	0	0	0	0	2	0	0	2
Count Total	0	26	49	18	93	0	0	0	0	0	4	4	0	0	8
Peak Hour	0	14	39	9	62	0	0	0	0	0	2	0	0	0	2

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Driveway				132nd Street NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	0	0	1	0	5	0	0	4	0	0	1	1	0	12	0
4:15 PM	0	0	0	0	0	0	0	5	0	0	11	0	0	0	3	0	19	0
4:30 PM	0	0	0	0	0	0	0	3	0	0	11	0	0	2	1	0	17	0
4:45 PM	0	0	0	0	0	0	0	4	0	0	9	1	0	1	0	0	15	63
5:00 PM	0	0	0	0	0	1	0	1	0	0	6	1	0	1	1	0	11	62
5:15 PM	0	0	0	0	0	0	0	2	0	0	4	0	0	1	2	0	9	52
5:30 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	2	37
5:45 PM	0	0	0	0	0	1	0	1	0	0	1	1	0	1	3	0	8	30
Count Total	0	0	0	0	0	3	0	23	0	0	46	3	0	7	11	0	93	0
Peak Hour	0	0	0	0	0	1	0	13	0	0	37	2	0	4	5	0	62	0

Two-Hour Count Summaries - Bikes																	
Interval Start	Driveway			132nd Street NE			51st Ave NE			51st Ave NE			15-min Total	Rolling One Hour			
	Eastbound			Westbound			Northbound			Southbound							
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT					
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

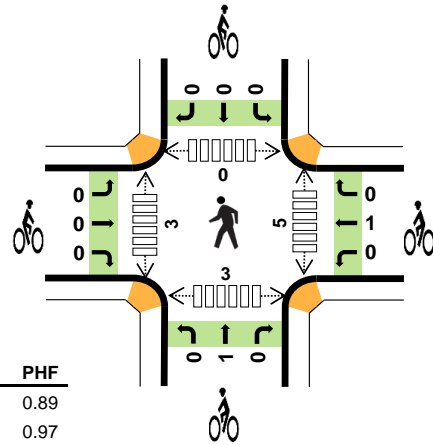
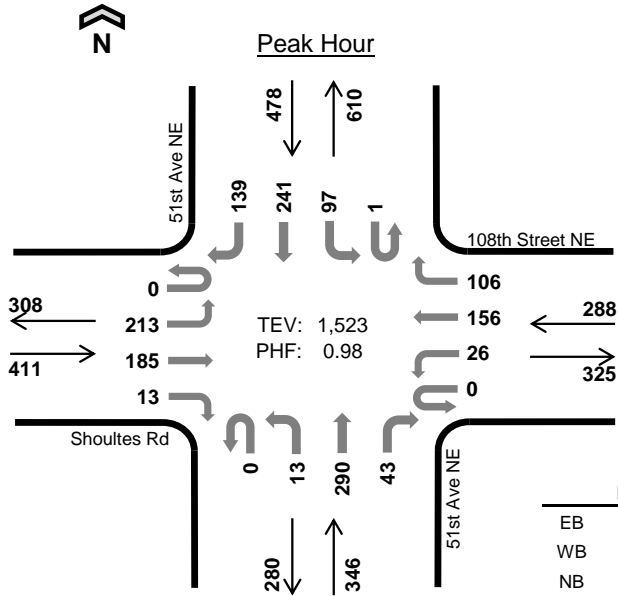


51st Ave NE 108th Street NE

Date: 05/31/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:00 PM to 5:00 PM



	HV %:	PHF
EB	2.4%	0.89
WB	7.6%	0.97
NB	4.9%	0.84
SB	1.7%	0.95
TOTAL	3.7%	0.98

Two-Hour Count Summaries

Interval Start	Shoultes Rd				108th Street NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour	
	Eastbound				Westbound				Northbound				Southbound						
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	52	37	5	0	7	43	19	0	6	81	16	1	26	55	37	385	0	
4:15 PM	0	52	38	2	0	8	44	22	0	2	72	6	0	24	60	33	363	0	
4:30 PM	0	52	54	3	0	3	33	38	0	1	70	6	0	26	60	40	386	0	
4:45 PM	0	57	56	3	0	8	36	27	0	4	67	15	0	21	66	29	389	1,523	
5:00 PM	0	56	50	3	0	10	33	19	0	3	71	18	1	28	62	30	384	1,522	
5:15 PM	0	70	36	2	0	5	28	24	0	1	52	17	0	25	60	29	349	1,508	
5:30 PM	0	55	44	2	0	4	38	18	0	2	42	12	1	30	57	36	341	1,463	
5:45 PM	0	65	34	3	0	6	48	22	0	4	34	13	1	20	49	36	335	1,409	
Count Total	0	459	349	23	0	51	303	189	0	23	489	103	4	200	469	270	2,932	0	
Peak Hour	All	0	213	185	13	0	26	156	106	0	13	290	43	1	97	241	139	1,523	0
	HV	0	5	3	2	0	1	2	19	0	0	17	0	1	2	3	3	57	0
	HV%	-	2%	2%	15%	-	4%	1%	18%	-	0%	6%	0%	100%	2%	1%	2%	4%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	1	2	7	3	13	0	1	0	0	1	1	0	0	0	1
4:15 PM	4	6	3	4	17	0	0	0	0	0	3	1	0	0	4
4:30 PM	2	10	2	1	15	0	0	1	0	1	1	1	0	3	5
4:45 PM	3	4	5	0	12	0	0	0	0	0	0	1	0	0	1
5:00 PM	0	6	7	2	15	0	0	0	0	0	0	1	0	0	1
5:15 PM	1	4	1	2	8	0	0	0	0	0	0	1	1	0	2
5:30 PM	0	0	1	0	1	0	0	0	0	0	0	1	0	0	1
5:45 PM	1	0	0	2	3	0	0	0	0	0	0	1	1	1	3
Count Total	12	32	26	14	84	0	1	1	0	2	5	7	2	4	18
Peak Hour	10	22	17	8	57	0	1	1	0	2	5	3	0	3	11

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Shoultés Rd				108th Street NE				51st Ave NE				51st Ave NE				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	1	0	0	0	0	1	1	0	0	7	0	1	1	1	0	13	0
4:15 PM	0	2	2	0	0	0	0	6	0	0	3	0	0	1	1	2	17	0
4:30 PM	0	1	1	0	0	1	0	9	0	0	2	0	0	0	0	1	15	0
4:45 PM	0	1	0	2	0	0	1	3	0	0	5	0	0	0	0	0	12	57
5:00 PM	0	0	0	0	0	0	2	4	0	0	7	0	0	1	0	1	15	59
5:15 PM	0	1	0	0	0	0	1	3	0	0	1	0	0	0	1	1	8	50
5:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	36
5:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	1	3	27
Count Total	0	7	3	2	0	1	5	26	0	0	26	0	1	3	4	6	84	0
Peak Hour	0	5	3	2	0	1	2	19	0	0	17	0	1	2	2	3	57	0
Two-Hour Count Summaries - Bikes																		
Interval Start	Shoultés Rd			108th Street NE			51st Ave NE			51st Ave NE			15-min Total	Rolling One Hour				
	Eastbound			Westbound			Northbound			Southbound								
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT						
4:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Count Total	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2	0	
Peak Hour	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2	0	
<i>Note: U-Turn volumes for bikes are included in Left-Turn, if any.</i>																		

Highway Capacity Manual, 2000

Signalized intersection level of service (LOS) is defined in terms of the average total vehicle delay of all movements through an intersection. Vehicle delay is a method of quantifying several intangible factors, including driver discomfort, frustration, and lost travel time. Specifically, LOS criteria are stated in terms of average delay per vehicle during a specified time period (for example, the PM peak hour). Vehicle delay is a complex measure based on many variables, including signal phasing (i.e., progression of movements through the intersection), signal cycle length, and traffic volumes with respect to intersection capacity. Table 1 shows LOS criteria for signalized intersections, as described in the *Highway Capacity Manual* (Transportation Research Board, Special Report 209, 2000).

Table 1. Level of Service Criteria for Signalized Intersections

Level of Service	Average Control Delay (sec/veh)	General Description (Signalized Intersections)
A	≤10	Free Flow
B	>10 - 20	Stable Flow (slight delays)
C	>20 - 35	Stable flow (acceptable delays)
D	>35 - 55	Approaching unstable flow (tolerable delay, occasionally wait through more than one signal cycle before proceeding)
E	>55 - 80	Unstable flow (intolerable delay)
F	>80	Forced flow (jammed)

Source: *Highway Capacity Manual*, Transportation Research Board, Special Report 209, 2000.

Unsignalized intersection LOS criteria can be further reduced into two intersection types: all-way stop-controlled and two-way stop-controlled. All-way, stop-controlled intersection LOS is expressed in terms of the average vehicle delay of all of the movements, much like that of a signalized intersection. Two-way, stop-controlled intersection LOS is defined in terms of the average vehicle delay of an individual movement(s). This is because the performance of a two-way, stop-controlled intersection is more closely reflected in terms of its individual movements, rather than its performance overall. For this reason, LOS for a two-way, stop-controlled intersection is defined in terms of its individual movements. With this in mind, total average vehicle delay (i.e., average delay of all movements) for a two-way, stop-controlled intersection should be viewed with discretion. Table 2 shows LOS criteria for unsignalized intersections (both all-way and two-way, stop-controlled).

Table 2. Level of Service Criteria for Unsignalized Intersections

Level of Service	Average Control Delay (sec/veh)
A	0 - 10
B	>10 - 15
C	>15 - 25
D	>25 - 35
E	>35 - 50
F	>50

Source: *Highway Capacity Manual*, Transportation Research Board, Special Report 209, 2000.

Highway Capacity Manual 2010/6th Edition

Signalized intersection level of service (LOS) is defined in terms of a weighted average control delay for the entire intersection. Control delay quantifies the increase in travel time that a vehicle experiences due to the traffic signal control as well as provides a surrogate measure for driver discomfort and fuel consumption. Signalized intersection LOS is stated in terms of average control delay per vehicle (in seconds) during a specified time period (e.g., weekday PM peak hour). Control delay is a complex measure based on many variables, including signal phasing and coordination (i.e., progression of movements through the intersection and along the corridor), signal cycle length, and traffic volumes with respect to intersection capacity and resulting queues. Table 1 summarizes the LOS criteria for signalized intersections, as described in the *Highway Capacity Manual 2010* and 6th Edition (Transportation Research Board, 2010 and 2016, respectively).

Table 1. Level of Service Criteria for Signalized Intersections

Level of Service	Average Control Delay (seconds/vehicle)	General Description
A	≤10	Free Flow
B	>10 – 20	Stable Flow (slight delays)
C	>20 – 35	Stable flow (acceptable delays)
D	>35 – 55	Approaching unstable flow (tolerable delay, occasionally wait through more than one signal cycle before proceeding)
E	>55 – 80	Unstable flow (intolerable delay)
F ¹	>80	Forced flow (congested and queues fail to clear)

Source: *Highway Capacity Manual 2010 and 6th Edition*, Transportation Research Board, 2010 and 2016, respectively.

1. If the volume-to-capacity (v/c) ratio for a lane group exceeds 1.0 LOS F is assigned to the individual lane group. LOS for overall approach or intersection is determined solely by the control delay.

Unsignalized intersection LOS criteria can be further reduced into two intersection types: all-way stop and two-way stop control. All-way stop control intersection LOS is expressed in terms of the weighted average control delay of the overall intersection or by approach. Two-way stop-controlled intersection LOS is defined in terms of the average control delay for each minor-street movement (or shared movement) as well as major-street left-turns. This approach is because major-street through vehicles are assumed to experience zero delay, a weighted average of all movements results in very low overall average delay, and this calculated low delay could mask deficiencies of minor movements. Table 2 shows LOS criteria for unsignalized intersections.

Table 2. Level of Service Criteria for Unsignalized Intersections

Level of Service	Average Control Delay (seconds/vehicle)
A	0 – 10
B	>10 – 15
C	>15 – 25
D	>25 – 35
E	>35 – 50
F ¹	>50

Source: *Highway Capacity Manual 2010 and 6th Edition*, Transportation Research Board, 2010 and 2016, respectively.

1. If the volume-to-capacity (v/c) ratio exceeds 1.0, LOS F is assigned an individual lane group for all unsignalized intersections, or minor street approach at two-way stop-controlled intersections. Overall intersection LOS is determined solely by control delay.

HCM 6th Signalized Intersection Summary
3: Smokey Point Blvd & 156th St NE

Williams Industrial
Existing PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	20	5	215	30	10	30	240	685	25	40	765	70
Future Volume (veh/h)	20	5	215	30	10	30	240	685	25	40	765	70
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1856	1856	1856	1841	1841	1841
Adj Flow Rate, veh/h	21	5	226	32	11	32	253	721	26	42	805	74
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	4	4	4	3	3	3	4	4	4
Cap, veh/h	466	357	494	346	51	148	434	1480	53	421	1177	108
Arrive On Green	0.12	0.19	0.19	0.05	0.12	0.12	0.12	0.43	0.43	0.06	0.36	0.36
Sat Flow, veh/h	1767	1856	1570	1753	414	1206	1767	3470	125	1753	3237	298
Grp Volume(v), veh/h	21	5	226	32	0	43	253	366	381	42	435	444
Grp Sat Flow(s),veh/h/ln	1767	1856	1570	1753	0	1620	1767	1763	1833	1753	1749	1786
Q Serve(g_s), s	0.5	0.1	6.7	0.9	0.0	1.4	4.7	8.8	8.8	0.8	12.3	12.3
Cycle Q Clear(g_c), s	0.5	0.1	6.7	0.9	0.0	1.4	4.7	8.8	8.8	0.8	12.3	12.3
Prop In Lane	1.00		1.00	1.00		0.74	1.00		0.07	1.00		0.17
Lane Grp Cap(c), veh/h	466	357	494	346	0	198	434	752	781	421	636	649
V/C Ratio(X)	0.05	0.01	0.46	0.09	0.00	0.22	0.58	0.49	0.49	0.10	0.68	0.68
Avail Cap(c_a), veh/h	890	825	890	891	0	582	702	1386	1441	796	1375	1404
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.7	19.1	16.1	20.5	0.0	23.1	10.9	12.1	12.1	10.2	15.8	15.8
Incr Delay (d2), s/veh	0.0	0.0	0.7	0.1	0.0	0.5	1.2	0.5	0.5	0.1	1.3	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.1	2.3	0.4	0.0	0.5	1.7	3.2	3.3	0.3	4.7	4.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	15.7	19.1	16.7	20.6	0.0	23.7	12.1	12.6	12.6	10.3	17.1	17.1
LnGrp LOS	B	B	B	C	A	C	B	B	B	B	B	B
Approach Vol, veh/h		252			75			1000			921	
Approach Delay, s/veh		16.7			22.4			12.5			16.8	
Approach LOS		B			C			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.5	28.9	6.8	15.3	11.1	25.3	10.9	11.2				
Change Period (Y+Rc), s	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0				
Max Green Setting (Gmax), s	16.0	46.0	21.0	26.0	16.0	46.0	21.0	21.0				
Max Q Clear Time (g_c+I1), s	2.8	10.8	2.9	8.7	6.7	14.3	2.5	3.4				
Green Ext Time (p_c), s	0.0	5.7	0.0	0.7	0.5	6.9	0.0	0.1				
Intersection Summary												
HCM 6th Ctrl Delay			15.0									
HCM 6th LOS			B									

HCM 6th Signalized Intersection Summary
 4: Smokey Point Blvd & Private Drive/152nd St NE

Williams Industrial
 Existing PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	15	10	5	130	5	260	5	670	155	305	700	5
Future Volume (veh/h)	15	10	5	130	5	260	5	670	155	305	700	5
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1856	1856	1856
Adj Flow Rate, veh/h	16	11	5	141	5	283	5	728	168	332	761	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	3	3	3
Cap, veh/h	227	196	89	499	6	361	9	922	213	130	1418	9
Arrive On Green	0.02	0.16	0.16	0.09	0.24	0.24	0.01	0.33	0.33	0.07	0.40	0.40
Sat Flow, veh/h	1753	1197	544	1753	27	1535	1753	2820	651	1767	3590	24
Grp Volume(v), veh/h	16	0	16	141	0	288	5	451	445	332	374	392
Grp Sat Flow(s),veh/h/ln	1753	0	1742	1753	0	1562	1753	1749	1722	1767	1763	1851
Q Serve(g_s), s	0.4	0.0	0.4	3.4	0.0	9.4	0.2	12.8	12.8	4.0	8.9	8.9
Cycle Q Clear(g_c), s	0.4	0.0	0.4	3.4	0.0	9.4	0.2	12.8	12.8	4.0	8.9	8.9
Prop In Lane	1.00		0.31	1.00		0.98	1.00		0.38	1.00		0.01
Lane Grp Cap(c), veh/h	227	0	285	499	0	367	9	572	563	130	696	731
V/C Ratio(X)	0.07	0.00	0.06	0.28	0.00	0.78	0.53	0.79	0.79	2.56	0.54	0.54
Avail Cap(c_a), veh/h	360	0	671	507	0	602	129	674	664	130	696	731
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	18.8	0.0	19.2	15.2	0.0	19.5	27.0	16.6	16.6	25.2	12.7	12.7
Incr Delay (d2), s/veh	0.1	0.0	0.1	0.3	0.0	3.7	39.8	5.4	5.5	723.8	0.8	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.2	1.3	0.0	3.5	0.2	5.4	5.4	27.7	3.2	3.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	18.9	0.0	19.3	15.5	0.0	23.2	66.8	22.0	22.1	749.1	13.5	13.4
LnGrp LOS	B	A	B	B	A	C	E	C	C	F	B	B
Approach Vol, veh/h		32			429			901			1098	
Approach Delay, s/veh		19.1			20.7			22.3			235.9	
Approach LOS		B			C			C			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	22.8	8.8	13.9	5.3	26.5	4.9	17.8				
Change Period (Y+Rc), s	5.0	5.0	4.0	5.0	5.0	5.0	4.0	5.0				
Max Green Setting (Gmax), s	4.0	21.0	5.0	21.0	4.0	21.0	5.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	14.8	5.4	2.4	2.2	10.9	2.4	11.4				
Green Ext Time (p_c), s	0.0	3.1	0.0	0.0	0.0	3.6	0.0	1.3				
Intersection Summary												
HCM 6th Ctrl Delay	117.3											
HCM 6th LOS	F											

Intersection	
Intersection Delay, s/veh	58.3
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	45	185	135	50	230	20	145	205	70	40	250	75
Future Vol, veh/h	45	185	135	50	230	20	145	205	70	40	250	75
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles, %	3	3	3	4	4	4	1	1	1	2	2	2
Mvmt Flow	46	189	138	51	235	20	148	209	71	41	255	77
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	52	38.2	82.7	52.9
HCM LOS	F	E	F	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	35%	12%	17%	11%
Vol Thru, %	49%	51%	77%	68%
Vol Right, %	17%	37%	7%	21%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	420	365	300	365
LT Vol	145	45	50	40
Through Vol	205	185	230	250
RT Vol	70	135	20	75
Lane Flow Rate	429	372	306	372
Geometry Grp	1	1	1	1
Degree of Util (X)	1.032	0.892	0.775	0.896
Departure Headway (Hd)	8.666	8.871	9.387	8.912
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	420	410	389	410
Service Time	6.666	6.871	7.387	6.912
HCM Lane V/C Ratio	1.021	0.907	0.787	0.907
HCM Control Delay	82.7	52	38.2	52.9
HCM Lane LOS	F	F	E	F
HCM 95th-tile Q	13.5	9.2	6.5	9.3

Intersection	
Intersection Delay, s/veh	15.7
Intersection LOS	C























Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	120	175	175	215	250	135
Future Vol, veh/h	120	175	175	215	250	135
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	124	180	180	222	258	139
Number of Lanes	1	0	0	1	1	0

Approach	EB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	1	1
Conflicting Approach Left	SB	EB	
Conflicting Lanes Left	1	1	0
Conflicting Approach Right	NB		EB
Conflicting Lanes Right	1	0	1
HCM Control Delay	14	17.3	15.5
HCM LOS	B	C	C

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	45%	41%	0%
Vol Thru, %	55%	0%	65%
Vol Right, %	0%	59%	35%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	390	295	385
LT Vol	175	120	0
Through Vol	215	0	250
RT Vol	0	175	135
Lane Flow Rate	402	304	397
Geometry Grp	1	1	1
Degree of Util (X)	0.619	0.481	0.582
Departure Headway (Hd)	5.546	5.699	5.279
Convergence, Y/N	Yes	Yes	Yes
Cap	651	630	683
Service Time	3.596	3.754	3.329
HCM Lane V/C Ratio	0.618	0.483	0.581
HCM Control Delay	17.3	14	15.5
HCM Lane LOS	C	B	C
HCM 95th-tile Q	4.3	2.6	3.8

HCM 6th Signalized Intersection Summary
7: Smokey Point Blvd & 136th St NE

Williams Industrial
Existing PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	175	225	80	185	225	125	115	480	185	145	490	215
Future Volume (veh/h)	175	225	80	185	225	125	115	480	185	145	490	215
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1796	1796	1796	1870	1870	1870	1856	1856	1856	1870	1870	1870
Adj Flow Rate, veh/h	184	237	84	195	237	132	121	505	195	153	516	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	7	7	7	2	2	2	3	3	3	2	2	2
Cap, veh/h	282	454	385	381	285	159	115	721	277	116	1030	
Arrive On Green	0.07	0.25	0.25	0.07	0.25	0.25	0.07	0.29	0.29	0.07	0.29	0.00
Sat Flow, veh/h	1711	1796	1522	1781	1129	629	1767	2488	956	1781	3647	0
Grp Volume(v), veh/h	184	237	84	195	0	369	121	357	343	153	516	0
Grp Sat Flow(s),veh/h/ln	1711	1796	1522	1781	0	1757	1767	1763	1682	1781	1777	0
Q Serve(g_s), s	4.0	7.0	2.7	4.0	0.0	12.2	4.0	11.0	11.1	4.0	7.4	0.0
Cycle Q Clear(g_c), s	4.0	7.0	2.7	4.0	0.0	12.2	4.0	11.0	11.1	4.0	7.4	0.0
Prop In Lane	1.00		1.00	1.00		0.36	1.00		0.57	1.00		0.00
Lane Grp Cap(c), veh/h	282	454	385	381	0	444	115	511	487	116	1030	
V/C Ratio(X)	0.65	0.52	0.22	0.51	0.00	0.83	1.05	0.70	0.70	1.31	0.50	
Avail Cap(c_a), veh/h	282	792	671	381	0	603	115	864	824	116	1219	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	19.5	19.7	18.1	17.8	0.0	21.6	28.6	19.4	19.4	28.6	18.1	0.0
Incr Delay (d2), s/veh	5.3	0.9	0.3	1.1	0.0	7.1	97.2	1.7	1.9	189.7	0.4	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.2	2.9	0.9	2.1	0.0	5.6	4.8	4.5	4.3	7.8	2.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	24.8	20.6	18.4	19.0	0.0	28.8	125.8	21.1	21.3	218.3	18.4	0.0
LnGrp LOS	C	C	B	B	A	C	F	C	C	F	B	
Approach Vol, veh/h		505			564			821			669	
Approach Delay, s/veh		21.8			25.4			36.6			64.1	
Approach LOS		C			C			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	22.7	9.0	20.5	9.0	22.7	9.0	20.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	30.0	4.0	27.0	4.0	21.0	4.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	13.1	6.0	9.0	6.0	9.4	6.0	14.2				
Green Ext Time (p_c), s	0.0	4.4	0.0	1.6	0.0	2.8	0.0	1.3				

Intersection Summary





















HCM 6th Ctrl Delay	38.4
HCM 6th LOS	D

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

HCM Signalized Intersection Capacity Analysis
 8: 51st Ave NE & 136th St NE

Williams Industrial
 Existing PM Peak Hour

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	190	0	335	5	0	5	270	295	0	0	280	110	
Future Volume (vph)	190	0	335	5	0	5	270	295	0	0	280	110	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00			1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00			1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00	0.95	1.00			1.00	1.00	
Satd. Flow (prot)		1787	1583		1805	1615	1656	1743			1863	1583	
Flt Permitted		0.95	1.00		0.95	1.00	0.43	1.00			1.00	1.00	
Satd. Flow (perm)		1787	1583		1805	1615	742	1743			1863	1583	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	202	0	356	5	0	5	287	314	0	0	298	117	
RTOR Reduction (vph)	0	0	212	0	0	5	0	0	0	0	0	52	
Lane Group Flow (vph)	0	202	144	0	5	0	287	314	0	0	298	65	
Confl. Peds. (#/hr)			1	1					2	2			
Heavy Vehicles (%)	1%	1%	1%	0%	0%	0%	9%	9%	9%	2%	2%	2%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	D.P+P	NA			NA	pm+ov	
Protected Phases	4	4	5	8	8		5	2			6	4	
Permitted Phases			4			8	6					6	
Actuated Green, G (s)		11.6	24.8		1.0	1.0	28.7	33.7			15.5	27.1	
Effective Green, g (s)		11.6	24.8		1.0	1.0	28.7	33.7			15.5	27.1	
Actuated g/C Ratio		0.19	0.40		0.02	0.02	0.47	0.55			0.25	0.44	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Vehicle Extension (s)		2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0	
Lane Grp Cap (vph)		338	640		29	26	544	958			471	828	
v/s Ratio Prot		c0.11	0.05		c0.00		c0.11	0.18			c0.16	0.01	
v/s Ratio Perm			0.04			0.00	0.13					0.03	
v/c Ratio		0.60	0.23		0.17	0.00	0.53	0.33			0.63	0.08	
Uniform Delay, d1		22.7	12.0		29.7	29.7	10.7	7.6			20.4	9.9	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Incremental Delay, d2		1.9	0.1		1.0	0.0	0.4	0.1			2.0	0.0	
Delay (s)		24.6	12.0		30.8	29.7	11.1	7.7			22.4	9.9	
Level of Service		C	B		C	C	B	A			C	A	
Approach Delay (s)		16.6			30.2			9.3			18.9		
Approach LOS		B			C			A			B		
Intersection Summary													
HCM 2000 Control Delay			14.5		HCM 2000 Level of Service							B	
HCM 2000 Volume to Capacity ratio			0.58										
Actuated Cycle Length (s)			61.3		Sum of lost time (s)						20.0		
Intersection Capacity Utilization			59.4%		ICU Level of Service						B		
Analysis Period (min)			15										
c Critical Lane Group													

Intersection	
Intersection Delay, s/veh	27.8
Intersection LOS	D

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	5	60	0	175	0	355	65	205	345	0
Future Vol, veh/h	0	0	5	60	0	175	0	355	65	205	345	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	0	0	6	6	6	9	9	9	2	2	2
Mvmt Flow	0	0	5	64	0	186	0	378	69	218	367	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	10	14.1	21.3	38.8
HCM LOS	A	B	C	E

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	0%	0%	26%	37%
Vol Thru, %	85%	0%	0%	63%
Vol Right, %	15%	100%	74%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	420	5	235	550
LT Vol	0	0	60	205
Through Vol	355	0	0	345
RT Vol	65	5	175	0
Lane Flow Rate	447	5	250	585
Geometry Grp	1	1	1	1
Degree of Util (X)	0.705	0.01	0.434	0.9
Departure Headway (Hd)	5.684	6.917	6.246	5.54
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	630	521	573	649
Service Time	3.756	4.917	4.328	3.604
HCM Lane V/C Ratio	0.71	0.01	0.436	0.901
HCM Control Delay	21.3	10	14.1	38.8
HCM Lane LOS	C	A	B	E
HCM 95th-tile Q	5.7	0	2.2	11.2

HCM 6th Signalized Intersection Summary
 10: 51st Ave NE & 122nd PI NE

Williams Industrial
 Existing PM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	25	120	95	405	425	30
Future Volume (veh/h)	25	120	95	405	425	30
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	0.99	1.00			0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1378	1378	1913	1913	2012	2012
Adj Flow Rate, veh/h	26	124	98	418	438	31
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	3	3	7	7	3	3
Cap, veh/h	31	148	522	1102	650	46
Arrive On Green	0.15	0.15	0.09	0.58	0.35	0.35
Sat Flow, veh/h	204	973	1822	1913	1854	131
Grp Volume(v), veh/h	151	0	98	418	0	469
Grp Sat Flow(s),veh/h/ln	1185	0	1822	1913	0	1985
Q Serve(g_s), s	4.1	0.0	1.1	3.9	0.0	6.7
Cycle Q Clear(g_c), s	4.1	0.0	1.1	3.9	0.0	6.7
Prop In Lane	0.17	0.82	1.00			0.07
Lane Grp Cap(c), veh/h	180	0	522	1102	0	696
V/C Ratio(X)	0.84	0.00	0.19	0.38	0.00	0.67
Avail Cap(c_a), veh/h	913	0	937	1473	0	1528
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	13.6	0.0	6.2	3.8	0.0	9.1
Incr Delay (d2), s/veh	9.8	0.0	0.2	0.2	0.0	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	0.0	0.3	0.7	0.0	2.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	23.5	0.0	6.4	4.0	0.0	10.3
LnGrp LOS	C	A	A	A	A	B
Approach Vol, veh/h	151			516	469	
Approach Delay, s/veh	23.5			4.5	10.3	
Approach LOS	C			A	B	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		23.6		9.5	7.5	16.1
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		25.5		25.5	10.5	25.5
Max Q Clear Time (g_c+I1), s		5.9		6.1	3.1	8.7
Green Ext Time (p_c), s		2.7		0.4	0.1	2.9

Intersection Summary

HCM 6th Ctrl Delay	9.4
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.

MOVEMENT SUMMARY

Site: 11 [51st Avenue NE/108th Street NE - Existing (Site Folder: General)]

Existing PM Peak Hour
 Site Category: -
 Roundabout

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h	HV %	[Total veh/h	HV %				[Veh. veh	Dist] ft				
South: 51st Avenue NE														
3u	U	1	5.0	1	5.0	0.399	14.8	LOS B	2.6	67.0	0.70	0.73	0.70	35.7
3	L2	15	5.0	15	5.0	0.399	12.6	LOS B	2.6	67.0	0.70	0.73	0.70	35.0
8	T1	290	5.0	296	5.0	0.399	7.3	LOS A	2.6	67.0	0.70	0.73	0.70	35.0
18	R2	45	5.0	46	5.0	0.399	7.2	LOS A	2.6	67.0	0.70	0.73	0.70	34.1
Approach		351	5.0	358	5.0	0.399	7.6	LOS A	2.6	67.0	0.70	0.73	0.70	34.9
East: 108th Street NE														
1	L2	25	8.0	26	8.0	0.340	12.8	LOS B	2.1	54.8	0.68	0.75	0.68	34.9
6	T1	155	8.0	158	8.0	0.340	7.5	LOS A	2.1	54.8	0.68	0.75	0.68	35.0
16	R2	105	8.0	107	8.0	0.340	7.4	LOS A	2.1	54.8	0.68	0.75	0.68	34.0
Approach		285	8.0	291	8.0	0.340	8.0	LOS A	2.1	54.8	0.68	0.75	0.68	34.6
North: 51st Avenue NE														
7u	U	1	2.0	1	2.0	0.421	12.9	LOS B	2.9	72.8	0.50	0.59	0.50	36.2
7	L2	100	2.0	102	2.0	0.421	10.7	LOS B	2.9	72.8	0.50	0.59	0.50	35.5
4	T1	240	2.0	245	2.0	0.421	5.4	LOS A	2.9	72.8	0.50	0.59	0.50	35.5
14	R2	140	2.0	143	2.0	0.421	5.3	LOS A	2.9	72.8	0.50	0.59	0.50	34.6
Approach		481	2.0	491	2.0	0.421	6.5	LOS A	2.9	72.8	0.50	0.59	0.50	35.2
West: Shoulttes Road														
5	L2	215	3.0	219	3.0	0.411	11.6	LOS B	2.6	67.6	0.61	0.71	0.61	34.4
2	T1	185	3.0	189	3.0	0.411	6.4	LOS A	2.6	67.6	0.61	0.71	0.61	34.4
12	R2	15	3.0	15	3.0	0.411	6.3	LOS A	2.6	67.6	0.61	0.71	0.61	33.5
Approach		415	3.0	423	3.0	0.411	9.1	LOS A	2.6	67.6	0.61	0.71	0.61	34.4
All Vehicles		1532	4.1	1563	4.1	0.421	7.7	LOS A	2.9	72.8	0.61	0.68	0.61	34.8

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).
























Queue Model: HCM Queue Formula.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

HCM 6th Signalized Intersection Summary
3: Smokey Point Blvd & 156th St NE

Williams Industrial
Future (2026) Without-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	20	5	230	45	10	110	255	770	30	65	845	75
Future Volume (veh/h)	20	5	230	45	10	110	255	770	30	65	845	75
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1856	1856	1856	1841	1841	1841
Adj Flow Rate, veh/h	21	5	242	47	11	116	268	811	32	68	889	79
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	4	4	4	3	3	3	4	4	4
Cap, veh/h	334	345	292	404	29	304	386	1329	52	401	1176	104
Arrive On Green	0.03	0.19	0.19	0.06	0.21	0.21	0.12	0.38	0.38	0.10	0.36	0.36
Sat Flow, veh/h	1767	1856	1570	1753	137	1442	1767	3457	136	1753	3248	289
Grp Volume(v), veh/h	21	5	242	47	0	127	268	413	430	68	479	489
Grp Sat Flow(s),veh/h/ln	1767	1856	1570	1753	0	1579	1767	1763	1831	1753	1749	1788
Q Serve(g_s), s	0.7	0.2	11.0	1.5	0.0	5.1	6.8	14.0	14.0	1.6	17.8	17.8
Cycle Q Clear(g_c), s	0.7	0.2	11.0	1.5	0.0	5.1	6.8	14.0	14.0	1.6	17.8	17.8
Prop In Lane	1.00		1.00	1.00		0.91	1.00		0.07	1.00		0.16
Lane Grp Cap(c), veh/h	334	345	292	404	0	333	386	678	704	401	633	647
V/C Ratio(X)	0.06	0.01	0.83	0.12	0.00	0.38	0.69	0.61	0.61	0.17	0.76	0.76
Avail Cap(c_a), veh/h	633	626	529	774	0	639	525	1070	1111	577	1061	1085
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	23.0	24.6	29.1	21.8	0.0	25.1	15.2	18.4	18.4	12.2	20.8	20.8
Incr Delay (d2), s/veh	0.1	0.0	6.0	0.1	0.0	0.7	2.4	0.9	0.9	0.2	1.9	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.1	4.5	0.6	0.0	1.9	2.7	5.5	5.7	0.6	7.1	7.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	23.0	24.7	35.1	21.9	0.0	25.8	17.6	19.3	19.2	12.4	22.6	22.6
LnGrp LOS	C	C	D	C	A	C	B	B	B	B	C	C
Approach Vol, veh/h		268			174			1111			1036	
Approach Delay, s/veh		34.0			24.7			18.9			22.0	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.5	33.5	9.3	18.8	14.2	31.8	7.5	20.7				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	45.0	20.0	25.0	15.0	45.0	15.0	30.0				
Max Q Clear Time (g_c+I1), s	3.6	16.0	3.5	13.0	8.8	19.8	2.7	7.1				
Green Ext Time (p_c), s	0.1	6.0	0.1	0.6	0.4	7.0	0.0	0.7				
Intersection Summary												
HCM 6th Ctrl Delay			22.1									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary
4: Smokey Point Blvd & 152nd St NE

Williams Industrial
Future (2026) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	15	10	5	170	5	275	5	755	265	325	785	5
Future Volume (veh/h)	15	10	5	170	5	275	5	755	265	325	785	5
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1856	1856	1856
Adj Flow Rate, veh/h	16	11	5	185	5	299	5	821	288	353	853	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	3	3	3
Cap, veh/h	204	224	102	483	6	366	9	883	309	117	1469	9
Arrive On Green	0.02	0.19	0.19	0.07	0.24	0.24	0.01	0.35	0.35	0.07	0.41	0.41
Sat Flow, veh/h	1753	1197	544	1753	26	1536	1753	2539	890	1767	3593	21
Grp Volume(v), veh/h	16	0	16	185	0	304	5	565	544	353	418	440
Grp Sat Flow(s),veh/h/ln	1753	0	1742	1753	0	1562	1753	1749	1680	1767	1763	1852
Q Serve(g_s), s	0.4	0.0	0.5	4.0	0.0	11.1	0.2	18.8	18.8	4.0	11.1	11.1
Cycle Q Clear(g_c), s	0.4	0.0	0.5	4.0	0.0	11.1	0.2	18.8	18.8	4.0	11.1	11.1
Prop In Lane	1.00		0.31	1.00		0.98	1.00		0.53	1.00		0.01
Lane Grp Cap(c), veh/h	204	0	327	483	0	372	9	608	584	117	721	757
V/C Ratio(X)	0.08	0.00	0.05	0.38	0.00	0.82	0.54	0.93	0.93	3.01	0.58	0.58
Avail Cap(c_a), veh/h	293	0	607	483	0	545	116	610	586	117	721	757
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.8	0.0	20.1	18.8	0.0	21.7	29.9	18.9	19.0	28.1	13.8	13.8
Incr Delay (d2), s/veh	0.2	0.0	0.1	0.5	0.0	6.2	40.2	20.9	21.7	926.1	1.2	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.2	2.1	0.0	4.4	0.2	10.2	10.0	31.8	4.1	4.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	20.0	0.0	20.1	19.3	0.0	27.9	70.1	39.8	40.7	954.2	15.0	14.9
LnGrp LOS	C	A	C	B	A	C	E	D	D	F	B	B
Approach Vol, veh/h		32			489			1114			1211	
Approach Delay, s/veh		20.1			24.6			40.4			288.7	
Approach LOS		C			C			D			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	25.9	9.0	16.3	5.3	29.6	5.9	19.4				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	21.0	4.0	21.0	4.0	21.0	4.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	20.8	6.0	2.5	2.2	13.1	2.4	13.1				
Green Ext Time (p_c), s	0.0	0.1	0.0	0.0	0.0	3.3	0.0	1.2				
Intersection Summary												
HCM 6th Ctrl Delay	143.1											
HCM 6th LOS	F											




Intersection	
Intersection Delay, s/veh	171.8
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	55	255	185	55	260	25	165	275	75	55	335	90
Future Vol, veh/h	55	255	185	55	260	25	165	275	75	55	335	90
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles, %	3	3	3	4	4	4	1	1	1	2	2	2
Mvmt Flow	56	260	189	56	265	26	168	281	77	56	342	92
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	186.8	74.5	216.3	177.5
HCM LOS	F	F	F	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	32%	11%	16%	11%
Vol Thru, %	53%	52%	76%	70%
Vol Right, %	15%	37%	7%	19%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	515	495	340	480
LT Vol	165	55	55	55
Through Vol	275	255	260	335
RT Vol	75	185	25	90
Lane Flow Rate	526	505	347	490
Geometry Grp	1	1	1	1
Degree of Util (X)	1.373	1.3	0.93	1.273
Departure Headway (Hd)	11.2	11.122	12.694	11.444
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	331	333	288	323
Service Time	9.2	9.122	10.694	9.444
HCM Lane V/C Ratio	1.589	1.517	1.205	1.517
HCM Control Delay	216.3	186.8	74.5	177.5
HCM Lane LOS	F	F	F	F
HCM 95th-tile Q	22.4	20	8.8	18.7

Intersection	
Intersection Delay, s/veh	24.4
Intersection LOS	C

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	170	215	195	245	275	155
Future Vol, veh/h	170	215	195	245	275	155
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	175	222	201	253	284	160
Number of Lanes	1	0	0	1	1	0

Approach	EB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	1	1
Conflicting Approach Left	SB	EB	
Conflicting Lanes Left	1	1	0
Conflicting Approach Right	NB		EB
Conflicting Lanes Right	1	0	1
HCM Control Delay	21.9	27.6	23.3
HCM LOS	C	D	C

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	44%	44%	0%
Vol Thru, %	56%	0%	64%
Vol Right, %	0%	56%	36%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	440	385	430
LT Vol	195	170	0
Through Vol	245	0	275
RT Vol	0	215	155
Lane Flow Rate	454	397	443
Geometry Grp	1	1	1
Degree of Util (X)	0.778	0.688	0.728
Departure Headway (Hd)	6.171	6.236	5.915
Convergence, Y/N	Yes	Yes	Yes
Cap	585	583	612
Service Time	4.22	4.236	3.964
HCM Lane V/C Ratio	0.776	0.681	0.724
HCM Control Delay	27.6	21.9	23.3
HCM Lane LOS	D	C	C
HCM 95th-tile Q	7.2	5.3	6.2

HCM 6th Signalized Intersection Summary
7: Smokey Point Blvd & 136th St NE

Williams Industrial
Future (2026) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	185	240	85	200	240	135	120	655	195	155	595	230
Future Volume (veh/h)	185	240	85	200	240	135	120	655	195	155	595	230
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1796	1796	1796	1870	1870	1870	1856	1856	1856	1870	1870	1870
Adj Flow Rate, veh/h	195	253	89	211	253	142	126	689	205	163	626	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	7	7	7	2	2	2	3	3	3	2	2	2
Cap, veh/h	247	464	394	352	291	163	0	887	264	104	1646	
Arrive On Green	0.06	0.26	0.26	0.06	0.26	0.26	0.00	0.33	0.33	0.06	0.46	0.00
Sat Flow, veh/h	1711	1796	1522	1781	1125	632	0	2678	796	1781	3647	0
Grp Volume(v), veh/h	195	253	89	211	0	395	0	454	440	163	626	0
Grp Sat Flow(s),veh/h/ln	1711	1796	1522	1781	0	1757	0	1763	1711	1781	1777	0
Q Serve(g_s), s	4.0	8.3	3.1	4.0	0.0	14.7	0.0	15.8	15.8	4.0	7.8	0.0
Cycle Q Clear(g_c), s	4.0	8.3	3.1	4.0	0.0	14.7	0.0	15.8	15.8	4.0	7.8	0.0
Prop In Lane	1.00		1.00	1.00		0.36	0.00		0.47	1.00		0.00
Lane Grp Cap(c), veh/h	247	464	394	352	0	454	0	584	567	104	1646	
V/C Ratio(X)	0.79	0.54	0.23	0.60	0.00	0.87	0.00	0.78	0.78	1.56	0.38	
Avail Cap(c_a), veh/h	247	710	602	352	0	540	0	774	752	104	1646	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	23.7	21.8	19.9	21.5	0.0	24.2	0.0	20.5	20.6	32.1	11.9	0.0
Incr Delay (d2), s/veh	15.7	1.0	0.3	2.8	0.0	12.6	0.0	3.6	3.7	294.2	0.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.3	3.4	1.1	1.4	0.0	7.3	0.0	6.6	6.4	10.2	2.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.5	22.8	20.2	24.3	0.0	36.8	0.0	24.2	24.3	326.3	12.1	0.0
LnGrp LOS	D	C	C	C	A	D	A	C	C	F	B	
Approach Vol, veh/h		537			606			894			789	
Approach Delay, s/veh		28.4			32.5			24.2			77.0	
Approach LOS		C			C			C			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	27.6	9.0	22.7	0.0	36.6	9.0	22.7				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	30.0	4.0	27.0	4.0	21.0	4.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	17.8	6.0	10.3	0.0	9.8	6.0	16.7				
Green Ext Time (p_c), s	0.0	4.7	0.0	1.6	0.0	3.2	0.0	1.0				

Intersection Summary

HCM 6th Ctrl Delay	41.5
HCM 6th LOS	D

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

HCM Signalized Intersection Capacity Analysis

8: 51st Ave NE & 136th St NE

Williams Industrial
Future (2026) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕	↗		↕	↗	↗	↖			↖	↗	
Traffic Volume (vph)	205	0	355	5	0	5	285	370	0	0	380	120	
Future Volume (vph)	205	0	355	5	0	5	285	370	0	0	380	120	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00			1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00			1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00	0.95	1.00			1.00	1.00	
Satd. Flow (prot)		1787	1584		1805	1615	1656	1743			1863	1583	
Flt Permitted		0.95	1.00		0.95	1.00	0.29	1.00			1.00	1.00	
Satd. Flow (perm)		1787	1584		1805	1615	513	1743			1863	1583	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	218	0	378	5	0	5	303	394	0	0	404	128	
RTOR Reduction (vph)	0	0	228	0	0	5	0	0	0	0	0	40	
Lane Group Flow (vph)	0	218	150	0	5	0	303	394	0	0	404	88	
Confl. Peds. (#/hr)			1	1					2	2			
Heavy Vehicles (%)	1%	1%	1%	0%	0%	0%	9%	9%	9%	2%	2%	2%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	D.P+P	NA			NA	pm+ov	
Protected Phases	4	4	5	8	8		5	2			6	4	
Permitted Phases			4			8	6					6	
Actuated Green, G (s)		12.2	27.4		1.1	1.1	35.8	40.8			20.6	32.8	
Effective Green, g (s)		12.2	27.4		1.1	1.1	35.8	40.8			20.6	32.8	
Actuated g/C Ratio		0.18	0.40		0.02	0.02	0.52	0.59			0.30	0.47	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Vehicle Extension (s)		2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0	
Lane Grp Cap (vph)		315	628		28	25	517	1029			555	865	
v/s Ratio Prot		c0.12	0.05		c0.00		c0.13	0.23			c0.22	0.02	
v/s Ratio Perm			0.04			0.00	0.17					0.04	
v/c Ratio		0.69	0.24		0.18	0.00	0.59	0.38			0.73	0.10	
Uniform Delay, d1		26.7	13.9		33.6	33.5	10.7	7.5			21.7	10.0	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Incremental Delay, d2		5.2	0.1		1.1	0.0	1.1	0.1			4.0	0.0	
Delay (s)		31.9	14.0		34.7	33.5	11.8	7.6			25.8	10.0	
Level of Service		C	B		C	C	B	A			C	B	
Approach Delay (s)		20.5			34.1			9.4			22.0		
Approach LOS		C			C			A			C		
Intersection Summary													
HCM 2000 Control Delay			16.8		HCM 2000 Level of Service							B	
HCM 2000 Volume to Capacity ratio			0.66										
Actuated Cycle Length (s)			69.1		Sum of lost time (s)						20.0		
Intersection Capacity Utilization			66.3%		ICU Level of Service						C		
Analysis Period (min)			15										
c Critical Lane Group													

Intersection	
Intersection Delay, s/veh	67.2
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	5	65	0	190	0	430	70	225	440	0
Future Vol, veh/h	0	0	5	65	0	190	0	430	70	225	440	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.95	0.94	0.95	0.94	0.94	0.94	0.94	0.95
Heavy Vehicles, %	5	5	5	6	5	6	5	9	9	2	2	5
Mvmt Flow	0	0	5	69	0	202	0	457	74	239	468	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	11.1	16.5	37.7	109.3
HCM LOS	B	C	E	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	0%	0%	25%	34%
Vol Thru, %	86%	0%	0%	66%
Vol Right, %	14%	100%	75%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	500	5	255	665
LT Vol	0	0	65	225
Through Vol	430	0	0	440
RT Vol	70	5	190	0
Lane Flow Rate	532	5	271	707
Geometry Grp	1	1	1	1
Degree of Util (X)	0.874	0.011	0.496	1.155
Departure Headway (Hd)	6.184	7.975	6.906	5.879
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	590	451	526	615
Service Time	4.184	5.975	4.906	3.92
HCM Lane V/C Ratio	0.902	0.011	0.515	1.15
HCM Control Delay	37.7	11.1	16.5	109.3
HCM Lane LOS	E	B	C	F
HCM 95th-tile Q	10	0	2.7	23.2

HCM 6th Signalized Intersection Summary
 10: 51st Ave NE & 122nd PI NE

Williams Industrial
 Future (2026) Without-Project PM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	25	125	100	485	525	35
Future Volume (veh/h)	25	125	100	485	525	35
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	0.99	1.00			0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1856	1856	1796	1796	1856	1856
Adj Flow Rate, veh/h	26	129	103	500	541	36
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	3	3	7	7	3	3
Cap, veh/h	35	171	463	1122	710	47
Arrive On Green	0.13	0.13	0.09	0.62	0.41	0.41
Sat Flow, veh/h	266	1318	1711	1796	1718	114
Grp Volume(v), veh/h	156	0	103	500	0	577
Grp Sat Flow(s),veh/h/ln	1594	0	1711	1796	0	1832
Q Serve(g_s), s	3.5	0.0	1.2	5.3	0.0	9.9
Cycle Q Clear(g_c), s	3.5	0.0	1.2	5.3	0.0	9.9
Prop In Lane	0.17	0.83	1.00			0.06
Lane Grp Cap(c), veh/h	207	0	463	1122	0	758
V/C Ratio(X)	0.75	0.00	0.22	0.45	0.00	0.76
Avail Cap(c_a), veh/h	1107	0	801	1248	0	1273
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	15.4	0.0	6.4	3.6	0.0	9.2
Incr Delay (d2), s/veh	4.1	0.0	0.1	0.3	0.0	1.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	0.0	0.2	0.7	0.0	3.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	19.5	0.0	6.5	3.9	0.0	10.8
LnGrp LOS	B	A	A	A	A	B
Approach Vol, veh/h	156			603	577	
Approach Delay, s/veh	19.5			4.3	10.8	
Approach LOS	B			A	B	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		27.4		9.3	7.7	19.7
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		25.5		25.5	10.5	25.5
Max Q Clear Time (g_c+I1), s		7.3		5.5	3.2	11.9
Green Ext Time (p_c), s		3.1		0.3	0.1	3.3
Intersection Summary						
HCM 6th Ctrl Delay			8.9			
HCM 6th LOS			A			

MOVEMENT SUMMARY

Site: 11 [51st Avenue NE/108th Street NE - Baseline 2026 (Site Folder: General)]

Future (2026) Without-Project PM Peak Hour
 Site Category: -
 Roundabout

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h	HV %	[Total veh/h	HV %				[Veh. veh	Dist] ft				
South: 51st Avenue NE														
3u	U	1	5.0	1	5.0	0.503	16.0	LOS B	3.9	101.6	0.78	0.84	0.85	35.3
3	L2	15	5.0	15	5.0	0.503	13.7	LOS B	3.9	101.6	0.78	0.84	0.85	34.6
8	T1	360	5.0	367	5.0	0.503	8.5	LOS A	3.9	101.6	0.78	0.84	0.85	34.7
18	R2	50	5.0	51	5.0	0.503	8.4	LOS A	3.9	101.6	0.78	0.84	0.85	33.8
Approach		426	5.0	435	5.0	0.503	8.7	LOS A	3.9	101.6	0.78	0.84	0.85	34.6
East: 108th Street NE														
1	L2	25	8.0	26	8.0	0.393	13.6	LOS B	2.5	67.7	0.76	0.82	0.76	34.5
6	T1	165	8.0	168	8.0	0.393	8.3	LOS A	2.5	67.7	0.76	0.82	0.76	34.7
16	R2	110	8.0	112	8.0	0.393	8.3	LOS A	2.5	67.7	0.76	0.82	0.76	33.7
Approach		300	8.0	306	8.0	0.393	8.8	LOS A	2.5	67.7	0.76	0.82	0.76	34.3
North: 51st Avenue NE														
7u	U	1	2.0	1	2.0	0.510	13.1	LOS B	3.9	98.9	0.57	0.60	0.57	36.0
7	L2	105	2.0	107	2.0	0.510	10.9	LOS B	3.9	98.9	0.57	0.60	0.57	35.3
4	T1	320	2.0	327	2.0	0.510	5.6	LOS A	3.9	98.9	0.57	0.60	0.57	35.4
14	R2	150	2.0	153	2.0	0.510	5.5	LOS A	3.9	98.9	0.57	0.60	0.57	34.4
Approach		576	2.0	588	2.0	0.510	6.6	LOS A	3.9	98.9	0.57	0.60	0.57	35.1
West: Shoulttes Road														
5	L2	230	3.0	235	3.0	0.470	12.4	LOS B	3.3	84.8	0.71	0.78	0.72	34.0
2	T1	195	3.0	199	3.0	0.470	7.2	LOS A	3.3	84.8	0.71	0.78	0.72	34.1
12	R2	15	3.0	15	3.0	0.470	7.1	LOS A	3.3	84.8	0.71	0.78	0.72	33.2
Approach		440	3.0	449	3.0	0.470	9.9	LOS A	3.3	84.8	0.71	0.78	0.72	34.0
All Vehicles		1742	4.0	1778	4.0	0.510	8.3	LOS A	3.9	101.6	0.69	0.74	0.71	34.6

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).












Queue Model: HCM Queue Formula.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

HCM 6th Signalized Intersection Summary
 1: Smokey Point Blvd & 160th St NE

Williams Industrial
 Future (2032) Without-Project PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	25	25	1120	25	25	1285
Future Volume (veh/h)	25	25	1120	25	25	1285
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	27	27	1217	27	27	1397
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	104	93	1693	38	59	2322
Arrive On Green	0.06	0.06	0.48	0.48	0.03	0.65
Sat Flow, veh/h	1781	1585	3648	79	1781	3647
Grp Volume(v), veh/h	27	27	608	636	27	1397
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1856	1781	1777
Q Serve(g_s), s	0.5	0.6	9.5	9.5	0.5	7.8
Cycle Q Clear(g_c), s	0.5	0.6	9.5	9.5	0.5	7.8
Prop In Lane	1.00	1.00		0.04	1.00	
Lane Grp Cap(c), veh/h	104	93	846	884	59	2322
V/C Ratio(X)	0.26	0.29	0.72	0.72	0.46	0.60
Avail Cap(c_a), veh/h	257	228	1024	1070	257	3072
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.6	15.6	7.2	7.2	16.5	3.4
Incr Delay (d2), s/veh	1.3	1.7	1.9	1.9	5.5	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.2	2.5	2.5	0.3	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	16.9	17.4	9.2	9.1	22.0	3.7
LnGrp LOS	B	B	A	A	C	A
Approach Vol, veh/h	54		1244			1424
Approach Delay, s/veh	17.1		9.1			4.0
Approach LOS	B		A			A
Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	6.1	21.5			27.7	7.0
Change Period (Y+Rc), s	5.0	5.0			5.0	5.0
Max Green Setting (Gmax), s	5.0	20.0			30.0	5.0
Max Q Clear Time (g_c+I1), s	2.5	11.5			9.8	2.6
Green Ext Time (p_c), s	0.0	5.1			10.9	0.0
Intersection Summary						
HCM 6th Ctrl Delay			6.6			
HCM 6th LOS			A			

HCM 6th Signalized Intersection Summary
2: 51st Ave NE & 160th St NE

Williams Industrial
Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	20	0	20	0	470	10	10	610	0
Future Volume (veh/h)	0	0	0	20	0	20	0	470	10	10	610	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	0	0	0	22	0	22	0	511	11	11	663	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	6	6	0	50	0	82	6	761	16	26	1130	0
Arrive On Green	0.00	0.00	0.00	0.03	0.00	0.05	0.00	0.42	0.42	0.01	0.60	0.00
Sat Flow, veh/h	1781	1870	0	1781	0	1585	1781	1824	39	1781	1870	0
Grp Volume(v), veh/h	0	0	0	22	0	22	0	0	522	11	663	0
Grp Sat Flow(s),veh/h/ln	1781	1870	0	1781	0	1585	1781	0	1863	1781	1870	0
Q Serve(g_s), s	0.0	0.0	0.0	0.4	0.0	0.4	0.0	0.0	6.6	0.2	6.3	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.4	0.0	0.4	0.0	0.0	6.6	0.2	6.3	0.0
Prop In Lane	1.00		0.00	1.00		1.00	1.00		0.02	1.00		0.00
Lane Grp Cap(c), veh/h	6	6	0	50	0	82	6	0	777	26	1130	0
V/C Ratio(X)	0.00	0.00	0.00	0.44	0.00	0.27	0.00	0.00	0.67	0.42	0.59	0.00
Avail Cap(c_a), veh/h	307	1160	0	307	0	983	307	0	2055	307	2063	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	13.9	0.0	13.2	0.0	0.0	6.9	14.2	3.5	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	6.0	0.0	1.8	0.0	0.0	1.0	10.5	0.5	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0	0.2	0.0	0.1	0.0	0.0	1.5	0.1	0.5	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.0	0.0	0.0	19.9	0.0	15.0	0.0	0.0	7.9	24.7	4.0	0.0
LnGrp LOS	A	A	A	B	A	B	A	A	A	C	A	A
Approach Vol, veh/h		0			44			522			674	
Approach Delay, s/veh		0.0			17.4			7.9			4.4	
Approach LOS					B			A			A	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	5.4	17.1	5.8	0.7	0.0	22.5	0.0	6.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	5.0	32.0	5.0	18.0	5.0	32.0	5.0	18.0				
Max Q Clear Time (g_c+I1), s	2.2	8.6	2.4	0.0	0.0	8.3	0.0	2.4				
Green Ext Time (p_c), s	0.0	3.5	0.0	0.0	0.0	4.9	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			6.3									
HCM 6th LOS			A									

HCM 6th Signalized Intersection Summary

3: Smokey Point Blvd & 156th St NE

Williams Industrial
Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	25	20	255	40	55	65	285	855	30	60	935	90
Future Volume (veh/h)	25	20	255	40	55	65	285	855	30	60	935	90
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1856	1856	1856	1841	1841	1841
Adj Flow Rate, veh/h	26	21	268	42	58	68	300	900	32	63	984	95
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	4	4	4	3	3	3	4	4	4
Cap, veh/h	399	369	312	392	394	333	362	1453	52	339	1146	111
Arrive On Green	0.04	0.20	0.20	0.05	0.21	0.21	0.13	0.42	0.42	0.07	0.36	0.36
Sat Flow, veh/h	1767	1856	1570	1753	1841	1558	1767	3472	123	1753	3221	311
Grp Volume(v), veh/h	26	21	268	42	58	68	300	457	475	63	534	545
Grp Sat Flow(s),veh/h/ln	1767	1856	1570	1753	1841	1558	1767	1763	1833	1753	1749	1784
Q Serve(g_s), s	0.9	0.7	12.6	1.4	2.0	2.7	7.7	15.6	15.6	1.6	21.7	21.7
Cycle Q Clear(g_c), s	0.9	0.7	12.6	1.4	2.0	2.7	7.7	15.6	15.6	1.6	21.7	21.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		0.17
Lane Grp Cap(c), veh/h	399	369	312	392	394	333	362	738	767	339	622	635
V/C Ratio(X)	0.07	0.06	0.86	0.11	0.15	0.20	0.83	0.62	0.62	0.19	0.86	0.86
Avail Cap(c_a), veh/h	492	485	410	458	481	407	432	829	862	381	685	699
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	22.6	24.8	29.6	21.9	24.4	24.7	16.3	17.5	17.5	14.1	22.9	22.9
Incr Delay (d2), s/veh	0.1	0.1	13.2	0.1	0.2	0.3	11.0	1.2	1.1	0.3	9.9	9.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.3	5.7	0.6	0.8	1.0	3.8	6.1	6.3	0.6	10.0	10.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	22.6	24.9	42.9	22.0	24.6	25.0	27.3	18.6	18.6	14.3	32.8	32.7
LnGrp LOS	C	C	D	C	C	C	C	B	B	B	C	C
Approach Vol, veh/h		315			168			1232			1142	
Approach Delay, s/veh		40.0			24.1			20.7			31.7	
Approach LOS		D			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	37.0	9.1	20.2	15.0	32.2	8.0	21.4				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	7.0	36.0	7.0	20.0	13.0	30.0	7.0	20.0				
Max Q Clear Time (g_c+I1), s	3.6	17.6	3.4	14.6	9.7	23.7	2.9	4.7				
Green Ext Time (p_c), s	0.0	5.9	0.0	0.5	0.3	3.5	0.0	0.4				
Intersection Summary												
HCM 6th Ctrl Delay			27.4									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary
4: Smokey Point Blvd & 152nd St NE

























Williams Industrial
Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	20	10	5	200	5	310	5	840	305	365	865	5
Future Volume (veh/h)	20	10	5	200	5	310	5	840	305	365	865	5
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1856	1856	1856
Adj Flow Rate, veh/h	22	11	5	217	5	337	5	913	332	397	940	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	3	3	3
Cap, veh/h	106	218	99	394	5	322	9	915	331	396	2095	11
Arrive On Green	0.02	0.18	0.18	0.05	0.21	0.21	0.01	0.36	0.36	0.22	0.58	0.58
Sat Flow, veh/h	1753	1197	544	1753	23	1538	1753	2514	910	1767	3596	19
Grp Volume(v), veh/h	22	0	16	217	0	342	5	634	611	397	461	484
Grp Sat Flow(s),veh/h/ln	1753	0	1742	1753	0	1561	1753	1749	1675	1767	1763	1852
Q Serve(g_s), s	1.0	0.0	0.7	4.5	0.0	20.5	0.3	35.3	35.6	21.9	14.4	14.4
Cycle Q Clear(g_c), s	1.0	0.0	0.7	4.5	0.0	20.5	0.3	35.3	35.6	21.9	14.4	14.4
Prop In Lane	1.00		0.31	1.00		0.99	1.00		0.54	1.00		0.01
Lane Grp Cap(c), veh/h	106	0	317	394	0	327	9	637	610	396	1027	1079
V/C Ratio(X)	0.21	0.00	0.05	0.55	0.00	1.05	0.55	1.00	1.00	1.00	0.45	0.45
Avail Cap(c_a), veh/h	145	0	356	394	0	327	72	637	610	396	1027	1079
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.3	0.0	33.0	34.0	0.0	38.6	48.5	31.0	31.1	37.9	11.5	11.5
Incr Delay (d2), s/veh	1.0	0.0	0.1	1.6	0.0	62.0	42.9	34.4	37.0	46.0	0.3	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.3	2.6	0.0	13.3	0.2	20.4	20.1	14.3	5.4	5.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	34.2	0.0	33.1	35.7	0.0	100.7	91.4	65.5	68.1	84.0	11.8	11.8
LnGrp LOS	C	A	C	D	A	F	F	E	F	F	B	B
Approach Vol, veh/h		38			559			1250			1342	
Approach Delay, s/veh		33.8			75.4			66.9			33.2	
Approach LOS		C			E			E			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	26.4	40.1	9.0	22.3	5.0	61.5	6.3	25.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	21.9	35.6	4.5	20.0	4.0	53.5	4.0	20.5				
Max Q Clear Time (g_c+I1), s	23.9	37.6	6.5	2.7	2.3	16.4	3.0	22.5				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.0	7.3	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			53.8									
HCM 6th LOS			D									

HCM 6th Signalized Intersection Summary
5: 51st Ave NE & 152nd St NE

Williams Industrial
Future (2032) Without-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	90	270	200	60	290	30	185	300	85	65	370	115
Future Volume (veh/h)	90	270	200	60	290	30	185	300	85	65	370	115
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1885	1885	1885	1870	1870	1870
Adj Flow Rate, veh/h	92	276	204	61	296	31	189	306	87	66	378	117
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	3	3	3	4	4	4	1	1	1	2	2	2
Cap, veh/h	128	672	514	101	614	362	245	657	648	107	509	546
Arrive On Green	0.07	0.19	0.19	0.06	0.18	0.18	0.14	0.35	0.35	0.06	0.27	0.27
Sat Flow, veh/h	1767	3526	1572	1753	3497	1525	1795	1885	1598	1781	1870	1585
Grp Volume(v), veh/h	92	276	204	61	296	31	189	306	87	66	378	117
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1749	1525	1795	1885	1598	1781	1870	1585
Q Serve(g_s), s	2.5	3.4	5.0	1.7	3.8	0.8	5.0	6.3	1.7	1.8	9.1	2.6
Cycle Q Clear(g_c), s	2.5	3.4	5.0	1.7	3.8	0.8	5.0	6.3	1.7	1.8	9.1	2.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	128	672	514	101	614	362	245	657	648	107	509	546
V/C Ratio(X)	0.72	0.41	0.40	0.61	0.48	0.09	0.77	0.47	0.13	0.62	0.74	0.21
Avail Cap(c_a), veh/h	246	2136	1167	485	2599	1228	489	1351	1237	496	974	941
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	22.5	17.6	12.9	22.8	18.4	14.8	20.6	12.5	9.2	22.7	16.4	11.5
Incr Delay (d2), s/veh	7.3	0.4	0.5	5.8	0.6	0.1	5.1	0.5	0.1	5.6	2.2	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.2	1.3	1.6	0.8	1.4	0.3	2.2	2.3	0.5	0.9	3.7	0.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	29.8	18.0	13.4	28.6	19.0	14.9	25.8	13.1	9.3	28.3	18.6	11.7
LnGrp LOS	C	B	B	C	B	B	C	B	A	C	B	B
Approach Vol, veh/h		572			388			582			561	
Approach Delay, s/veh		18.2			20.2			16.6			18.3	
Approach LOS		B			C			B			B	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	7.5	21.3	7.3	13.4	11.3	17.5	8.1	12.7				
Change Period (Y+Rc), s	4.5	4.0	4.5	4.0	4.5	4.0	4.5	4.0				
Max Green Setting (Gmax), s	13.8	35.5	13.7	30.0	13.5	25.8	6.9	36.8				
Max Q Clear Time (g_c+I1), s	3.8	8.3	3.7	7.0	7.0	11.1	4.5	5.8				
Green Ext Time (p_c), s	0.1	2.2	0.1	2.5	0.3	2.4	0.0	2.1				
Intersection Summary												
HCM 6th Ctrl Delay				18.2								
HCM 6th LOS				B								

Intersection

Intersection Delay, s/veh 38.5
Intersection LOS E























Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			W	W	
Traffic Vol, veh/h	170	245	220	275	310	170
Future Vol, veh/h	170	245	220	275	310	170
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	175	253	227	284	320	175
Number of Lanes	1	0	0	1	1	0

Approach	EB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	1	1
Conflicting Approach Left	SB	EB	
Conflicting Lanes Left	1	1	0
Conflicting Approach Right	NB		EB
Conflicting Lanes Right	1	0	1
HCM Control Delay	29	47.7	37.1
HCM LOS	D	E	E

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	44%	41%	0%
Vol Thru, %	56%	0%	65%
Vol Right, %	0%	59%	35%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	495	415	480
LT Vol	220	170	0
Through Vol	275	0	310
RT Vol	0	245	170
Lane Flow Rate	510	428	495
Geometry Grp	1	1	1
Degree of Util (X)	0.925	0.778	0.864
Departure Headway (Hd)	6.522	6.546	6.284
Convergence, Y/N	Yes	Yes	Yes
Cap	556	552	574
Service Time	4.584	4.603	4.346
HCM Lane V/C Ratio	0.917	0.775	0.862
HCM Control Delay	47.7	29	37.1
HCM Lane LOS	E	D	E
HCM 95th-tile Q	11.4	7.1	9.6

HCM 6th Signalized Intersection Summary
7: Smokey Point Blvd & 136th St NE

Williams Industrial
Future (2032) Without-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	210	270	95	225	270	150	135	735	220	175	650	260
Future Volume (veh/h)	210	270	95	225	270	150	135	735	220	175	650	260
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1796	1796	1796	1870	1870	1870	1856	1856	1856	1870	1870	1870
Adj Flow Rate, veh/h	221	284	100	237	284	158	142	774	232	184	684	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	7	7	7	2	2	2	3	3	3	2	2	2
Cap, veh/h	241	500	424	351	303	169	0	980	294	172	1807	
Arrive On Green	0.09	0.28	0.28	0.08	0.27	0.27	0.00	0.37	0.37	0.10	0.51	0.00
Sat Flow, veh/h	1711	1796	1522	1781	1129	628	0	2672	801	1781	3647	0
Grp Volume(v), veh/h	221	284	100	237	0	442	0	511	495	184	684	0
Grp Sat Flow(s),veh/h/ln	1711	1796	1522	1781	0	1757	0	1763	1710	1781	1777	0
Q Serve(g_s), s	8.5	13.4	5.0	7.5	0.0	24.3	0.0	25.5	25.5	9.5	11.6	0.0
Cycle Q Clear(g_c), s	8.5	13.4	5.0	7.5	0.0	24.3	0.0	25.5	25.5	9.5	11.6	0.0
Prop In Lane	1.00		1.00	1.00		0.36	0.00		0.47	1.00		0.00
Lane Grp Cap(c), veh/h	241	500	424	351	0	472	0	646	627	172	1807	
V/C Ratio(X)	0.92	0.57	0.24	0.67	0.00	0.94	0.00	0.79	0.79	1.07	0.38	
Avail Cap(c_a), veh/h	241	510	432	351	0	481	0	1465	1421	172	3083	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	28.7	30.5	27.5	28.3	0.0	35.3	0.0	27.9	27.9	44.6	14.8	0.0
Incr Delay (d2), s/veh	36.5	1.4	0.3	5.0	0.0	25.9	0.0	2.2	2.3	89.4	0.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.2	5.9	1.8	2.0	0.0	13.5	0.0	10.8	10.5	8.4	4.5	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	65.2	31.9	27.8	33.3	0.0	61.2	0.0	30.1	30.1	134.0	14.9	0.0
LnGrp LOS	E	C	C	C	A	E	A	C	C	F	B	
Approach Vol, veh/h		605			679			1006			868	
Approach Delay, s/veh		43.4			51.5			30.1			40.1	
Approach LOS		D			D			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.0	40.7	12.0	32.0	0.0	54.7	13.0	31.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.5	82.0	7.5	28.0	5.9	85.6	8.5	27.0				
Max Q Clear Time (g_c+I1), s	11.5	27.5	9.5	15.4	0.0	13.6	10.5	26.3				
Green Ext Time (p_c), s	0.0	8.7	0.0	1.6	0.0	5.7	0.0	0.2				
Intersection Summary												
HCM 6th Ctrl Delay			40.0									
HCM 6th LOS			D									
Notes												
Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.												

HCM Signalized Intersection Capacity Analysis

8: 51st Ave NE & 136th St NE

Williams Industrial
Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↕	↗		↕	↗	↗	↗			↕	↗		
Traffic Volume (vph)	230	0	400	5	0	5	325	410	0	0	420	135		
Future Volume (vph)	230	0	400	5	0	5	325	410	0	0	420	135		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00		
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00			1.00	1.00		
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00		
Frt		1.00	0.85		1.00	0.85	1.00	1.00			1.00	0.85		
Flt Protected		0.95	1.00		0.95	1.00	0.95	1.00			1.00	1.00		
Satd. Flow (prot)		1787	1581		1803	1615	1656	1743			1863	1583		
Flt Permitted		0.95	1.00		0.95	1.00	0.26	1.00			1.00	1.00		
Satd. Flow (perm)		1787	1581		1803	1615	446	1743			1863	1583		
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94		
Adj. Flow (vph)	245	0	426	5	0	5	346	436	0	0	447	144		
RTOR Reduction (vph)	0	0	200	0	0	5	0	0	0	0	0	52		
Lane Group Flow (vph)	0	245	226	0	5	0	346	436	0	0	447	92		
Confl. Peds. (#/hr)			1	1					2	2				
Heavy Vehicles (%)	1%	1%	1%	0%	0%	0%	9%	9%	9%	2%	2%	2%		
Turn Type	Split	NA	pm+ov	Split	NA	Perm	D.P+P	NA			NA	pm+ov		
Protected Phases	4	4	5	8	8		5	2			6	4		
Permitted Phases			4			8	6					6		
Actuated Green, G (s)		14.4	27.5		0.9	0.9	35.4	40.4			22.3	36.7		
Effective Green, g (s)		14.4	27.5		0.9	0.9	35.4	40.4			22.3	36.7		
Actuated g/C Ratio		0.20	0.39		0.01	0.01	0.50	0.57			0.32	0.52		
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0		
Vehicle Extension (s)		2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0		
Lane Grp Cap (vph)		363	614		22	20	447	996			587	933		
v/s Ratio Prot		c0.14	0.07		c0.00		c0.14	0.25			0.24	0.02		
v/s Ratio Perm			0.07			0.00	c0.24					0.04		
v/c Ratio		0.67	0.37		0.23	0.00	0.77	0.44			0.76	0.10		
Uniform Delay, d1		26.0	15.4		34.6	34.5	12.5	8.7			21.8	8.6		
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00		
Incremental Delay, d2		3.9	0.1		1.9	0.0	7.5	0.1			5.2	0.0		
Delay (s)		29.9	15.5		36.5	34.5	20.0	8.8			27.0	8.6		
Level of Service		C	B		D	C	C	A			C	A		
Approach Delay (s)		20.8			35.5			13.7			22.5			
Approach LOS		C			D			B			C			
Intersection Summary														
HCM 2000 Control Delay			18.7									HCM 2000 Level of Service	B	
HCM 2000 Volume to Capacity ratio			0.73											
Actuated Cycle Length (s)			70.7								20.0		Sum of lost time (s)	
Intersection Capacity Utilization			72.0%										ICU Level of Service	C
Analysis Period (min)			15											

c Critical Lane Group

Intersection	
Intersection Delay, s/veh	45.1
Intersection LOS	E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	5	70	0	210	0	480	80	250	485	0
Future Vol, veh/h	0	0	5	70	0	210	0	480	80	250	485	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	5	5	5	6	5	6	5	9	9	2	2	5
Mvmt Flow	0	0	5	74	0	223	0	511	85	266	516	0
Number of Lanes	0	1	0	0	1	0	0	1	0	1	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	2	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	2	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	2	1	1
HCM Control Delay	11.2	18.1	67.2	38.7
HCM LOS	B	C	F	E

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	25%	100%	0%
Vol Thru, %	86%	0%	0%	0%	100%
Vol Right, %	14%	100%	75%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	560	5	280	250	485
LT Vol	0	0	70	250	0
Through Vol	480	0	0	0	485
RT Vol	80	5	210	0	0
Lane Flow Rate	596	5	298	266	516
Geometry Grp	5	2	2	7	7
Degree of Util (X)	1.019	0.012	0.558	0.52	0.936
Departure Headway (Hd)	6.16	8.06	6.747	7.144	6.633
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	584	447	530	508	552
Service Time	4.242	6.06	4.836	4.844	4.333
HCM Lane V/C Ratio	1.021	0.011	0.562	0.524	0.935
HCM Control Delay	67.2	11.2	18.1	17.3	49.7
HCM Lane LOS	F	B	C	C	E
HCM 95th-tile Q	15.5	0	3.4	3	11.8

HCM 6th Signalized Intersection Summary
 10: 51st Ave NE & 122nd PI NE

Williams Industrial
 Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	30	145	115	535	580	40
Future Volume (veh/h)	30	145	115	535	580	40
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	0.99	1.00			0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1856	1856	1796	1796	1856	1856
Adj Flow Rate, veh/h	31	149	119	552	598	41
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	3	3	7	7	3	3
Cap, veh/h	41	195	435	1152	767	53
Arrive On Green	0.15	0.15	0.09	0.64	0.45	0.45
Sat Flow, veh/h	273	1313	1711	1796	1714	118
Grp Volume(v), veh/h	181	0	119	552	0	639
Grp Sat Flow(s),veh/h/ln	1595	0	1711	1796	0	1831
Q Serve(g_s), s	4.7	0.0	1.5	6.8	0.0	12.7
Cycle Q Clear(g_c), s	4.7	0.0	1.5	6.8	0.0	12.7
Prop In Lane	0.17	0.82	1.00			0.06
Lane Grp Cap(c), veh/h	237	0	435	1152	0	820
V/C Ratio(X)	0.76	0.00	0.27	0.48	0.00	0.78
Avail Cap(c_a), veh/h	749	0	503	1926	0	1536
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	17.5	0.0	7.2	4.0	0.0	10.0
Incr Delay (d2), s/veh	3.8	0.0	0.1	0.3	0.0	1.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.7	0.0	0.3	1.2	0.0	4.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	21.3	0.0	7.3	4.3	0.0	11.7
LnGrp LOS	C	A	A	A	A	B
Approach Vol, veh/h	181			671	639	
Approach Delay, s/veh	21.3			4.8	11.7	
Approach LOS	C			A	B	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		32.0		10.9	8.3	23.7
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		45.9		20.1	5.5	35.9
Max Q Clear Time (g_c+I1), s		8.8		6.7	3.5	14.7
Green Ext Time (p_c), s		4.2		0.3	0.0	4.5
Intersection Summary						
HCM 6th Ctrl Delay			9.8			
HCM 6th LOS			A			

MOVEMENT SUMMARY

**Site: 11 [51st Avenue NE/108th Street NE - With-Project 2026
(Site Folder: General)]**

Future (2026) With-Project PM Peak Hour
Site Category: -
Roundabout

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h	HV %	[Total veh/h	HV %				[Veh. veh	Dist] ft				
South: 51st Avenue NE														
3u	U	1	5.0	1	5.0	0.518	16.3	LOS B	4.2	108.2	0.79	0.85	0.88	35.3
3	L2	15	5.0	15	5.0	0.518	14.0	LOS B	4.2	108.2	0.79	0.85	0.88	34.6
8	T1	368	5.0	376	5.0	0.518	8.8	LOS A	4.2	108.2	0.79	0.85	0.88	34.7
18	R2	50	5.0	51	5.0	0.518	8.7	LOS A	4.2	108.2	0.79	0.85	0.88	33.7
Approach		434	5.0	443	5.0	0.518	8.9	LOS A	4.2	108.2	0.79	0.85	0.88	34.6
East: 108th Street NE														
1	L2	25	8.0	26	8.0	0.400	13.8	LOS B	2.6	70.2	0.77	0.83	0.78	34.5
6	T1	165	8.0	168	8.0	0.400	8.5	LOS A	2.6	70.2	0.77	0.83	0.78	34.6
16	R2	112	8.0	114	8.0	0.400	8.4	LOS A	2.6	70.2	0.77	0.83	0.78	33.7
Approach		302	8.0	308	8.0	0.400	8.9	LOS A	2.6	70.2	0.77	0.83	0.78	34.2
North: 51st Avenue NE														
7u	U	1	2.0	1	2.0	0.549	13.2	LOS B	4.4	111.6	0.60	0.61	0.60	36.0
7	L2	110	2.0	112	2.0	0.549	11.0	LOS B	4.4	111.6	0.60	0.61	0.60	35.3
4	T1	347	2.0	354	2.0	0.549	5.7	LOS A	4.4	111.6	0.60	0.61	0.60	35.3
14	R2	161	2.0	164	2.0	0.549	5.6	LOS A	4.4	111.6	0.60	0.61	0.60	34.4
Approach		619	2.0	632	2.0	0.549	6.6	LOS A	4.4	111.6	0.60	0.61	0.60	35.0
West: Shoulttes Road														
5	L2	233	3.0	238	3.0	0.488	12.9	LOS B	3.6	93.0	0.74	0.82	0.78	33.8
2	T1	195	3.0	199	3.0	0.488	7.7	LOS A	3.6	93.0	0.74	0.82	0.78	33.9
12	R2	15	3.0	15	3.0	0.488	7.6	LOS A	3.6	93.0	0.74	0.82	0.78	33.0
Approach		443	3.0	452	3.0	0.488	10.4	LOS B	3.6	93.0	0.74	0.82	0.78	33.8
All Vehicles		1798	4.0	1835	4.0	0.549	8.5	LOS A	4.4	111.6	0.71	0.76	0.74	34.5

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: HCM Queue Formula.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

HCM Signalized Intersection Capacity Analysis
 12: 152nd St NE & 156th St NE

Williams Industrial
 Future (2032) Without-Project PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↙	↑↑	↙	↗
Traffic Volume (vph)	465	0	75	260	0	95
Future Volume (vph)	465	0	75	260	0	95
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0	4.0		4.5
Lane Util. Factor	0.95		1.00	0.95		1.00
Frt	1.00		1.00	1.00		0.85
Flt Protected	1.00		0.95	1.00		1.00
Satd. Flow (prot)	3374		1770	3539		1568
Flt Permitted	1.00		0.95	1.00		1.00
Satd. Flow (perm)	3374		1770	3539		1568
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	489	0	79	274	0	100
RTOR Reduction (vph)	0	0	0	0	0	92
Lane Group Flow (vph)	489	0	79	274	0	8
Heavy Vehicles (%)	7%	7%	2%	2%	3%	3%
Turn Type	NA		Split	NA	Prot	Perm
Protected Phases			8	8	2	
Permitted Phases	4					2
Actuated Green, G (s)	12.4		6.5	6.5		2.6
Effective Green, g (s)	12.4		6.5	6.5		2.6
Actuated g/C Ratio	0.36		0.19	0.19		0.08
Clearance Time (s)	4.0		4.0	4.0		4.5
Vehicle Extension (s)	3.0		3.0	3.0		3.0
Lane Grp Cap (vph)	1230		338	676		119
v/s Ratio Prot			0.04	c0.08		
v/s Ratio Perm	c0.14					c0.00
v/c Ratio	0.40		0.23	0.41		0.06
Uniform Delay, d1	8.0		11.6	12.1		14.6
Progression Factor	1.00		1.00	1.00		1.00
Incremental Delay, d2	0.2		0.4	0.4		0.2
Delay (s)	8.2		12.0	12.5		14.8
Level of Service	A		B	B		B
Approach Delay (s)	8.2			12.4	14.8	
Approach LOS	A			B	B	

Intersection Summary			
HCM 2000 Control Delay	10.5	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.36		
Actuated Cycle Length (s)	34.0	Sum of lost time (s)	12.5
Intersection Capacity Utilization	25.8%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

HCM 6th Signalized Intersection Summary
 13: 47th Ave NE & 156th St NE

Williams Industrial
 Future (2032) Without-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Volume (veh/h)	0	465	0	0	260	0	0	0	0	0	0	0
Future Volume (veh/h)	0	465	0	0	260	0	0	0	0	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826
Adj Flow Rate, veh/h	0	489	0	0	274	0	0	0	0	0	0	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	5	5	5	5	5	5	5	5	5	5	5	5
Cap, veh/h	3	778	0	3	778	0	3	1075	0	3	1075	0
Arrive On Green	0.00	0.22	0.00	0.00	0.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	1739	3561	0	1739	3561	0	1739	1826	0	1739	1826	0
Grp Volume(v), veh/h	0	489	0	0	274	0	0	0	0	0	0	0
Grp Sat Flow(s),veh/h/ln	1739	1735	0	1739	1735	0	1739	1826	0	1739	1826	0
Q Serve(g_s), s	0.0	6.8	0.0	0.0	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	6.8	0.0	0.0	3.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	1.00		0.00	1.00		0.00	1.00		0.00	1.00		0.00
Lane Grp Cap(c), veh/h	3	778	0	3	778	0	3	1075	0	3	1075	0
V/C Ratio(X)	0.00	0.63	0.00	0.00	0.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Avail Cap(c_a), veh/h	569	1848	0	163	1070	0	163	1075	0	569	1075	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	18.7	0.0	0.0	17.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.8	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	2.5	0.0	0.0	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.0	19.6	0.0	0.0	17.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS	A	B	A	A	B	A	A	A	A	A	A	A
Approach Vol, veh/h		489			274			0			0	
Approach Delay, s/veh		19.6			17.8			0.0			0.0	
Approach LOS		B			B							
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	0.0	36.5	0.0	17.0	0.0	36.5	0.0	17.0				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	* 5				
Max Green Setting (Gmax), s	17.5	19.0	5.0	28.5	5.0	31.5	17.5	* 17				
Max Q Clear Time (g_c+I1), s	0.0	0.0	0.0	8.8	0.0	0.0	0.0	5.6				
Green Ext Time (p_c), s	0.0	0.0	0.0	3.2	0.0	0.0	0.0	1.2				

Intersection Summary

HCM 6th Ctrl Delay	18.9
HCM 6th LOS	B
























Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM 6th Signalized Intersection Summary

3: Smokey Point Blvd & 156th St NE

Williams Industrial
Future (2026) With-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	20	5	230	45	10	110	255	803	30	65	854	75
Future Volume (veh/h)	20	5	230	45	10	110	255	803	30	65	854	75
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1856	1856	1856	1841	1841	1841
Adj Flow Rate, veh/h	21	5	242	47	11	116	268	845	32	68	899	79
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	4	4	4	3	3	3	4	4	4
Cap, veh/h	333	344	291	404	29	304	382	1440	55	363	1187	104
Arrive On Green	0.03	0.19	0.19	0.06	0.21	0.21	0.12	0.42	0.42	0.07	0.36	0.36
Sat Flow, veh/h	1767	1856	1570	1753	137	1442	1767	3463	131	1753	3251	286
Grp Volume(v), veh/h	21	5	242	47	0	127	268	430	447	68	484	494
Grp Sat Flow(s),veh/h/ln	1767	1856	1570	1753	0	1579	1767	1763	1832	1753	1749	1788
Q Serve(g_s), s	0.7	0.2	11.0	1.5	0.0	5.1	6.7	14.0	14.0	1.7	18.0	18.0
Cycle Q Clear(g_c), s	0.7	0.2	11.0	1.5	0.0	5.1	6.7	14.0	14.0	1.7	18.0	18.0
Prop In Lane	1.00		1.00	1.00		0.91	1.00		0.07	1.00		0.16
Lane Grp Cap(c), veh/h	333	344	291	404	0	333	382	733	762	363	638	653
V/C Ratio(X)	0.06	0.01	0.83	0.12	0.00	0.38	0.70	0.59	0.59	0.19	0.76	0.76
Avail Cap(c_a), veh/h	750	624	528	773	0	425	523	1067	1109	592	1058	1082
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	23.0	24.7	29.1	21.8	0.0	25.2	15.1	16.8	16.8	13.1	20.7	20.7
Incr Delay (d2), s/veh	0.1	0.0	6.1	0.1	0.0	0.7	2.6	0.7	0.7	0.2	1.9	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.1	4.5	0.6	0.0	1.9	2.6	5.4	5.6	0.6	7.2	7.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	23.1	24.7	35.2	22.0	0.0	25.9	17.7	17.5	17.5	13.3	22.6	22.6
LnGrp LOS	C	C	D	C	A	C	B	B	B	B	C	C
Approach Vol, veh/h		268			174			1145			1046	
Approach Delay, s/veh		34.1			24.8			17.6			22.0	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.3	35.9	9.3	18.8	14.1	32.1	7.5	20.7				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	45.0	20.0	25.0	15.0	45.0	20.0	20.0				
Max Q Clear Time (g_c+I1), s	3.7	16.0	3.5	13.0	8.7	20.0	2.7	7.1				
Green Ext Time (p_c), s	0.1	6.3	0.1	0.6	0.4	7.1	0.0	0.5				
Intersection Summary												
HCM 6th Ctrl Delay			21.5									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary
 4: Smokey Point Blvd & 152nd St NE

Williams Industrial
 Future (2026) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	15	10	5	197	5	308	5	755	273	334	785	5
Future Volume (veh/h)	15	10	5	197	5	308	5	755	273	334	785	5
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1856	1856	1856
Adj Flow Rate, veh/h	16	11	5	214	5	335	5	821	297	363	853	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	3	3	3
Cap, veh/h	196	250	114	506	6	397	9	853	308	114	1431	8
Arrive On Green	0.02	0.21	0.21	0.06	0.26	0.26	0.01	0.34	0.34	0.06	0.40	0.40
Sat Flow, veh/h	1753	1198	544	1753	23	1539	1753	2516	909	1767	3593	21
Grp Volume(v), veh/h	16	0	16	214	0	340	5	570	548	363	418	440
Grp Sat Flow(s),veh/h/ln	1753	0	1742	1753	0	1562	1753	1749	1676	1767	1763	1852
Q Serve(g_s), s	0.4	0.0	0.5	4.0	0.0	12.8	0.2	19.8	19.9	4.0	11.6	11.6
Cycle Q Clear(g_c), s	0.4	0.0	0.5	4.0	0.0	12.8	0.2	19.8	19.9	4.0	11.6	11.6
Prop In Lane	1.00		0.31	1.00		0.99	1.00		0.54	1.00		0.01
Lane Grp Cap(c), veh/h	196	0	364	506	0	403	9	593	568	114	702	737
V/C Ratio(X)	0.08	0.00	0.04	0.42	0.00	0.84	0.54	0.96	0.96	3.18	0.60	0.60
Avail Cap(c_a), veh/h	282	0	590	506	0	529	113	593	568	114	702	737
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	19.6	0.0	19.6	18.8	0.0	21.8	30.7	20.1	20.1	29.0	14.7	14.7
Incr Delay (d2), s/veh	0.2	0.0	0.0	0.6	0.0	9.3	40.3	27.7	28.8	1004.2	1.4	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.2	2.5	0.0	5.3	0.2	11.7	11.5	33.4	4.4	4.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	19.7	0.0	19.6	19.4	0.0	31.1	71.1	47.8	48.9	1033.2	16.1	16.0
LnGrp LOS	B	A	B	B	A	C	E	D	D	F	B	B
Approach Vol, veh/h		32			554			1123			1221	
Approach Delay, s/veh		19.7			26.6			48.4			318.5	
Approach LOS		B			C			D			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	26.0	9.0	18.0	5.3	29.7	6.0	21.0				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	21.0	4.0	21.0	4.0	21.0	4.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	21.9	6.0	2.5	2.2	13.6	2.4	14.8				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.0	3.2	0.0	1.1				

Intersection Summary												
HCM 6th Ctrl Delay	156.5											
HCM 6th LOS	F											




Intersection	
Intersection Delay, s/veh	322.9
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	78	255	185	55	260	54	165	306	75	158	444	172
Future Vol, veh/h	78	255	185	55	260	54	165	306	75	158	444	172
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles, %	3	3	3	4	4	4	1	1	1	2	2	2
Mvmt Flow	80	260	189	56	265	55	168	312	77	161	453	176
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	230.5	105.7	270	525.7
HCM LOS	F	F	F	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	30%	15%	15%	20%
Vol Thru, %	56%	49%	70%	57%
Vol Right, %	14%	36%	15%	22%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	546	518	369	774
LT Vol	165	78	55	158
Through Vol	306	255	260	444
RT Vol	75	185	54	172
Lane Flow Rate	557	529	377	790
Geometry Grp	1	1	1	1
Degree of Util (X)	1.478	1.384	1.005	2.084
Departure Headway (Hd)	14.295	13.97	16.141	12.297
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	262	264	230	308
Service Time	12.295	11.97	14.141	10.297
HCM Lane V/C Ratio	2.126	2.004	1.639	2.565
HCM Control Delay	270	230.5	105.7	525.7
HCM Lane LOS	F	F	F	F
HCM 95th-tile Q	21.5	19.3	9.2	44.8

Intersection	
Intersection Delay, s/veh	40.2
Intersection LOS	E

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	230	259	207	245	275	172
Future Vol, veh/h	230	259	207	245	275	172
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	237	267	213	253	284	177
Number of Lanes	1	0	0	1	1	0

Approach	EB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	1	1
Conflicting Approach Left	SB	EB	
Conflicting Lanes Left	1	1	0
Conflicting Approach Right	NB		EB
Conflicting Lanes Right	1	0	1
HCM Control Delay	44.6	41.2	34.3
HCM LOS	E	E	D

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	46%	47%	0%
Vol Thru, %	54%	0%	62%
Vol Right, %	0%	53%	38%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	452	489	447
LT Vol	207	230	0
Through Vol	245	0	275
RT Vol	0	259	172
Lane Flow Rate	466	504	461
Geometry Grp	1	1	1
Degree of Util (X)	0.878	0.908	0.833
Departure Headway (Hd)	6.783	6.487	6.506
Convergence, Y/N	Yes	Yes	Yes
Cap	531	560	553
Service Time	4.854	4.547	4.577
HCM Lane V/C Ratio	0.878	0.9	0.834
HCM Control Delay	41.2	44.6	34.3
HCM Lane LOS	E	E	D
HCM 95th-tile Q	9.7	10.9	8.6

HCM 6th Signalized Intersection Summary

7: Smokey Point Blvd & 136th St NE

Williams Industrial
Future (2026) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	190	240	85	200	240	135	120	658	195	155	606	246
Future Volume (veh/h)	190	240	85	200	240	135	120	658	195	155	606	246
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1796	1796	1796	1870	1870	1870	1856	1856	1856	1870	1870	1870
Adj Flow Rate, veh/h	200	253	89	211	253	142	126	693	205	163	638	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	7	7	7	2	2	2	3	3	3	2	2	2
Cap, veh/h	246	464	393	352	291	163	0	891	264	104	1648	
Arrive On Green	0.06	0.26	0.26	0.06	0.26	0.26	0.00	0.33	0.33	0.06	0.46	0.00
Sat Flow, veh/h	1711	1796	1522	1781	1125	632	0	2682	793	1781	3647	0
Grp Volume(v), veh/h	200	253	89	211	0	395	0	455	443	163	638	0
Grp Sat Flow(s),veh/h/ln	1711	1796	1522	1781	0	1757	0	1763	1712	1781	1777	0
Q Serve(g_s), s	4.0	8.3	3.2	4.0	0.0	14.7	0.0	15.9	15.9	4.0	8.0	0.0
Cycle Q Clear(g_c), s	4.0	8.3	3.2	4.0	0.0	14.7	0.0	15.9	15.9	4.0	8.0	0.0
Prop In Lane	1.00		1.00	1.00		0.36	0.00		0.46	1.00		0.00
Lane Grp Cap(c), veh/h	246	464	393	352	0	454	0	586	569	104	1648	
V/C Ratio(X)	0.81	0.55	0.23	0.60	0.00	0.87	0.00	0.78	0.78	1.56	0.39	
Avail Cap(c_a), veh/h	246	709	601	352	0	539	0	773	751	104	1648	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	24.1	21.9	20.0	21.6	0.0	24.3	0.0	20.6	20.6	32.2	12.0	0.0
Incr Delay (d2), s/veh	18.3	1.0	0.3	2.8	0.0	12.7	0.0	3.7	3.8	295.4	0.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	3.4	1.1	1.4	0.0	7.3	0.0	6.6	6.5	10.2	2.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	42.3	22.9	20.3	24.4	0.0	36.9	0.0	24.3	24.4	327.6	12.1	0.0
LnGrp LOS	D	C	C	C	A	D	A	C	C	F	B	
Approach Vol, veh/h		542			606			898			801	
Approach Delay, s/veh		29.6			32.6			24.3			76.3	
Approach LOS		C			C			C			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	27.7	9.0	22.7	0.0	36.7	9.0	22.7				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	4.0	30.0	4.0	27.0	4.0	21.0	4.0	21.0				
Max Q Clear Time (g_c+I1), s	6.0	17.9	6.0	10.3	0.0	10.0	6.0	16.7				
Green Ext Time (p_c), s	0.0	4.7	0.0	1.6	0.0	3.2	0.0	1.0				

Intersection Summary

HCM 6th Ctrl Delay	41.7
HCM 6th LOS	D

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

HCM Signalized Intersection Capacity Analysis

8: 51st Ave NE & 136th St NE

Williams Industrial
Future (2026) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕	↗		↕	↗	↗	↗			↕	↗	
Traffic Volume (vph)	207	0	355	5	0	5	285	388	0	0	445	126	
Future Volume (vph)	207	0	355	5	0	5	285	388	0	0	445	126	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00			1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00			1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00	0.95	1.00			1.00	1.00	
Satd. Flow (prot)		1787	1583		1805	1615	1656	1743			1863	1583	
Flt Permitted		0.95	1.00		0.95	1.00	0.20	1.00			1.00	1.00	
Satd. Flow (perm)		1787	1583		1805	1615	345	1743			1863	1583	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	220	0	378	5	0	5	303	413	0	0	473	134	
RTOR Reduction (vph)	0	0	228	0	0	5	0	0	0	0	0	38	
Lane Group Flow (vph)	0	220	150	0	5	0	303	413	0	0	473	96	
Confl. Peds. (#/hr)			1	1					2	2			
Heavy Vehicles (%)	1%	1%	1%	0%	0%	0%	9%	9%	9%	2%	2%	2%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	D.P+P	NA			NA	pm+ov	
Protected Phases	4	4	5	8	8		5	2			6	4	
Permitted Phases			4			8	6					6	
Actuated Green, G (s)		12.3	27.5		1.1	1.1	35.8	40.8			20.6	32.9	
Effective Green, g (s)		12.3	27.5		1.1	1.1	35.8	40.8			20.6	32.9	
Actuated g/C Ratio		0.18	0.40		0.02	0.02	0.52	0.59			0.30	0.48	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Vehicle Extension (s)		2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0	
Lane Grp Cap (vph)		317	629		28	25	466	1027			554	866	
v/s Ratio Prot		c0.12	0.05		c0.00		c0.14	0.24			c0.25	0.02	
v/s Ratio Perm			0.04			0.00	0.19					0.04	
v/c Ratio		0.69	0.24		0.18	0.00	0.65	0.40			0.85	0.11	
Uniform Delay, d1		26.7	13.9		33.6	33.5	11.6	7.6			22.9	10.0	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Incremental Delay, d2		5.2	0.1		1.1	0.0	2.5	0.1			11.8	0.0	
Delay (s)		31.9	14.0		34.7	33.5	14.0	7.7			34.6	10.1	
Level of Service		C	B		C	C	B	A			C	B	
Approach Delay (s)		20.6			34.1			10.4			29.2		
Approach LOS		C			C			B			C		
Intersection Summary													
HCM 2000 Control Delay			19.6		HCM 2000 Level of Service							B	
HCM 2000 Volume to Capacity ratio			0.73										
Actuated Cycle Length (s)			69.2		Sum of lost time (s)						20.0		
Intersection Capacity Utilization			69.8%		ICU Level of Service						C		
Analysis Period (min)			15										

c Critical Lane Group

Intersection	
Intersection Delay, s/veh	93.2
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	5	65	0	193	0	445	70	236	494	0
Future Vol, veh/h	0	0	5	65	0	193	0	445	70	236	494	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.95	0.94	0.95	0.94	0.94	0.94	0.94	0.95
Heavy Vehicles, %	5	5	5	6	5	6	5	9	9	2	2	5
Mvmt Flow	0	0	5	69	0	205	0	473	74	251	526	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	1	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	1	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	1	1	1
HCM Control Delay	11.4	17	42.7	156.4
HCM LOS	B	C	E	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	0%	0%	25%	32%
Vol Thru, %	86%	0%	0%	68%
Vol Right, %	14%	100%	75%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	515	5	258	730
LT Vol	0	0	65	236
Through Vol	445	0	0	494
RT Vol	70	5	193	0
Lane Flow Rate	548	5	274	777
Geometry Grp	1	1	1	1
Degree of Util (X)	0.903	0.011	0.502	1.276
Departure Headway (Hd)	6.33	8.287	7.101	5.915
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	575	434	511	614
Service Time	4.33	6.287	5.101	3.961
HCM Lane V/C Ratio	0.953	0.012	0.536	1.265
HCM Control Delay	42.7	11.4	17	156.4
HCM Lane LOS	E	B	C	F
HCM 95th-tile Q	10.9	0	2.8	30.4

HCM 6th Signalized Intersection Summary
 10: 51st Ave NE & 122nd PI NE

Williams Industrial
 Future (2026) With-Project PM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	26	125	100	499	568	41
Future Volume (veh/h)	26	125	100	499	568	41
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	0.99	1.00			0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1856	1856	1796	1796	1856	1856
Adj Flow Rate, veh/h	27	129	103	514	586	42
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	3	3	7	7	3	3
Cap, veh/h	36	171	440	1145	743	53
Arrive On Green	0.13	0.13	0.09	0.64	0.43	0.43
Sat Flow, veh/h	274	1310	1711	1796	1708	122
Grp Volume(v), veh/h	157	0	103	514	0	628
Grp Sat Flow(s),veh/h/ln	1594	0	1711	1796	0	1830
Q Serve(g_s), s	3.7	0.0	1.2	5.6	0.0	11.4
Cycle Q Clear(g_c), s	3.7	0.0	1.2	5.6	0.0	11.4
Prop In Lane	0.17	0.82	1.00			0.07
Lane Grp Cap(c), veh/h	208	0	440	1145	0	796
V/C Ratio(X)	0.75	0.00	0.23	0.45	0.00	0.79
Avail Cap(c_a), veh/h	1048	0	756	1181	0	1204
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	16.3	0.0	6.7	3.6	0.0	9.4
Incr Delay (d2), s/veh	4.1	0.0	0.1	0.3	0.0	2.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	0.0	0.3	0.8	0.0	3.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	20.3	0.0	6.8	3.9	0.0	11.5
LnGrp LOS	C	A	A	A	A	B
Approach Vol, veh/h	157			617	628	
Approach Delay, s/veh	20.3			4.3	11.5	
Approach LOS	C			A	B	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		29.2		9.6	7.9	21.4
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		25.5		25.5	10.5	25.5
Max Q Clear Time (g_c+I1), s		7.6		5.7	3.2	13.4
Green Ext Time (p_c), s		3.2		0.3	0.1	3.4
Intersection Summary						
HCM 6th Ctrl Delay			9.3			
HCM 6th LOS			A			

MOVEMENT SUMMARY

**Site: 11 [51st Avenue NE/108th Street NE - With-Project 2026
(Site Folder: General)]**

Future (2026) With-Project PM Peak Hour
Site Category: -
Roundabout

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h	HV %	[Total veh/h	HV %				[Veh. veh	Dist] ft				
South: 51st Avenue NE														
3u	U	1	5.0	1	5.0	0.518	16.3	LOS B	4.2	108.2	0.79	0.85	0.88	35.3
3	L2	15	5.0	15	5.0	0.518	14.0	LOS B	4.2	108.2	0.79	0.85	0.88	34.6
8	T1	368	5.0	376	5.0	0.518	8.8	LOS A	4.2	108.2	0.79	0.85	0.88	34.7
18	R2	50	5.0	51	5.0	0.518	8.7	LOS A	4.2	108.2	0.79	0.85	0.88	33.7
Approach		434	5.0	443	5.0	0.518	8.9	LOS A	4.2	108.2	0.79	0.85	0.88	34.6
East: 108th Street NE														
1	L2	25	8.0	26	8.0	0.400	13.8	LOS B	2.6	70.2	0.77	0.83	0.78	34.5
6	T1	165	8.0	168	8.0	0.400	8.5	LOS A	2.6	70.2	0.77	0.83	0.78	34.6
16	R2	112	8.0	114	8.0	0.400	8.4	LOS A	2.6	70.2	0.77	0.83	0.78	33.7
Approach		302	8.0	308	8.0	0.400	8.9	LOS A	2.6	70.2	0.77	0.83	0.78	34.2
North: 51st Avenue NE														
7u	U	1	2.0	1	2.0	0.549	13.2	LOS B	4.4	111.6	0.60	0.61	0.60	36.0
7	L2	110	2.0	112	2.0	0.549	11.0	LOS B	4.4	111.6	0.60	0.61	0.60	35.3
4	T1	347	2.0	354	2.0	0.549	5.7	LOS A	4.4	111.6	0.60	0.61	0.60	35.3
14	R2	161	2.0	164	2.0	0.549	5.6	LOS A	4.4	111.6	0.60	0.61	0.60	34.4
Approach		619	2.0	632	2.0	0.549	6.6	LOS A	4.4	111.6	0.60	0.61	0.60	35.0
West: Shoulttes Road														
5	L2	233	3.0	238	3.0	0.488	12.9	LOS B	3.6	93.0	0.74	0.82	0.78	33.8
2	T1	195	3.0	199	3.0	0.488	7.7	LOS A	3.6	93.0	0.74	0.82	0.78	33.9
12	R2	15	3.0	15	3.0	0.488	7.6	LOS A	3.6	93.0	0.74	0.82	0.78	33.0
Approach		443	3.0	452	3.0	0.488	10.4	LOS B	3.6	93.0	0.74	0.82	0.78	33.8
All Vehicles		1798	4.0	1835	4.0	0.549	8.5	LOS A	4.4	111.6	0.71	0.76	0.74	34.5

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).














Queue Model: HCM Queue Formula.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

HCM 6th Signalized Intersection Summary
 1: Smokey Point Blvd & 160th St NE

Williams Industrial
 Future (2032) With-Project PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			 			 
Traffic Volume (veh/h)	25	79	1120	25	40	1285
Future Volume (veh/h)	25	79	1120	25	40	1285
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	27	86	1217	27	43	1397
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	165	146	1627	36	86	2274
Arrive On Green	0.09	0.09	0.46	0.46	0.05	0.64
Sat Flow, veh/h	1781	1585	3648	79	1781	3647
Grp Volume(v), veh/h	27	86	608	636	43	1397
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1856	1781	1777
Q Serve(g_s), s	0.5	1.9	10.5	10.6	0.9	8.7
Cycle Q Clear(g_c), s	0.5	1.9	10.5	10.6	0.9	8.7
Prop In Lane	1.00	1.00		0.04	1.00	
Lane Grp Cap(c), veh/h	165	146	814	850	86	2274
V/C Ratio(X)	0.16	0.59	0.75	0.75	0.50	0.61
Avail Cap(c_a), veh/h	238	212	951	994	238	2854
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.6	16.3	8.3	8.3	17.3	4.0
Incr Delay (d2), s/veh	0.5	3.7	2.8	2.7	4.5	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.7	3.2	3.3	0.4	1.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	16.1	20.0	11.1	11.0	21.8	4.3
LnGrp LOS	B	B	B	B	C	A
Approach Vol, veh/h	113		1244			1440
Approach Delay, s/veh	19.0		11.1			4.8
Approach LOS	B		B			A
Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	6.8	22.1			28.9	8.5
Change Period (Y+Rc), s	5.0	5.0			5.0	5.0
Max Green Setting (Gmax), s	5.0	20.0			30.0	5.0
Max Q Clear Time (g_c+I1), s	2.9	12.6			10.7	3.9
Green Ext Time (p_c), s	0.0	4.6			10.6	0.0
Intersection Summary						
HCM 6th Ctrl Delay			8.2			
HCM 6th LOS			A			

HCM 6th Signalized Intersection Summary
2: 51st Ave NE & 160th St NE

























Williams Industrial
Future (2032) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	47	6	169	20	2	20	48	498	10	10	659	14
Future Volume (veh/h)	47	6	169	20	2	20	48	498	10	10	659	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	51	7	184	22	2	22	52	541	11	11	716	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	84	9	241	45	18	199	85	872	18	25	809	17
Arrive On Green	0.05	0.16	0.16	0.03	0.14	0.14	0.05	0.48	0.48	0.01	0.44	0.44
Sat Flow, veh/h	1781	58	1536	1781	134	1472	1781	1827	37	1781	1825	38
Grp Volume(v), veh/h	51	0	191	22	0	24	52	0	552	11	0	731
Grp Sat Flow(s),veh/h/ln	1781	0	1594	1781	0	1605	1781	0	1864	1781	0	1863
Q Serve(g_s), s	1.7	0.0	7.0	0.7	0.0	0.8	1.8	0.0	13.5	0.4	0.0	22.0
Cycle Q Clear(g_c), s	1.7	0.0	7.0	0.7	0.0	0.8	1.8	0.0	13.5	0.4	0.0	22.0
Prop In Lane	1.00		0.96	1.00		0.92	1.00		0.02	1.00		0.02
Lane Grp Cap(c), veh/h	84	0	250	45	0	217	85	0	890	25	0	826
V/C Ratio(X)	0.60	0.00	0.76	0.48	0.00	0.11	0.61	0.00	0.62	0.44	0.00	0.88
Avail Cap(c_a), veh/h	145	0	468	145	0	472	145	0	973	145	0	973
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	28.6	0.0	24.7	29.5	0.0	23.3	28.6	0.0	11.9	30.0	0.0	15.6
Incr Delay (d2), s/veh	6.8	0.0	4.8	7.8	0.0	0.2	6.8	0.0	1.1	11.9	0.0	8.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	0.0	2.8	0.4	0.0	0.3	0.9	0.0	5.0	0.2	0.0	9.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	35.4	0.0	29.5	37.3	0.0	23.5	35.4	0.0	13.0	41.9	0.0	24.4
LnGrp LOS	D	A	C	D	A	C	D	A	B	D	A	C
Approach Vol, veh/h		242			46			604			742	
Approach Delay, s/veh		30.8			30.1			14.9			24.6	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	5.9	34.3	6.6	14.6	7.9	32.2	7.9	13.3				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	5.0	32.0	5.0	18.0	5.0	32.0	5.0	18.0				
Max Q Clear Time (g_c+I1), s	2.4	15.5	2.7	9.0	3.8	24.0	3.7	2.8				
Green Ext Time (p_c), s	0.0	3.3	0.0	0.7	0.0	3.1	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			22.1									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary
3: Smokey Point Blvd & 156th St NE

Williams Industrial
Future (2032) With-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	25	49	255	40	159	65	285	855	30	60	935	90
Future Volume (veh/h)	25	49	255	40	159	65	285	855	30	60	935	90
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1856	1856	1856	1841	1841	1841
Adj Flow Rate, veh/h	26	52	268	42	167	68	300	900	32	63	984	95
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	4	4	4	3	3	3	4	4	4
Cap, veh/h	317	370	313	372	395	335	361	1452	52	339	1145	111
Arrive On Green	0.04	0.20	0.20	0.05	0.21	0.21	0.13	0.42	0.42	0.07	0.36	0.36
Sat Flow, veh/h	1767	1856	1570	1753	1841	1558	1767	3472	123	1753	3221	311
Grp Volume(v), veh/h	26	52	268	42	167	68	300	457	475	63	534	545
Grp Sat Flow(s),veh/h/ln	1767	1856	1570	1753	1841	1558	1767	1763	1833	1753	1749	1784
Q Serve(g_s), s	0.9	1.8	12.6	1.4	6.0	2.7	7.7	15.6	15.6	1.6	21.7	21.7
Cycle Q Clear(g_c), s	0.9	1.8	12.6	1.4	6.0	2.7	7.7	15.6	15.6	1.6	21.7	21.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		0.17
Lane Grp Cap(c), veh/h	317	370	313	372	395	335	361	737	767	339	621	634
V/C Ratio(X)	0.08	0.14	0.85	0.11	0.42	0.20	0.83	0.62	0.62	0.19	0.86	0.86
Avail Cap(c_a), veh/h	410	484	409	438	480	406	431	828	861	381	684	698
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	22.7	25.3	29.6	21.9	26.0	24.7	16.3	17.5	17.5	14.1	22.9	22.9
Incr Delay (d2), s/veh	0.1	0.2	12.9	0.1	0.7	0.3	11.2	1.2	1.1	0.3	10.0	9.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.8	5.7	0.6	2.6	1.0	3.8	6.1	6.4	0.6	10.1	10.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	22.8	25.4	42.6	22.0	26.7	25.0	27.5	18.7	18.7	14.4	33.0	32.8
LnGrp LOS	C	C	D	C	C	C	C	B	B	B	C	C
Approach Vol, veh/h		346			277			1232			1142	
Approach Delay, s/veh		38.5			25.6			20.8			31.9	
Approach LOS		D			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	37.1	9.1	20.3	15.0	32.3	8.0	21.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	7.0	36.0	7.0	20.0	13.0	30.0	7.0	20.0				
Max Q Clear Time (g_c+I1), s	3.6	17.6	3.4	14.6	9.7	23.7	2.9	8.0				
Green Ext Time (p_c), s	0.0	5.9	0.0	0.6	0.3	3.5	0.0	0.8				
Intersection Summary												
HCM 6th Ctrl Delay			27.5									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary
 4: Smokey Point Blvd & 152nd St NE




























Williams Industrial
 Future (2032) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	20	10	5	227	5	310	5	840	313	365	865	5
Future Volume (veh/h)	20	10	5	227	5	310	5	840	313	365	865	5
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1841	1841	1841	1841	1841	1841	1841	1841	1841	1856	1856	1856
Adj Flow Rate, veh/h	22	11	5	247	5	337	5	913	340	397	940	5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	4	4	4	4	4	4	4	4	4	3	3	3
Cap, veh/h	106	218	99	394	5	322	9	909	337	396	2095	11
Arrive On Green	0.02	0.18	0.18	0.05	0.21	0.21	0.01	0.36	0.36	0.22	0.58	0.58
Sat Flow, veh/h	1753	1197	544	1753	23	1538	1753	2497	925	1767	3596	19
Grp Volume(v), veh/h	22	0	16	247	0	342	5	638	615	397	461	484
Grp Sat Flow(s),veh/h/ln	1753	0	1742	1753	0	1561	1753	1749	1673	1767	1763	1852
Q Serve(g_s), s	1.0	0.0	0.7	4.5	0.0	20.5	0.3	35.6	35.6	21.9	14.4	14.4
Cycle Q Clear(g_c), s	1.0	0.0	0.7	4.5	0.0	20.5	0.3	35.6	35.6	21.9	14.4	14.4
Prop In Lane	1.00		0.31	1.00		0.99	1.00		0.55	1.00		0.01
Lane Grp Cap(c), veh/h	106	0	317	394	0	327	9	637	609	396	1027	1079
V/C Ratio(X)	0.21	0.00	0.05	0.63	0.00	1.05	0.55	1.00	1.01	1.00	0.45	0.45
Avail Cap(c_a), veh/h	145	0	356	394	0	327	72	637	609	396	1027	1079
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.3	0.0	33.0	35.0	0.0	38.6	48.5	31.1	31.1	37.9	11.5	11.5
Incr Delay (d2), s/veh	1.0	0.0	0.1	3.1	0.0	62.0	42.9	36.2	39.0	46.0	0.3	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.3	3.6	0.0	13.3	0.2	20.8	20.4	14.3	5.4	5.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	34.2	0.0	33.1	38.1	0.0	100.7	91.4	67.3	70.1	84.0	11.8	11.8
LnGrp LOS	C	A	C	D	A	F	F	F	F	F	B	B
Approach Vol, veh/h		38			589			1258			1342	
Approach Delay, s/veh		33.8			74.4			68.8			33.2	
Approach LOS		C			E			E			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	26.4	40.1	9.0	22.3	5.0	61.5	6.3	25.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	21.9	35.6	4.5	20.0	4.0	53.5	4.0	20.5				
Max Q Clear Time (g_c+I1), s	23.9	37.6	6.5	2.7	2.3	16.4	3.0	22.5				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.0	7.3	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			54.6									
HCM 6th LOS			D									

HCM 6th Signalized Intersection Summary
5: 51st Ave NE & 152nd St NE

Williams Industrial
Future (2032) With-Project PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 						 	
Traffic Volume (veh/h)	99	270	200	60	290	62	185	331	85	179	474	148
Future Volume (veh/h)	99	270	200	60	290	62	185	331	85	179	474	148
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1885	1885	1885	1870	1870	1870
Adj Flow Rate, veh/h	101	276	204	61	296	63	189	338	87	183	484	151
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	3	3	3	4	4	4	1	1	1	2	2	2
Cap, veh/h	129	642	498	96	573	455	242	612	607	234	602	626
Arrive On Green	0.07	0.18	0.18	0.05	0.16	0.16	0.13	0.32	0.32	0.13	0.32	0.32
Sat Flow, veh/h	1767	3526	1572	1753	3497	1525	1795	1885	1598	1781	1870	1585
Grp Volume(v), veh/h	101	276	204	61	296	63	189	338	87	183	484	151
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1749	1525	1795	1885	1598	1781	1870	1585
Q Serve(g_s), s	3.1	3.9	5.6	1.9	4.3	1.7	5.6	8.2	2.0	5.5	13.1	3.5
Cycle Q Clear(g_c), s	3.1	3.9	5.6	1.9	4.3	1.7	5.6	8.2	2.0	5.5	13.1	3.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	129	642	498	96	573	455	242	612	607	234	602	626
V/C Ratio(X)	0.78	0.43	0.41	0.63	0.52	0.14	0.78	0.55	0.14	0.78	0.80	0.24
Avail Cap(c_a), veh/h	220	1907	1062	433	2320	1217	437	1207	1110	443	870	853
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	25.3	20.1	14.9	25.7	21.2	14.4	23.2	15.4	11.3	23.3	17.2	11.2
Incr Delay (d2), s/veh	9.7	0.5	0.5	6.7	0.7	0.1	5.5	0.8	0.1	5.6	3.6	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	1.5	1.9	0.9	1.7	0.5	2.6	3.2	0.6	2.5	5.5	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	35.0	20.6	15.4	32.4	21.9	14.5	28.7	16.2	11.4	28.9	20.8	11.4
LnGrp LOS	C	C	B	C	C	B	C	B	B	C	C	B
Approach Vol, veh/h		581			420			614			818	
Approach Delay, s/veh		21.3			22.3			19.4			20.9	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.8	22.0	7.5	14.1	12.0	21.8	8.6	13.1				
Change Period (Y+Rc), s	4.5	4.0	4.5	4.0	4.5	4.0	4.5	4.0				
Max Green Setting (Gmax), s	13.8	35.5	13.7	30.0	13.5	25.8	6.9	36.8				
Max Q Clear Time (g_c+I1), s	7.5	10.2	3.9	7.6	7.6	15.1	5.1	6.3				
Green Ext Time (p_c), s	0.2	2.4	0.1	2.5	0.2	2.7	0.0	2.2				
Intersection Summary												
HCM 6th Ctrl Delay				20.8								
HCM 6th LOS				C								

Intersection

Intersection Delay, s/veh 67
Intersection LOS F

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	224	305	237	275	310	185
Future Vol, veh/h	224	305	237	275	310	185
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	231	314	244	284	320	191
Number of Lanes	1	0	0	1	1	0

Approach	EB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	1	1
Conflicting Approach Left	SB	EB	
Conflicting Lanes Left	1	1	0
Conflicting Approach Right	NB		EB
Conflicting Lanes Right	1	0	1
HCM Control Delay	68.6	75.1	56.8
HCM LOS	F	F	F

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	46%	42%	0%
Vol Thru, %	54%	0%	63%
Vol Right, %	0%	58%	37%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	512	529	495
LT Vol	237	224	0
Through Vol	275	0	310
RT Vol	0	305	185
Lane Flow Rate	528	545	510
Geometry Grp	1	1	1
Degree of Util (X)	1.029	1.012	0.96
Departure Headway (Hd)	7.258	6.875	7.018
Convergence, Y/N	Yes	Yes	Yes
Cap	501	534	521
Service Time	5.258	4.875	5.018
HCM Lane V/C Ratio	1.054	1.021	0.979
HCM Control Delay	75.1	68.6	56.8
HCM Lane LOS	F	F	F
HCM 95th-tile Q	14.8	14.5	12.4

HCM 6th Signalized Intersection Summary

7: Smokey Point Blvd & 136th St NE

Williams Industrial
Future (2032) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	215	270	95	225	270	150	135	738	220	175	661	276
Future Volume (veh/h)	215	270	95	225	270	150	135	738	220	175	661	276
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1796	1796	1796	1870	1870	1870	1856	1856	1856	1870	1870	1870
Adj Flow Rate, veh/h	226	284	100	237	284	158	142	777	232	184	696	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	7	7	7	2	2	2	3	3	3	2	2	2
Cap, veh/h	240	500	424	351	303	169	0	983	294	171	1810	
Arrive On Green	0.09	0.28	0.28	0.08	0.27	0.27	0.00	0.37	0.37	0.10	0.51	0.00
Sat Flow, veh/h	1711	1796	1522	1781	1129	628	0	2675	799	1781	3647	0
Grp Volume(v), veh/h	226	284	100	237	0	442	0	512	497	184	696	0
Grp Sat Flow(s),veh/h/ln	1711	1796	1522	1781	0	1757	0	1763	1711	1781	1777	0
Q Serve(g_s), s	8.5	13.4	5.0	7.5	0.0	24.3	0.0	25.6	25.6	9.5	11.8	0.0
Cycle Q Clear(g_c), s	8.5	13.4	5.0	7.5	0.0	24.3	0.0	25.6	25.6	9.5	11.8	0.0
Prop In Lane	1.00		1.00	1.00		0.36	0.00		0.47	1.00		0.00
Lane Grp Cap(c), veh/h	240	500	424	351	0	471	0	648	629	171	1810	
V/C Ratio(X)	0.94	0.57	0.24	0.68	0.00	0.94	0.00	0.79	0.79	1.07	0.38	
Avail Cap(c_a), veh/h	240	509	431	351	0	480	0	1462	1419	171	3077	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	29.3	30.6	27.5	28.4	0.0	35.4	0.0	27.9	27.9	44.7	14.8	0.0
Incr Delay (d2), s/veh	41.9	1.4	0.3	5.1	0.0	26.1	0.0	2.2	2.3	90.1	0.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.7	5.9	1.8	2.0	0.0	13.6	0.0	10.9	10.5	8.4	4.6	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	71.2	32.0	27.8	33.5	0.0	61.4	0.0	30.1	30.1	134.8	14.9	0.0
LnGrp LOS	E	C	C	C	A	E	A	C	C	F	B	
Approach Vol, veh/h		610			679			1009			880	
Approach Delay, s/veh		45.9			51.7			30.1			40.0	
Approach LOS		D			D			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.0	40.8	12.0	32.0	0.0	54.8	13.0	31.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.5	82.0	7.5	28.0	5.9	85.6	8.5	27.0				
Max Q Clear Time (g_c+I1), s	11.5	27.6	9.5	15.4	0.0	13.8	10.5	26.3				
Green Ext Time (p_c), s	0.0	8.7	0.0	1.6	0.0	5.8	0.0	0.2				

Intersection Summary

HCM 6th Ctrl Delay	40.5
HCM 6th LOS	D





















Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

HCM Signalized Intersection Capacity Analysis

8: 51st Ave NE & 136th St NE

Williams Industrial
Future (2032) With-Project PM Peak Hour

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	232	0	400	5	0	5	325	430	0	0	485	140	
Future Volume (vph)	232	0	400	5	0	5	325	430	0	0	485	140	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00			1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00			1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00	0.95	1.00			1.00	1.00	
Satd. Flow (prot)		1787	1582		1805	1615	1656	1743			1863	1583	
Flt Permitted		0.95	1.00		0.95	1.00	0.19	1.00			1.00	1.00	
Satd. Flow (perm)		1787	1582		1805	1615	334	1743			1863	1583	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	247	0	426	5	0	5	346	457	0	0	516	149	
RTOR Reduction (vph)	0	0	174	0	0	5	0	0	0	0	0	45	
Lane Group Flow (vph)	0	247	252	0	5	0	346	457	0	0	516	104	
Confl. Peds. (#/hr)			1	1					2	2			
Heavy Vehicles (%)	1%	1%	1%	0%	0%	0%	9%	9%	9%	2%	2%	2%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	D.P+P	NA			NA	pm+ov	
Protected Phases	4	4	5	8	8		5	2			6	4	
Permitted Phases			4			8	6					6	
Actuated Green, G (s)		15.2	30.7		1.0	1.0	42.0	47.0			26.5	41.7	
Effective Green, g (s)		15.2	30.7		1.0	1.0	42.0	47.0			26.5	41.7	
Actuated g/C Ratio		0.19	0.39		0.01	0.01	0.54	0.60			0.34	0.53	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	5.0			5.0	5.0	
Vehicle Extension (s)		2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0	
Lane Grp Cap (vph)		347	621		23	20	441	1047			631	945	
v/s Ratio Prot		c0.14	0.08		c0.00		c0.16	0.26			c0.28	0.02	
v/s Ratio Perm			0.08			0.00	0.27					0.04	
v/c Ratio		0.71	0.41		0.22	0.00	0.78	0.44			0.82	0.11	
Uniform Delay, d1		29.5	17.2		38.2	38.1	13.9	8.4			23.6	9.0	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00			1.00	1.00	
Incremental Delay, d2		5.6	0.2		1.7	0.0	8.2	0.1			7.7	0.0	
Delay (s)		35.1	17.3		39.9	38.1	22.1	8.5			31.3	9.1	
Level of Service		D	B		D	D	C	A			C	A	
Approach Delay (s)		23.8			39.0			14.4			26.4		
Approach LOS		C			D			B			C		
Intersection Summary													
HCM 2000 Control Delay			21.2		HCM 2000 Level of Service							C	
HCM 2000 Volume to Capacity ratio			0.77										
Actuated Cycle Length (s)			78.2		Sum of lost time (s)						20.0		
Intersection Capacity Utilization			75.6%		ICU Level of Service						D		
Analysis Period (min)			15										
c Critical Lane Group													

Intersection	
Intersection Delay, s/veh	56.5
Intersection LOS	F

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	5	70	0	213	0	497	80	261	539	0
Future Vol, veh/h	0	0	5	70	0	213	0	497	80	261	539	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.95
Heavy Vehicles, %	5	5	5	6	5	6	5	9	9	2	2	5
Mvmt Flow	0	0	5	74	0	227	0	529	85	278	573	0
Number of Lanes	0	1	0	0	1	0	0	1	0	1	1	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	2	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	2	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	2	1	1
HCM Control Delay	11.3	18.5	76.9	55.5
HCM LOS	B	C	F	F

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	25%	100%	0%
Vol Thru, %	86%	0%	0%	0%	100%
Vol Right, %	14%	100%	75%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	577	5	283	261	539
LT Vol	0	0	70	261	0
Through Vol	497	0	0	0	539
RT Vol	80	5	213	0	0
Lane Flow Rate	614	5	301	278	573
Geometry Grp	5	2	2	7	7
Degree of Util (X)	1.052	0.012	0.562	0.54	1.035
Departure Headway (Hd)	6.305	8.235	6.937	7.187	6.675
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	583	437	524	505	550
Service Time	4.305	6.235	4.937	4.887	4.375
HCM Lane V/C Ratio	1.053	0.011	0.574	0.55	1.042
HCM Control Delay	76.9	11.3	18.5	18	73.6
HCM Lane LOS	F	B	C	C	F
HCM 95th-tile Q	17	0	3.4	3.2	15.7

HCM 6th Signalized Intersection Summary
 10: 51st Ave NE & 122nd PI NE

Williams Industrial
 Future (2032) With-Project PM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	32	145	115	550	628	46
Future Volume (veh/h)	32	145	115	550	628	46
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	0.99	1.00			0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No	No	
Adj Sat Flow, veh/h/ln	1856	1856	1796	1796	1856	1856
Adj Flow Rate, veh/h	33	149	119	567	647	47
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	3	3	7	7	3	3
Cap, veh/h	43	194	410	1176	805	58
Arrive On Green	0.15	0.15	0.09	0.65	0.47	0.47
Sat Flow, veh/h	288	1300	1711	1796	1706	124
Grp Volume(v), veh/h	183	0	119	567	0	694
Grp Sat Flow(s),veh/h/ln	1597	0	1711	1796	0	1830
Q Serve(g_s), s	5.1	0.0	1.5	7.3	0.0	14.8
Cycle Q Clear(g_c), s	5.1	0.0	1.5	7.3	0.0	14.8
Prop In Lane	0.18	0.81	1.00			0.07
Lane Grp Cap(c), veh/h	238	0	410	1176	0	863
V/C Ratio(X)	0.77	0.00	0.29	0.48	0.00	0.80
Avail Cap(c_a), veh/h	699	0	469	1796	0	1431
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	18.8	0.0	7.7	4.0	0.0	10.3
Incr Delay (d2), s/veh	3.9	0.0	0.1	0.3	0.0	1.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.9	0.0	0.4	1.4	0.0	4.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	22.7	0.0	7.9	4.3	0.0	12.1
LnGrp LOS	C	A	A	A	A	B
Approach Vol, veh/h	183			686	694	
Approach Delay, s/veh	22.7			4.9	12.1	
Approach LOS	C			A	B	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		34.6		11.3	8.4	26.2
Change Period (Y+Rc), s		4.5		4.5	4.5	4.5
Max Green Setting (Gmax), s		45.9		20.1	5.5	35.9
Max Q Clear Time (g_c+I1), s		9.3		7.1	3.5	16.8
Green Ext Time (p_c), s		4.3		0.3	0.0	4.8
Intersection Summary						
HCM 6th Ctrl Delay			10.2			
HCM 6th LOS			B			

MOVEMENT SUMMARY

**Site: 10 [51st Avenue NE/108th Street NE - With-Project 2032
(Site Folder: General)]**

Future (2032) With-Project PM Peak Hour
Site Category: (None)
Roundabout

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn v/c	Aver. Delay sec	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed mph
		[Total veh/h]	[HV %]	[Total veh/h]	[HV %]				[Veh. veh]	[Dist ft]				
South: 51st Avenue NE														
3u	U	1	5.0	1	5.0	0.633	19.0	LOS B	6.3	163.7	0.90	1.01	1.14	33.9
3	L2	20	5.0	20	5.0	0.633	16.7	LOS B	6.3	163.7	0.90	1.01	1.14	33.3
8	T1	408	5.0	416	5.0	0.633	11.4	LOS B	6.3	163.7	0.90	1.01	1.14	33.4
18	R2	55	5.0	56	5.0	0.633	11.4	LOS B	6.3	163.7	0.90	1.01	1.14	32.5
Approach		484	5.0	494	5.0	0.633	11.7	LOS B	6.3	163.7	0.90	1.01	1.14	33.3
East: 108th Street NE														
1	L2	30	8.0	31	8.0	0.501	16.1	LOS B	4.0	106.4	0.86	0.96	1.00	33.4
6	T1	185	8.0	189	8.0	0.501	10.8	LOS B	4.0	106.4	0.86	0.96	1.00	33.5
16	R2	127	8.0	130	8.0	0.501	10.7	LOS B	4.0	106.4	0.86	0.96	1.00	32.6
Approach		342	8.0	349	8.0	0.501	11.2	LOS B	4.0	106.4	0.86	0.96	1.00	33.2
North: 51st Avenue NE														
7u	U	1	2.0	1	2.0	0.631	14.0	LOS B	5.9	150.0	0.70	0.68	0.72	35.7
7	L2	125	2.0	128	2.0	0.631	11.7	LOS B	5.9	150.0	0.70	0.68	0.72	34.9
4	T1	382	2.0	390	2.0	0.631	6.5	LOS A	5.9	150.0	0.70	0.68	0.72	35.0
14	R2	181	2.0	185	2.0	0.631	6.4	LOS A	5.9	150.0	0.70	0.68	0.72	34.1
Approach		689	2.0	703	2.0	0.631	7.4	LOS A	5.9	150.0	0.70	0.68	0.72	34.7
West: Shoulttes Road														
5	L2	263	3.0	268	3.0	0.594	14.8	LOS B	5.5	140.8	0.84	0.93	1.00	33.1
2	T1	220	3.0	224	3.0	0.594	9.6	LOS A	5.5	140.8	0.84	0.93	1.00	33.2
12	R2	20	3.0	20	3.0	0.594	9.5	LOS A	5.5	140.8	0.84	0.93	1.00	32.3
Approach		503	3.0	513	3.0	0.594	12.3	LOS B	5.5	140.8	0.84	0.93	1.00	33.1
All Vehicles		2018	4.0	2059	4.0	0.633	10.3	LOS B	6.3	163.7	0.81	0.87	0.94	33.7

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: HCM Queue Formula.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

HCM Signalized Intersection Capacity Analysis
 12: 152nd St NE & 156th St NE

Williams Industrial
 Future (2032) With-Project PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (vph)	465	0	108	260	0	104
Future Volume (vph)	465	0	108	260	0	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0	4.0		4.5
Lane Util. Factor	0.95		1.00	0.95		1.00
Frt	1.00		1.00	1.00		0.85
Flt Protected	1.00		0.95	1.00		1.00
Satd. Flow (prot)	3374		1770	3539		1568
Flt Permitted	1.00		0.95	1.00		1.00
Satd. Flow (perm)	3374		1770	3539		1568
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	489	0	114	274	0	109
RTOR Reduction (vph)	0	0	0	0	0	101
Lane Group Flow (vph)	489	0	114	274	0	8
Heavy Vehicles (%)	7%	7%	2%	2%	3%	3%
Turn Type	NA		Split	NA	Prot	Perm
Protected Phases			8	8	2	
Permitted Phases	4					2
Actuated Green, G (s)	11.5		9.1	9.1		2.7
Effective Green, g (s)	11.5		9.1	9.1		2.7
Actuated g/C Ratio	0.32		0.25	0.25		0.08
Clearance Time (s)	4.0		4.0	4.0		4.5
Vehicle Extension (s)	3.0		3.0	3.0		3.0
Lane Grp Cap (vph)	1083		449	899		118
v/s Ratio Prot			0.06	c0.08		
v/s Ratio Perm	c0.14					c0.01
v/c Ratio	0.45		0.25	0.30		0.07
Uniform Delay, d1	9.6		10.6	10.8		15.4
Progression Factor	1.00		1.00	1.00		1.00
Incremental Delay, d2	0.3		0.3	0.2		0.3
Delay (s)	9.9		10.9	11.0		15.6
Level of Service	A		B	B		B
Approach Delay (s)	9.9			11.0	15.6	
Approach LOS	A			B	B	

Intersection Summary

HCM 2000 Control Delay	11.0	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.35		
Actuated Cycle Length (s)	35.8	Sum of lost time (s)	12.5
Intersection Capacity Utilization	26.4%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

HCM 6th Signalized Intersection Summary
 13: 47th Ave NE & 156th St NE

Williams Industrial
 Future (2032) With-Project PM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Volume (veh/h)	31	465	0	0	260	0	0	0	0	0	0	109
Future Volume (veh/h)	31	465	0	0	260	0	0	0	0	0	0	109
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826	1826
Adj Flow Rate, veh/h	33	489	0	0	274	0	0	0	0	0	0	115
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	5	5	5	5	5	5	5	5	5	5	5	5
Cap, veh/h	62	891	0	3	456	0	3	1030	0	3	0	873
Arrive On Green	0.04	0.26	0.00	0.00	0.13	0.00	0.00	0.00	0.00	0.00	0.00	0.56
Sat Flow, veh/h	1739	3561	0	1739	3561	0	1739	1826	0	1739	0	1547
Grp Volume(v), veh/h	33	489	0	0	274	0	0	0	0	0	0	115
Grp Sat Flow(s),veh/h/ln	1739	1735	0	1739	1735	0	1739	1826	0	1739	0	1547
Q Serve(g_s), s	1.0	6.8	0.0	0.0	4.2	0.0	0.0	0.0	0.0	0.0	0.0	2.0
Cycle Q Clear(g_c), s	1.0	6.8	0.0	0.0	4.2	0.0	0.0	0.0	0.0	0.0	0.0	2.0
Prop In Lane	1.00		0.00	1.00		0.00	1.00		0.00	1.00		1.00
Lane Grp Cap(c), veh/h	62	891	0	3	456	0	3	1030	0	3	0	873
V/C Ratio(X)	0.53	0.55	0.00	0.00	0.60	0.00	0.00	0.00	0.00	0.00	0.00	0.13
Avail Cap(c_a), veh/h	545	1770	0	156	1025	0	156	1030	0	545	0	873
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	26.5	18.0	0.0	0.0	22.9	0.0	0.0	0.0	0.0	0.0	0.0	5.7
Incr Delay (d2), s/veh	6.8	0.5	0.0	0.0	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	2.5	0.0	0.0	1.7	0.0	0.0	0.0	0.0	0.0	0.0	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	33.2	18.5	0.0	0.0	24.1	0.0	0.0	0.0	0.0	0.0	0.0	6.0
LnGrp LOS	C	B	A	A	C	A	A	A	A	A	A	A
Approach Vol, veh/h		522			274			0				115
Approach Delay, s/veh		19.4			24.1			0.0				6.0
Approach LOS		B			C							A
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	0.0	36.5	0.0	19.3	0.0	36.5	7.0	12.3				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	* 5				
Max Green Setting (Gmax), s	17.5	19.0	5.0	28.5	5.0	31.5	17.5	* 17				
Max Q Clear Time (g_c+I1), s	0.0	0.0	0.0	8.8	0.0	4.0	3.0	6.2				
Green Ext Time (p_c), s	0.0	0.0	0.0	3.2	0.0	0.7	0.0	1.2				

Intersection Summary

HCM 6th Ctrl Delay	19.1
HCM 6th LOS	B

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection						
Int Delay, s/veh	2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	5	0	0	15	0	0
Future Vol, veh/h	5	0	0	15	0	0
Conflicting Peds, #/hr	0	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	6	6	9	9	2	2
Mvmt Flow	5	0	0	16	0	0

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	11	10	0	0	18	0
Stage 1	10	-	-	-	-	-
Stage 2	1	-	-	-	-	-
Critical Hdwy	6.46	6.26	-	-	4.12	-
Critical Hdwy Stg 1	5.46	-	-	-	-	-
Critical Hdwy Stg 2	5.46	-	-	-	-	-
Follow-up Hdwy	3.554	3.354	-	-	2.218	-
Pot Cap-1 Maneuver	999	1060	-	-	1599	-
Stage 1	1003	-	-	-	-	-
Stage 2	1012	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	997	1058	-	-	1596	-
Mov Cap-2 Maneuver	997	-	-	-	-	-
Stage 1	1001	-	-	-	-	-
Stage 2	1012	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	8.6	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	997	1596
HCM Lane V/C Ratio	-	-	0.005	-
HCM Control Delay (s)	-	-	8.6	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	0	0

Intersection												
Int Delay, s/veh	6.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	5	0	11	16	0	5	3	5	5	1	2	2
Future Vol, veh/h	5	0	11	16	0	5	3	5	5	1	2	2
Conflicting Peds, #/hr	1	0	1	0	0	0	1	0	0	0	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0	8	8	8	4	4	4
Mvmt Flow	5	0	12	17	0	5	3	5	5	1	2	2

Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	23	22	5	26	21	9	5	0	0	10	0	0
Stage 1	6	6	-	14	14	-	-	-	-	-	-	-
Stage 2	17	16	-	12	7	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.18	-	-	4.14	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.272	-	-	2.236	-	-
Pot Cap-1 Maneuver	994	876	1084	990	877	1079	1578	-	-	1597	-	-
Stage 1	1021	895	-	1011	888	-	-	-	-	-	-	-
Stage 2	1008	886	-	1014	894	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	985	872	1082	976	873	1078	1576	-	-	1597	-	-
Mov Cap-2 Maneuver	985	872	-	976	873	-	-	-	-	-	-	-
Stage 1	1018	893	-	1009	886	-	-	-	-	-	-	-
Stage 2	1000	884	-	1001	892	-	-	-	-	-	-	-

Approach	EB		WB			NB			SB		
HCM Control Delay, s	8.5		8.7			1.7			1.5		
HCM LOS	A		A								

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1576	-	-	1050	998	1597	-	-
HCM Lane V/C Ratio	0.002	-	-	0.016	0.022	0.001	-	-
HCM Control Delay (s)	7.3	0	-	8.5	8.7	7.3	0	-
HCM Lane LOS	A	A	-	A	A	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.1	0	-	-

Intersection						
Int Delay, s/veh	4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		T
Traffic Vol, veh/h	5	22	6	8	27	2
Future Vol, veh/h	5	22	6	8	27	2
Conflicting Peds, #/hr	0	3	3	0	0	3
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	3	3	5	5	2	2
Mvmt Flow	5	22	6	8	28	2

Major/Minor	Minor2	Major1		Major2	
Conflicting Flow All	52	35	33	0	0
Stage 1	32	-	-	-	-
Stage 2	20	-	-	-	-
Critical Hdwy	6.43	6.23	4.15	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-
Follow-up Hdwy	3.527	3.327	2.245	-	-
Pot Cap-1 Maneuver	954	1035	1560	-	-
Stage 1	988	-	-	-	-
Stage 2	1000	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	944	1029	1556	-	-
Mov Cap-2 Maneuver	944	-	-	-	-
Stage 1	981	-	-	-	-
Stage 2	997	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	8.7	3.1	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1556	-	1012	-	-
HCM Lane V/C Ratio	0.004	-	0.027	-	-
HCM Control Delay (s)	7.3	0	8.7	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Intersection						
Int Delay, s/veh	2.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	1	14	50	8	27	4
Future Vol, veh/h	1	14	50	8	27	4
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	1	1	0	0	1	1
Mvmt Flow	1	15	54	9	29	4

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	68	0	-	0	81
Stage 1	-	-	-	-	64
Stage 2	-	-	-	-	17
Critical Hdwy	4.11	-	-	-	6.41
Critical Hdwy Stg 1	-	-	-	-	5.41
Critical Hdwy Stg 2	-	-	-	-	5.41
Follow-up Hdwy	2.209	-	-	-	3.509
Pot Cap-1 Maneuver	1540	-	-	-	924
Stage 1	-	-	-	-	961
Stage 2	-	-	-	-	1008
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1533	-	-	-	914
Mov Cap-2 Maneuver	-	-	-	-	914
Stage 1	-	-	-	-	955
Stage 2	-	-	-	-	1003

Approach	EB	WB	SB
HCM Control Delay, s	0.5	0	9
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1533	-	-	-	924
HCM Lane V/C Ratio	0.001	-	-	-	0.036
HCM Control Delay (s)	7.3	0	-	-	9
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.1

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	1	40	52	4	16	6
Future Vol, veh/h	1	40	52	4	16	6
Conflicting Peds, #/hr	2	0	0	2	0	5
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	3	3	2	2
Mvmt Flow	1	42	55	4	17	6

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	61	0	-	0	103 64
Stage 1	-	-	-	-	59 -
Stage 2	-	-	-	-	44 -
Critical Hdwy	4.12	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.218	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1542	-	-	-	895 1000
Stage 1	-	-	-	-	964 -
Stage 2	-	-	-	-	978 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1539	-	-	-	891 993
Mov Cap-2 Maneuver	-	-	-	-	891 -
Stage 1	-	-	-	-	961 -
Stage 2	-	-	-	-	976 -

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	9
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1539	-	-	-	917
HCM Lane V/C Ratio	0.001	-	-	-	0.025
HCM Control Delay (s)	7.3	0	-	-	9
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.1

Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Vol, veh/h	0	56	56	5	17	0
Future Vol, veh/h	0	56	56	5	17	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	3	3	2	2
Mvmt Flow	0	60	60	5	18	0

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	65	0	-	0	123 63
Stage 1	-	-	-	-	63 -
Stage 2	-	-	-	-	60 -
Critical Hdwy	4.1	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.2	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1550	-	-	-	872 1002
Stage 1	-	-	-	-	960 -
Stage 2	-	-	-	-	963 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1550	-	-	-	872 1002
Mov Cap-2 Maneuver	-	-	-	-	872 -
Stage 1	-	-	-	-	960 -
Stage 2	-	-	-	-	963 -

Approach	EB	WB	SB
HCM Control Delay, s	0	0	9.2
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1550	-	-	-	872
HCM Lane V/C Ratio	-	-	-	-	0.021
HCM Control Delay (s)	0	-	-	-	9.2
HCM Lane LOS	A	-	-	-	A
HCM 95th %tile Q(veh)	0	-	-	-	0.1

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	8	84	11	15	69	7	38	0	55	22	0	27
Future Vol, veh/h	8	84	11	15	69	7	38	0	55	22	0	27
Conflicting Peds, #/hr	0	0	5	5	0	0	3	0	3	3	0	3
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	99	99	99	99	99	99	99	99	99	99	99	99
Heavy Vehicles, %	1	1	1	2	2	2	1	1	1	1	1	1
Mvmt Flow	8	85	11	15	70	7	38	0	56	22	0	27

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	77	0	0	101	0	0	232	219	99	242	221	77
Stage 1	-	-	-	-	-	-	112	112	-	104	104	-
Stage 2	-	-	-	-	-	-	120	107	-	138	117	-
Critical Hdwy	4.11	-	-	4.12	-	-	7.11	6.51	6.21	7.11	6.51	6.21
Critical Hdwy Stg 1	-	-	-	-	-	-	6.11	5.51	-	6.11	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.11	5.51	-	6.11	5.51	-
Follow-up Hdwy	2.209	-	-	2.218	-	-	3.509	4.009	3.309	3.509	4.009	3.309
Pot Cap-1 Maneuver	1528	-	-	1491	-	-	725	681	960	714	679	987
Stage 1	-	-	-	-	-	-	895	805	-	904	811	-
Stage 2	-	-	-	-	-	-	887	809	-	868	801	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1528	-	-	1484	-	-	690	666	953	662	664	984
Mov Cap-2 Maneuver	-	-	-	-	-	-	690	666	-	662	664	-
Stage 1	-	-	-	-	-	-	885	796	-	899	802	-
Stage 2	-	-	-	-	-	-	850	800	-	810	792	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			1.2			9.9			9.7		
HCM LOS							A			A		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	825	1528	-	-	1484	-	-	808
HCM Lane V/C Ratio	0.114	0.005	-	-	0.01	-	-	0.061
HCM Control Delay (s)	9.9	7.4	0	-	7.5	0	-	9.7
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	7	146	8	14	42	8	27	0	49	27	0	22
Future Vol, veh/h	7	146	8	14	42	8	27	0	49	27	0	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	8	159	9	15	46	9	29	0	53	29	0	24

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	55	0	0	168	0	0	273	265	164	287	265	51
Stage 1	-	-	-	-	-	-	180	180	-	81	81	-
Stage 2	-	-	-	-	-	-	93	85	-	206	184	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1550	-	-	1410	-	-	679	640	881	665	640	1017
Stage 1	-	-	-	-	-	-	822	750	-	927	828	-
Stage 2	-	-	-	-	-	-	914	824	-	796	747	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1550	-	-	1410	-	-	655	629	881	616	629	1017
Mov Cap-2 Maneuver	-	-	-	-	-	-	655	629	-	616	629	-
Stage 1	-	-	-	-	-	-	817	746	-	921	819	-
Stage 2	-	-	-	-	-	-	883	815	-	743	743	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			1.7			10.1			10.2		
HCM LOS							B			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	785	1550	-	-	1410	-	-	749
HCM Lane V/C Ratio	0.105	0.005	-	-	0.011	-	-	0.071
HCM Control Delay (s)	10.1	7.3	0	-	7.6	0	-	10.2
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2

Intersection						
Int Delay, s/veh	2.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		
Traffic Vol, veh/h	50	33	9	565	642	14
Future Vol, veh/h	50	33	9	565	642	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	36	10	614	698	15

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1340	706	713	0	-	0
Stage 1	706	-	-	-	-	-
Stage 2	634	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	168	436	887	-	-	-
Stage 1	489	-	-	-	-	-
Stage 2	529	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	165	436	887	-	-	-
Mov Cap-2 Maneuver	165	-	-	-	-	-
Stage 1	481	-	-	-	-	-
Stage 2	529	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	32.5	0.1	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	887	-	219	-	-
HCM Lane V/C Ratio	0.011	-	0.412	-	-
HCM Control Delay (s)	9.1	0	32.5	-	-
HCM Lane LOS	A	A	D	-	-
HCM 95th %tile Q(veh)	0	-	1.9	-	-

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		
Traffic Vol, veh/h	11	11	2	563	672	3
Future Vol, veh/h	11	11	2	563	672	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	12	2	612	730	3

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1348	732	733	0	-	0
Stage 1	732	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	166	421	872	-	-	-
Stage 1	476	-	-	-	-	-
Stage 2	539	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	166	421	872	-	-	-
Mov Cap-2 Maneuver	166	-	-	-	-	-
Stage 1	475	-	-	-	-	-
Stage 2	539	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	21.8	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	872	-	238	-	-
HCM Lane V/C Ratio	0.002	-	0.1	-	-
HCM Control Delay (s)	9.1	0	21.8	-	-
HCM Lane LOS	A	A	C	-	-
HCM 95th %tile Q(veh)	0	-	0.3	-	-

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		T
Traffic Vol, veh/h	11	27	8	545	845	3
Future Vol, veh/h	11	27	8	545	845	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	29	9	592	918	3

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1530	920	921	0	-	0
Stage 1	920	-	-	-	-	-
Stage 2	610	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	129	328	741	-	-	-
Stage 1	388	-	-	-	-	-
Stage 2	542	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	127	328	741	-	-	-
Mov Cap-2 Maneuver	127	-	-	-	-	-
Stage 1	381	-	-	-	-	-
Stage 2	542	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	24.6	0.1	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	741	-	225	-	-
HCM Lane V/C Ratio	0.012	-	0.184	-	-
HCM Control Delay (s)	9.9	0	24.6	-	-
HCM Lane LOS	A	A	C	-	-
HCM 95th %tile Q(veh)	0	-	0.7	-	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		
Traffic Vol, veh/h	5	11	2	548	870	2
Future Vol, veh/h	5	11	2	548	870	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	12	2	596	946	2

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1547	947	948	0	-	0
Stage 1	947	-	-	-	-	-
Stage 2	600	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	126	317	724	-	-	-
Stage 1	377	-	-	-	-	-
Stage 2	548	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	125	317	724	-	-	-
Mov Cap-2 Maneuver	125	-	-	-	-	-
Stage 1	375	-	-	-	-	-
Stage 2	548	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	23.3	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	724	-	214	-	-
HCM Lane V/C Ratio	0.003	-	0.081	-	-
HCM Control Delay (s)	10	0	23.3	-	-
HCM Lane LOS	A	A	C	-	-
HCM 95th %tile Q(veh)	0	-	0.3	-	-

1.21144 Williams Industrial

<u>Proposed Use</u>										
								Total Trips		
Land Use	Setting	Size	Units	Model	Rate	Units	Inbound %	Inbound	Outbound	Total
Industrial Park (LU 130) - Marysville		2,055,069 sf								
Daily	General Urban/Suburban			Rate	3.37	per ksf	50%	3,463	3,463	6,926
AM Peak Hour	General Urban/Suburban			Rate	0.34	per ksf	81%	566	133	699
PM Peak Hour	General Urban/Suburban			Rate	0.34	per ksf	22%	154	545	699

Notes:

1. Trip rates based on Institute of Transportation Engineers' (ITE) *Trip Generation* 11th Edition equation and average trip rate as shown above.

Appendix E: Snohomish County Key Intersections

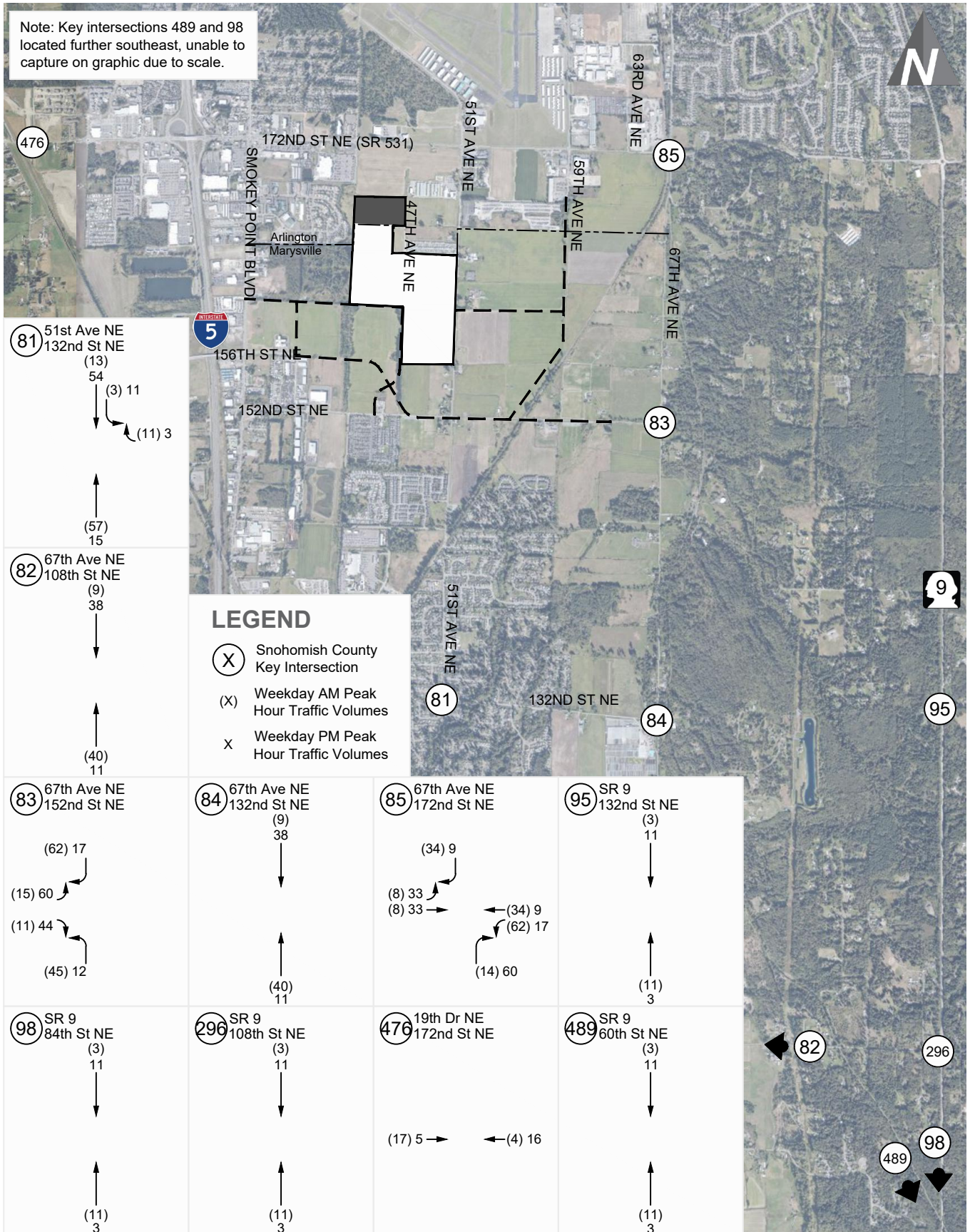
2026 AM														
Key Intersection #	North/South Street	East/West Street	EBL	EBT	EBR	NBL	NBT	NBR	WBL	WBT	WBR	SBL	SBT	SBR
81	51st Ave NE	132nd St NE					57				11	3	13	
82	67th Ave NE	108th St NE					40						9	
83	67th Ave NE	152nd St NE	15		11	45								62
84	67th Ave NE	132nd St NE					40						9	
85	67th Ave NE	172nd St NE	8	8				14	62	34				34
95	SR 9	132nd St NE					11						3	
98	SR 9	84th St NE					11						3	
296	SR 9	108th St NE					11						3	
476	19th Dr NE	172nd St NE		17						4				
489	SR 9	60th St NE					11						3	

2026 PM														
Key Intersection #	North/South Street	East/West Street	EBL	EBT	EBR	NBL	NBT_PM	NBR	WBL	WBT	WBR	SBL	SBT_AM	SBR
81	51st Ave NE	132nd St NE					15				3	11	54	
82	67th Ave NE	108th St NE					11						38	
83	67th Ave NE	152nd St NE	60		44	12								17
84	67th Ave NE	132nd St NE					11						38	
85	67th Ave NE	172nd St NE	33	33				60	17	9				9
95	SR 9	132nd St NE					3						11	
98	SR 9	84th St NE					3						11	
296	SR 9	108th St NE					3						11	
476	19th Dr NE	172nd St NE		5						16				
489	SR 9	60th St NE					3						11	

2032 AM														
Key Intersection #	North/South Street	East/West Street	EBL	EBT	EBR	NBL	NBT_PM	NBR	WBL	WBT	WBR	SBL	SBT_AM	SBR
81	51st Ave NE	132nd St NE					57				11	3	13	
82	67th Ave NE	108th St NE					57						13	
83	67th Ave NE	152nd St NE	13		15	62								57
84	67th Ave NE	132nd St NE					57						13	
85	67th Ave NE	172nd St NE	4	3				13	57	11				17
95	SR 9	132nd St NE					6						1	
98	SR 9	84th St NE					6						1	
296	SR 9	108th St NE					6						1	
476	19th Dr NE	172nd St NE		11						3				
489	SR 9	60th St NE					6						1	

2032 PM														
Key Intersection #	North/South Street	East/West Street	EBL	EBT	EBR	NBL	NBT_PM	NBR	WBL	WBT	WBR	SBL	SBT_AM	SBR
81	51st Ave NE	132nd St NE					17				3	11	54	
82	67th Ave NE	108th St NE					15						55	
83	67th Ave NE	152nd St NE	54		60	17								57
84	67th Ave NE	132nd St NE					15						55	
85	67th Ave NE	172nd St NE	16	11				54	15	3				17
95	SR 9	132nd St NE					2						5	
98	SR 9	84th St NE					2						5	
296	SR 9	108th St NE					2						5	
476	19th Dr NE	172nd St NE		3						11				
489	SR 9	60th St NE					2						5	

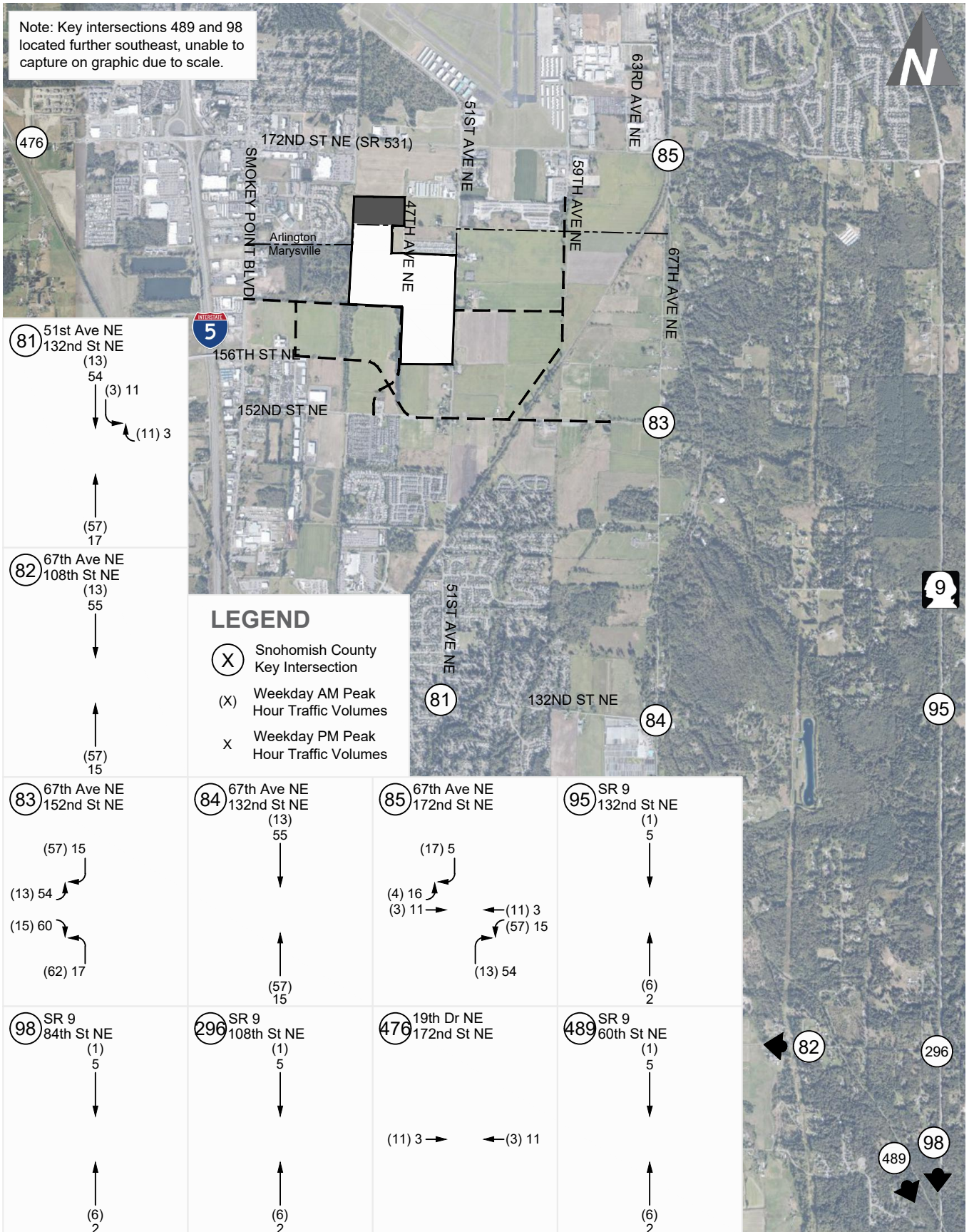
Note: Key intersections 489 and 98 located further southeast, unable to capture on graphic due to scale.



Future (2026) Peak Hour Project Trips at Snohomish County Key Intersections

APPENDIX

Note: Key intersections 489 and 98 located further southeast, unable to capture on graphic due to scale.



Future (2032) Peak Hour Project Trips at Snohomish County Key Intersections

APPENDIX